

February 9, 2011

Mr. Jamey Bertram Burns & McDonnell 9400 Ward Parkway Kansas City, MO 64114

Subject: Evaluation of Liquefaction Potential

Structures EP239-1, EP240 and EP242 SDG&E Sunrise Powerlink Project

San Diego and Imperial Counties, California

URS Project No. 27661032.01001

Dear Mr. Bertram:

URS Corporation Americas (URS) is submitting this letter to summarize the evaluation of the potential for liquefaction at structures EP239-1, EP240 and EP242 for the Sunrise Powerlink Project. This letter addresses Mitigation Measure G-4b, which requires evaluation of the potential for liquefaction for these structures.

BACKGROUND

Liquefaction is a phenomenon where saturated coarse-grained soils (less than 50% passing the No. 200 sieve) lose their strength and acquire some mobility from strong ground motion. While not related to liquefaction, some fine-grained soils (more than 50% passing the No. 200 sieve) are vulnerable to similar liquefaction-type behavior or strength loss.

Geologic hazards, including the potential for liquefaction, were discussed in the October 1, 2010 URS report titled "Geotechnical and Geologic Hazards Investigation, Sunrise Powerlink Project, San Diego and Imperial Counties, California". The report concluded that the potential for liquefaction required additional evaluation in several areas along the alignment, including Jacumba Valley, where EP239-1, EP240 and EP242 are located.

The need for additional evaluation of the potential for liquefaction was noted by the California Public Utilities Commission (CPUC) through their consultant, Aspen Environmental (Aspen). On June 28, 2010, Aspen transmitted a letter from Geotechnical Consultants, Inc. dated June 18, 2010 that provided review comments on all geotechnical-related mitigation measures.

EVALUATION

URS completed a subsurface exploration consisting of one boring at EP239-1 and one boring at EP240 on January 28 and 29, 2011. The borings were drilled to a depth of approximately 50 feet.

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Laboratory testing was performed to evaluate grain size distribution and plasticity characteristics to support the assessment of the potential for liquefaction.

The findings from the subsurface exploration and laboratory testing indicate that loose to medium dense sand and silty to clayey sand occur to depths of at least 50 feet. Groundwater was observed in the borings at depths ranging from about 14 to 17 feet.

A boring was not drilled at structure EP242. Seismic refraction data collected at the structure indicate that potentially loose alluvial soil is present to a depth of about 15 to 20 feet, and the alluvium is underlain by weathered rock.

The potential for liquefaction in coarse grained soils was evaluated using the Standard Penetration Test blow counts (SPT N-Values) from the borings in accordance with current criteria and procedures (Youd, *et al.*, 2001; Idriss and Boulanger, 2008). The procedure for evaluating liquefaction potential is empirical and it is based on data and observations at sites that have, and have not liquefied during an earthquake.

The potential for liquefaction was assessed in terms of a factor of safety against liquefaction, FS_{liq} . The factor of safety is defined as the Cyclic Resistance Ratio required to resist liquefaction (CRR) divided by the Cyclic Stress Ratio (CSR) generated by the design ground motion. The seismic demand is a function of the anticipated peak ground acceleration (PGA). The assessment adopted a PGA of 0.25g, representative of an earthquake with a probability of exceedence of 10 percent in 50 years, and an earthquake magnitude of M7.0. The calculations assumed the depth to groundwater was five feet above the highest level measured in the borings. Soils were considered potentially liquefiable if the factor of safety against liquefaction was calculated to be less than about 1.1.

A screening evaluation was completed by comparing the laboratory test data to evaluation criteria that relates potential behavior to index properties. Screening evaluations indicate that fine-grained soils should not be susceptible to liquefaction-type behavior or strength loss.

CONCLUSIONS AND RECOMMENDATIONS

The results of our evaluation indicate there is potential for liquefaction to occur at structures EP239-1 and EP240. To mitigate the potential for liquefaction, the foundations for these structures should be designed considering:

- A reduction in the axial and lateral soil resistances within the potentially liquefiable soils.
- The downdrag load on the pile shaft that can develop from liquefaction-induced settlement.
- A settlement below the pile tip that will provide acceptable performance of the structure.

A qualitative evaluation of liquefaction potential at structure EP242 was performed considering the results of the subsurface investigation and evaluation of the potential for liquefaction at structures EP239-1 and EP240. Based on that information, as well as the seismic refraction survey completed

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at EP242, the higher topography of the area and the drilled shaft installation logs from the nearby Southwest Powerlink (SWPL) structure, we conclude the potential for groundwater to occur within the alluvium is low. Consequently, it is our opinion that the potential for liquefaction to occur at structure EP242 is very low. If groundwater is encountered within the alluvium during construction, revised foundation design parameters will be provided.

