



SAN DIEGO GAS & ELECTRIC SUNCREST SUBSTATION DRAINAGE STUDY

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March 30, 2010

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1. INTRODUCTION

1.1 Purpose of Study

This study has been prepared in order to provide hydrologic and hydraulic analyses in support of Grading and Improvement plans for the proposed Suncrest Substation for San Diego Gas & Electric Company (SDGE). The proposed site development is for substation pad and access road. This study shows surrounding detail and onsite drainage patterns.

A hydrologic model was developed using industry-standard engineering practices. First, watershed boundaries, points of flow concentrations and land uses were established. The data was then entered into hydrologic modeling software for calculations of the 100-year, 6-hour peak storm flows. Finally, a hydrology exhibit was prepared to illustrate the modeling procedure and hydrologic characteristics of the existing and proposed conditions.

Hydraulic calculations were performed to size the proposed storm drain collection systems, brow ditches, and detention basins.

1.2 Scope

This study involves the hydrologic and hydraulic analysis of the proposed substation pad and access road and existing run-on flow areas. Storm drain pipes, brow ditches, and detention basins within the project limits have been sized to convey the 100-year design flows. This study should be used to evaluate pre- and post-development hydrologic conditions as well as post-development hydraulic design.

2. DESCRIPTION OF WATERSHED

2.1 Area Characteristics

The existing site is a large undeveloped parcel and is located approximately six miles east of the unincorporated community of Alpine within San Diego County, California. The site is located approximately 3 miles west of Japatul Valley Road and immediately south of Bell Bluff Truck Trail. The existing site is located north of the Cleveland National Forest and is comprised of undisturbed mountainous terrain with native vegetation. Runoff flows overland from the north to the south, then shifts towards the southwest as it crosses the proposed substation pad. The proposed site eventually drains to Peterson Creek, which drains into the Sweetwater River, which then drains into the Loveland Reservoir.

2.2 Soil Type

The soil classification used in this analysis is Type "D". The San Diego County Hydrology Manual recommends that soil type "D" be assumed for all urban areas when determining runoff coefficients. Type "D" describes soils having very slow infiltration rates when thoroughly wet. Type "D" soils are characterized as severely erodible; requiring protective measures prior to and during soil disturbances.

2.3 Land Uses

The existing land use is undeveloped. The proposed site will consist of asphalt pavement for the access road, decomposed granite (Class II Base) for the substation pad, and landscaping surrounding much of the substation pad.

3. METHODOLOGY

3.1 Modified Rational Method

Per the San Diego County Hydrology Manual (dated June 2003) for watersheds less than one square mile with stream junctions, we used the modified rational formula, as excerpted below. Applicable tables and charts for this method can be found in Appendix A of this report. Calculations were prepared using AES software. The calculations are included in Appendix B.

From the Hydrology Manual, this method is as follows:

$Q = CIA$, where:

Q is the discharge in cubic feet per second (cfs)

C is a runoff coefficient, proportion of the rainfall that runs off the surface (no units).

I is the average rainfall intensity in inches per hour for a storm duration equal to the time of concentration (T_c) of the contributing drainage area.

A is the drainage area in acres tributary to the design point.

Runoff Coefficient, C

For existing conditions, the San Diego County Hydrology Manual was initially used. We chose the Permanent Open Space runoff coefficient within soil Type “D” (see section 2.2 for soil type discussion). Under this assumption, the runoff coefficient is 0.35 (see Appendix A).

For proposed conditions, runoff coefficient (C) values were chosen as follows: 0.45 for the substation pad (Class II Base), 0.40 for graded slopes, 0.95 for paved areas, and 0.35 for undisturbed areas. Weighted C values were calculated for the proposed improvement areas by the following equation: $C = 0.45 \times (\% \text{ substation pad}) + 0.40 \times (\% \text{ graded slope}) + 0.95 \times (\% \text{ pavement}) + 0.35 \times (\% \text{ undisturbed})$. See Appendix B for calculations.

Time of Concentration, T_c

The time of concentration is the time required for runoff to flow from the most remote part of the watershed to the outlet point under consideration.

Methods of calculation differ for natural watersheds (non-urbanized) and for urban drainage systems.

- a) Natural watersheds: Obtain T_c from the natural watershed nomograph.

- b) Urban drainage systems: In the case of urban drainage systems, the time of concentration at any point within the drainage area is given by:

$$T_c = T_i + T_f \text{ where}$$

T_i is the inlet time or the time required for the storm water to flow to the first inlet in the system. It is the sum of time in overland flow across lots and in the street gutter.

T_f is the travel time or the time required for the storm water to flow in the storm drain from the most upstream inlet to the point in question.

Travel Time, T_f , is computed by dividing the length of storm drain by the computed flow velocity. Since the velocity normally changes at each inlet because of changes in flow rate or slope, total travel time must be computed as the sum of the travel times for each section of the storm drain.

The overland flow component of inlet time, T_i , may be estimated by the use of the Urban Areas Overland Time of Flow Curves.

Rainfall Intensity, I

Using the computed time of concentration and 6-hour precipitation, the rainfall intensity can be obtained from the Intensity - Duration Chart (see Appendix A).

The rational method calculations were performed using Advanced Engineering Software (AES) RATSCx 2008 program, version 15.0. The model output is provided in Appendix B of this report. The model was developed from the drainage boundaries and points of storm flow concentration shown on the Hydrology Maps (Appendix C). These were determined using the grading/improvement plans and two-foot contour data.

3.2 Storm Frequency

Per the requirements of the San Diego County Hydrology Manual, this study used the 100-year frequency storm. The 100-year, six-hour rainfall amount of 3.6 inches was selected from the rainfall isopluvial map shown in Appendix A.

3.3 Access Road Hydrology – Alternate Methodology

The access road to the substation pad will have several culvert crossings constructed to convey runoff under the road from north to south to keep the natural drainage course. In an effort to be more efficient, Nolte