Exp. 3/31/12

OF CALL

If you any questions regarding the letter please sourcet us at (858) 812-9292.

Sincerely,

URS CORPORATION

Charles Rebin (Rob) Stroop, G.E. 2298 Senior Project Geotechnical Engineer

Kelly C. Giesing, G.E. 2749 Project Geotechnical Engineer

Attachments:

Log of Boring B-EP239 Log of Boring B-EP240 Results of Laboratory Testing

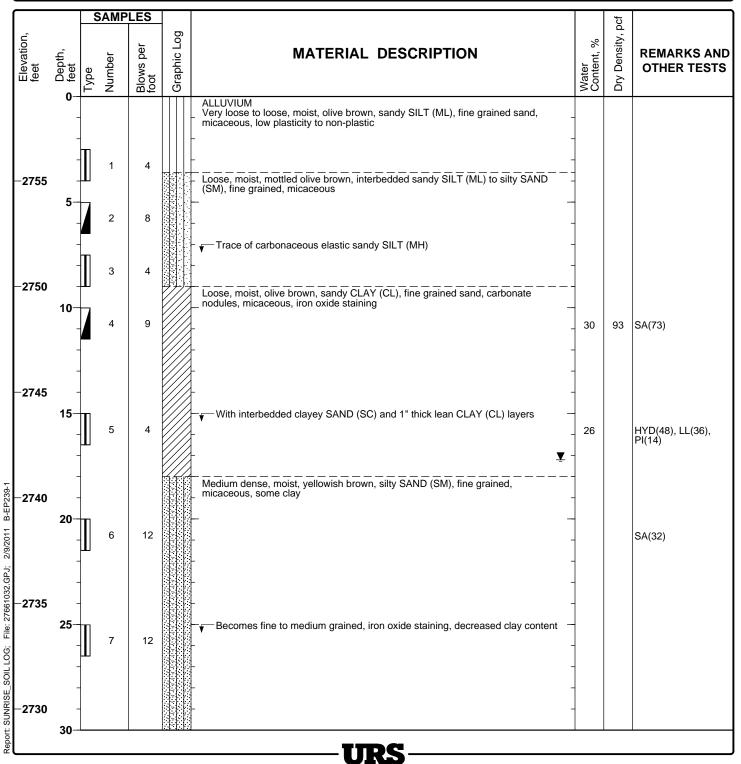
Section/Tower No.: San Diego and Imperial Counties, California

Project Number: 27661032

Log of Boring B-EP239-1

Sheet 1 of 2

Date(s) Drilled	01/28/11	Logged By	J. Gratzer	Checked By	M. Schmoll
Drilling Method	Rotary Wash	Drill Bit Size/Type	HWT-Tri-cone	Total Depth of Borehole	51 feet
Drill Rig Type	Burley 4000, Rig #1	Drilling Contractor	Crux	Approximate Surface Elevation	2759.0 ft (NAVD 88)
Water Level Depth (Feet		Sampling Method(s)	SPT/2.5" I.D.	Hammer Data 140 II	os/30-inch drop
Borehole Backfill	Bentonite chips	Coordinate Location (NA	AD 83) 32.63029 -116.17922	Location Link	1, Section 9C

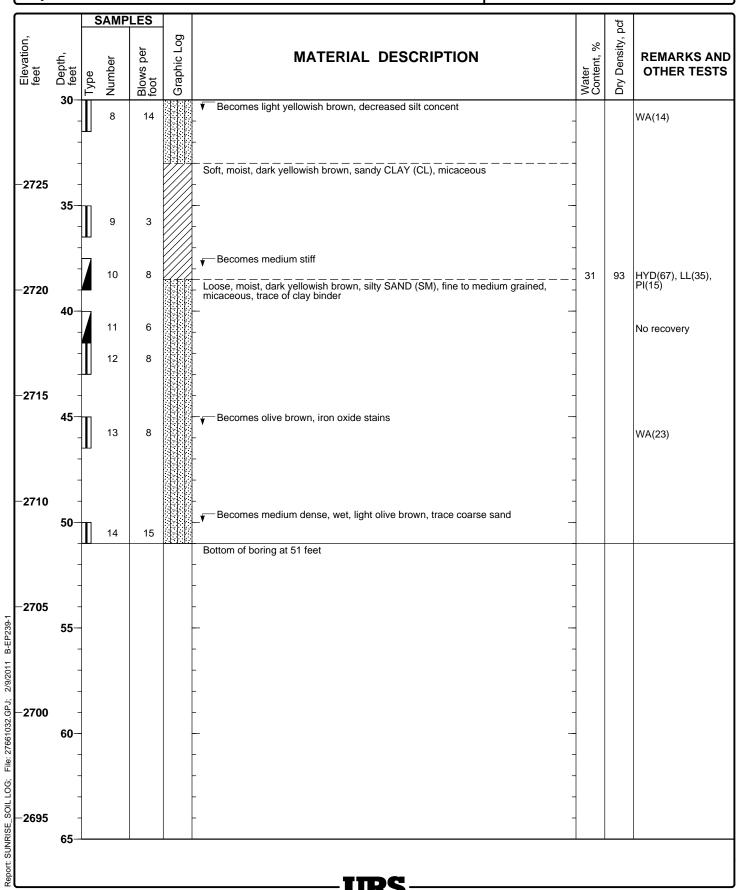


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Log of Boring B-EP239-1

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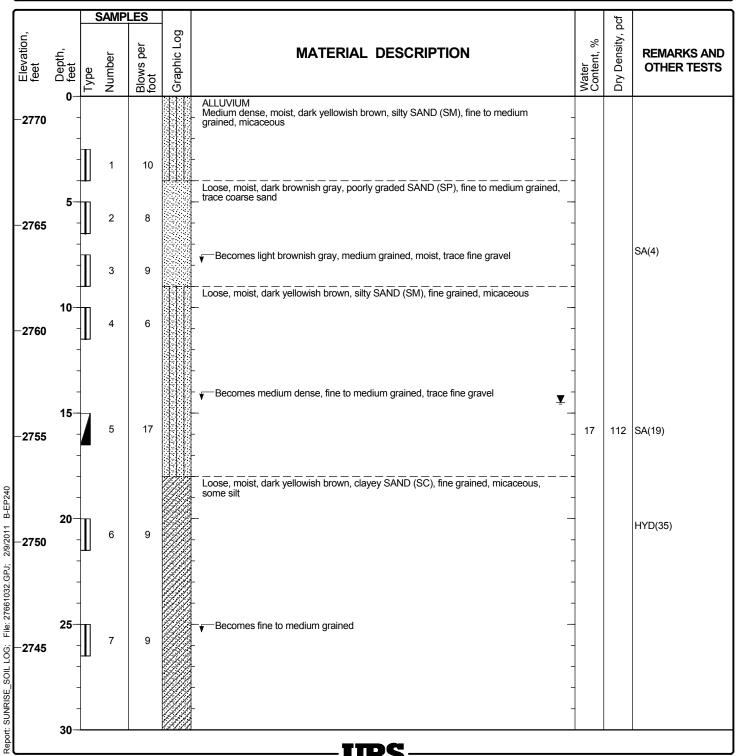
Section/Tower No.: San Diego and Imperial Counties, California

Project Number: 27661032

Log of Boring B-EP240

Sheet 1 of 2

Date(s) Drilled	01/29/11	Logged By	J. Gratzer	Checked By	M. Schmoll
Drilling Method	Rotary Wash	Drill Bit Size/Type	HWT-Tri-cone	Total Depth of Borehole	51.5 feet
Drill Rig Type	Burley 4000, Rig #1	Drilling Contractor	Crux	Approximate Surface Elevation	2771.1 ft (NAVD 88)
Water Level Depth (Fee		Sampling Method(s)	SPT/2.5" I.D.	Hammer Data 140 lb	s/30-inch drop
Borehole Backfill	Bentonite chips	Coordinate Location (NA	D 83) 32.63055 -116.17343	Location Link 1	, Section 9C

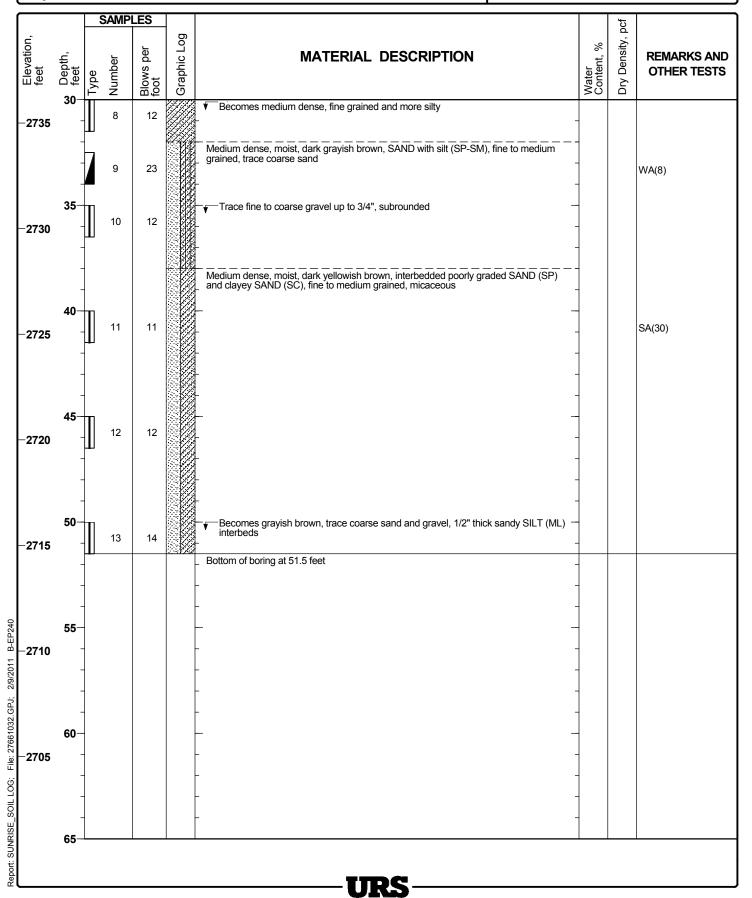


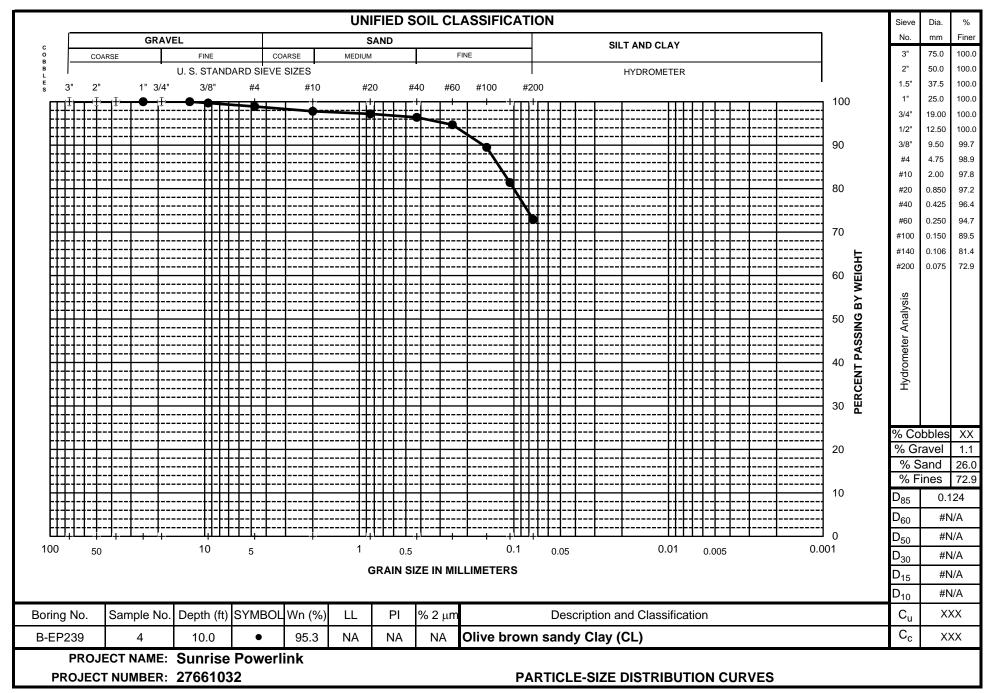
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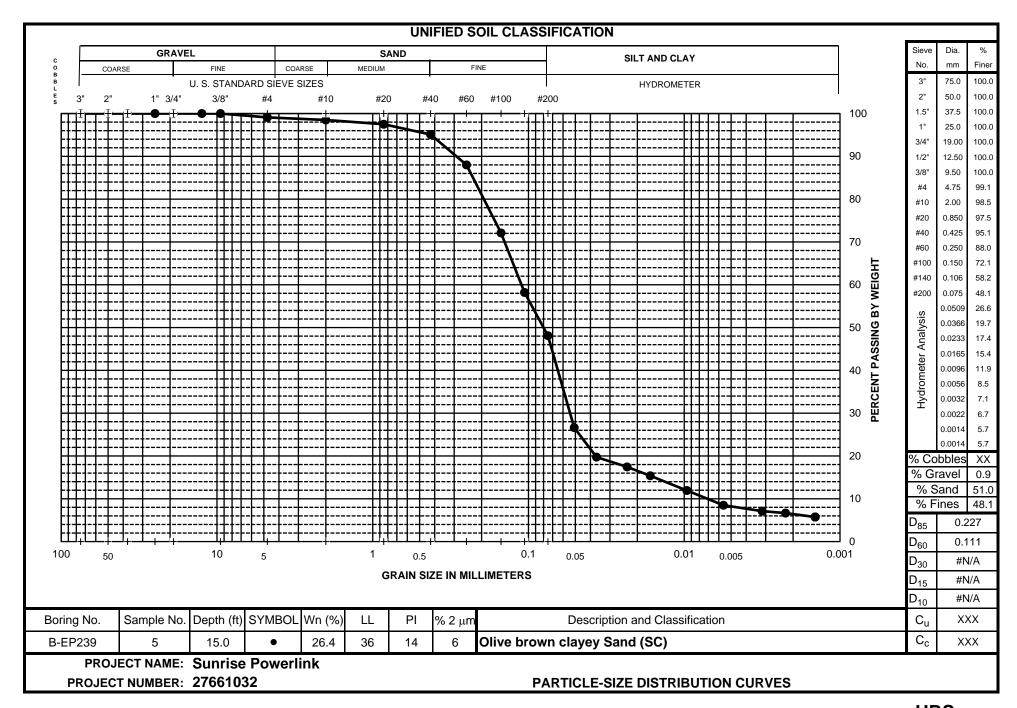
Log of Boring B-EP240

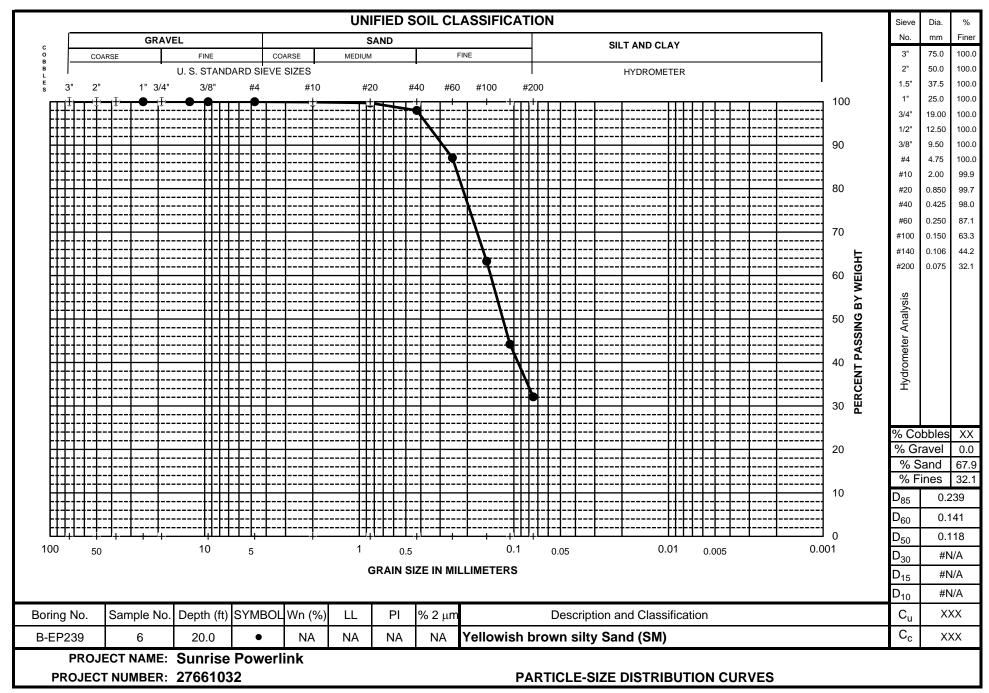
Sheet 2 of 2



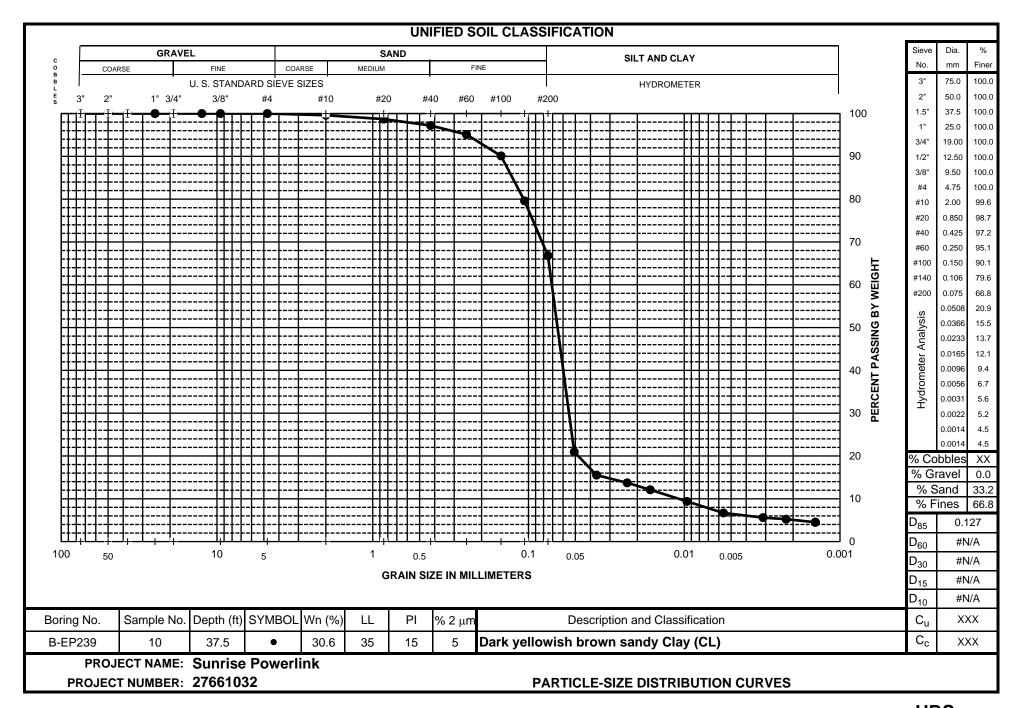


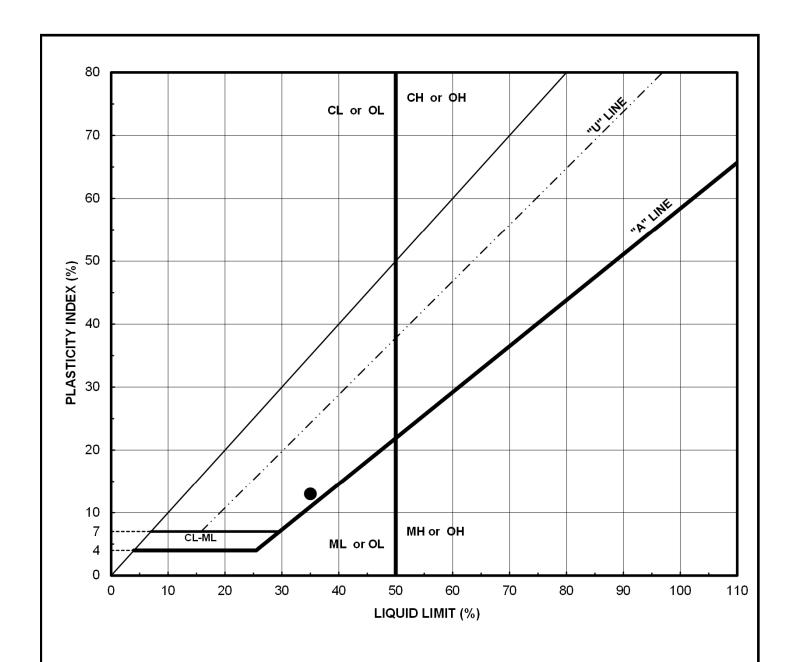
Sieve Sunrise EP239 010 URS





Sieve Sunrise EP239 020 URS





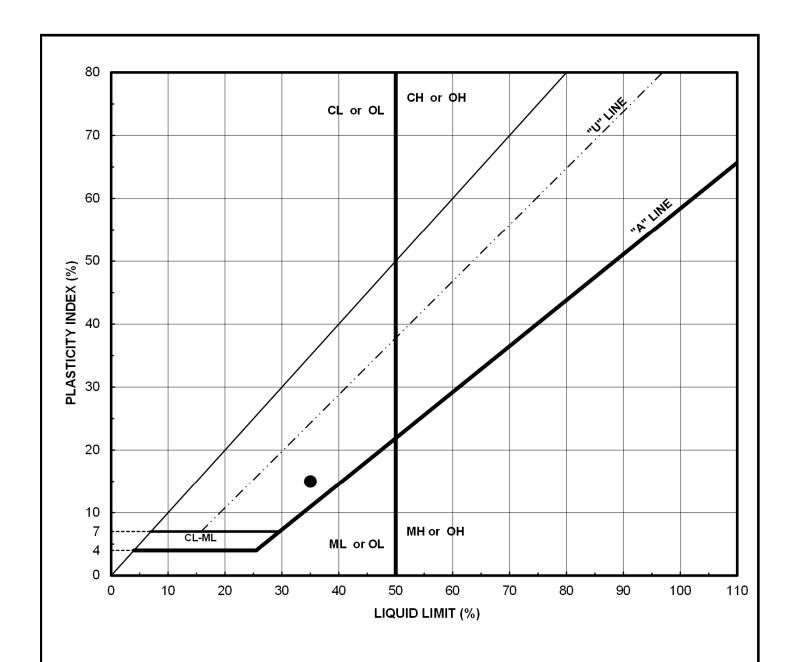
BORING No.	SAMPLE No.	DEPTH (ft.)	WATER CONTENT (%)	LL (%)	PI (%)	DESCRIPTION / CLASSIFICATION
B-EP239	5	15.0	26.4	35	13	Olive brown clayey Sand (SC)

Project Name: Sunrise Powerlink

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PLASTICITY CHART



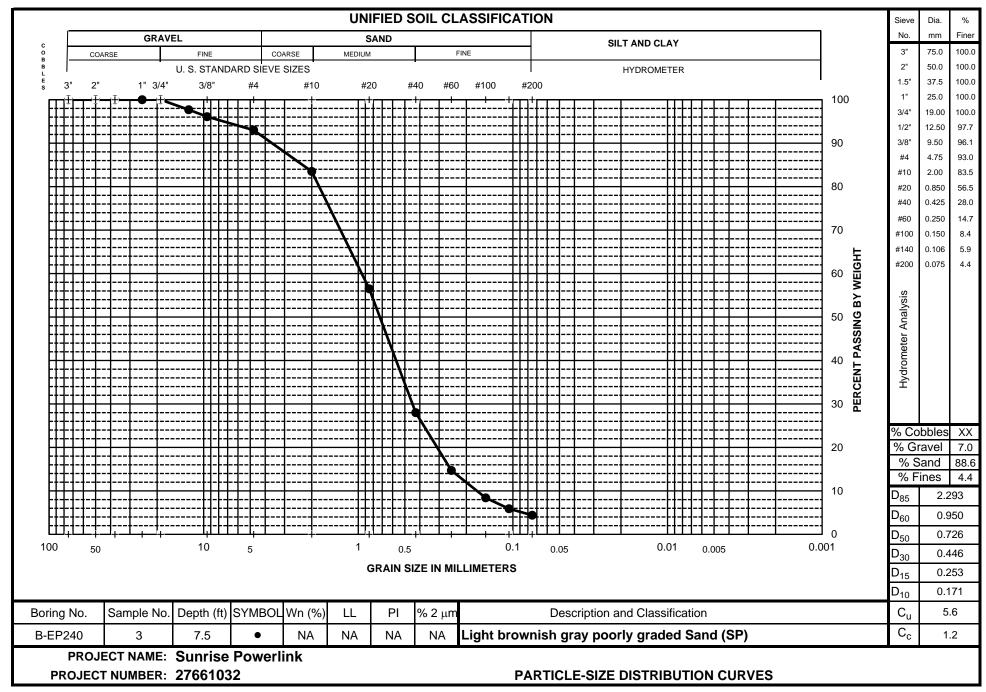


BORING No.	SAMPLE No.	DEPTH (ft.)	WATER CONTENT (%)	LL (%)	PI (%)	DESCRIPTION / CLASSIFICATION
B-EP239	10	37.5	30.6	35	15	Dark yellowish brown sandy Clay (CL)

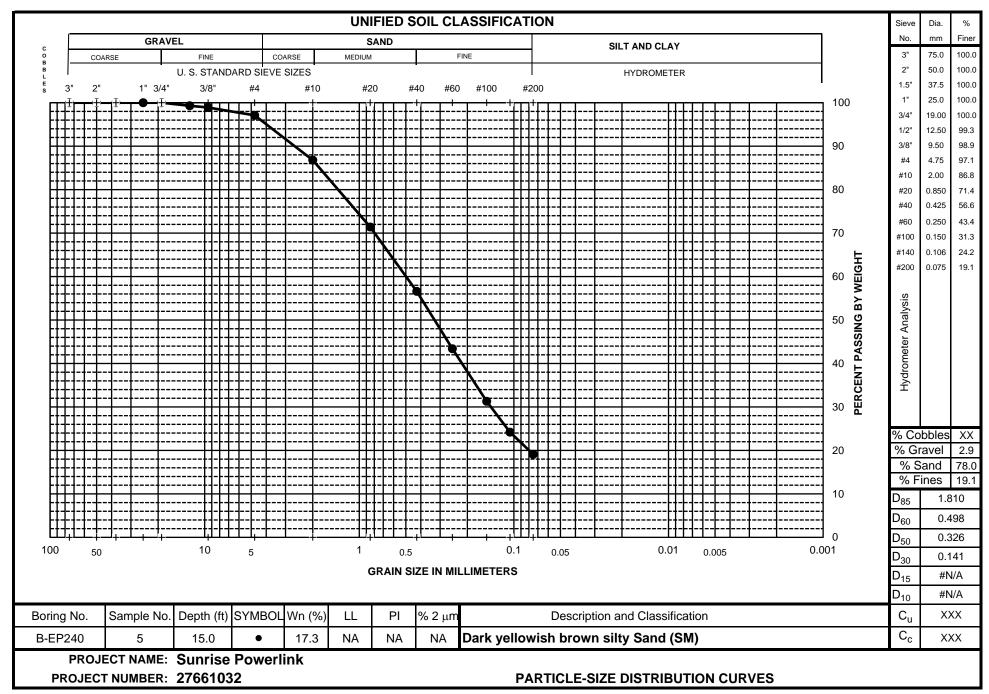
Project Name: Sunrise Powerlink PLASTICITY CHART

Project Number: 27661032

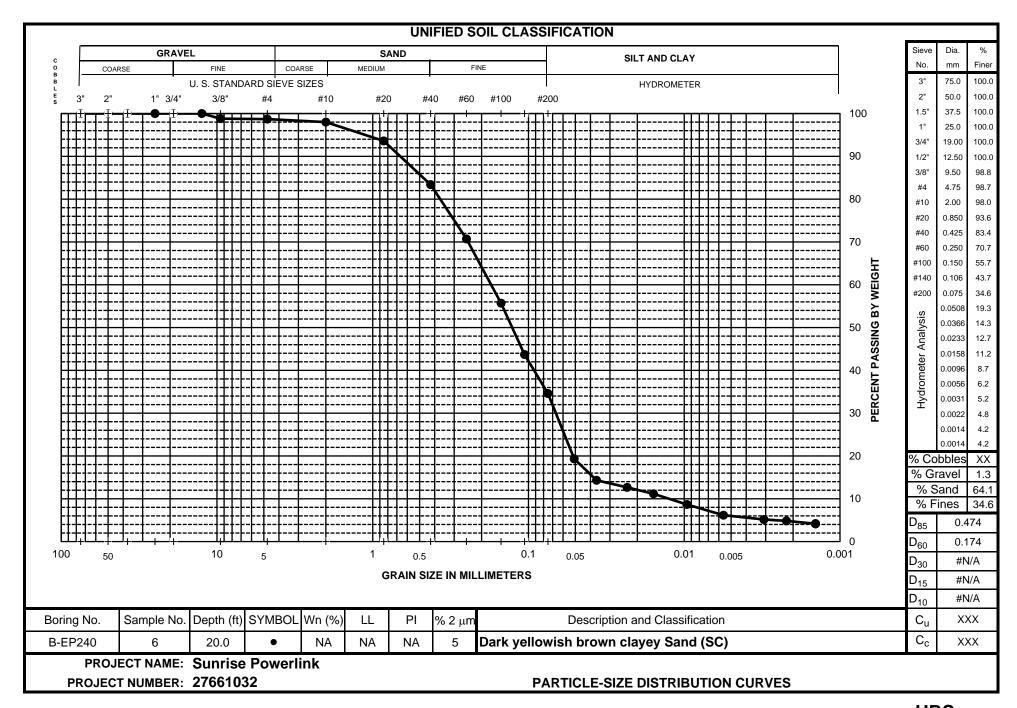


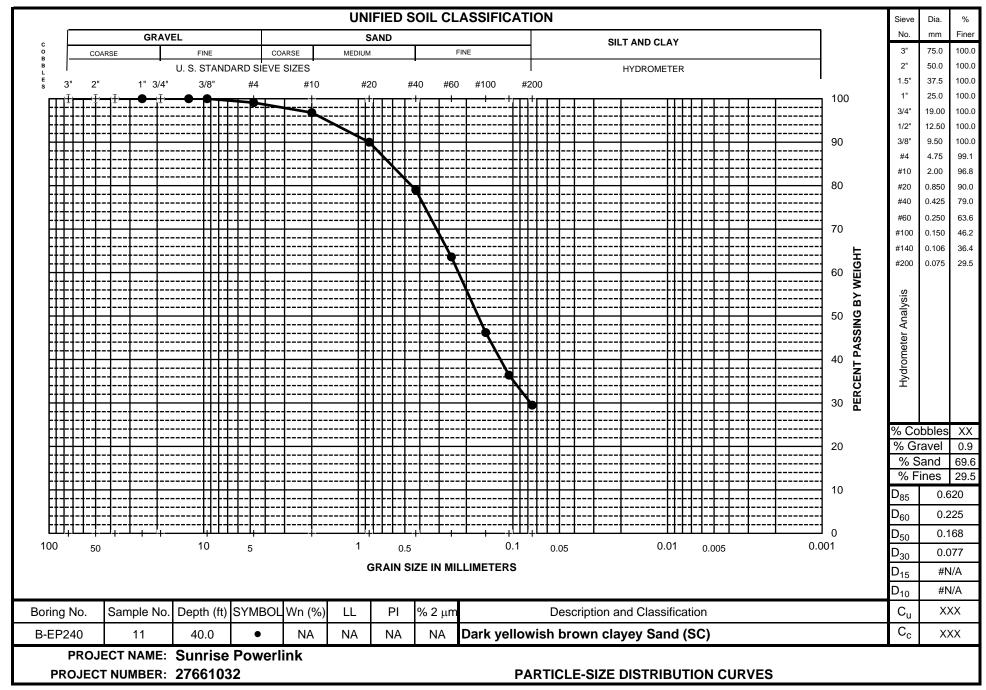


Sieve Sunrise EP240 007



Sieve Sunrise EP240 015





Sieve Sunrise EP240 040 URS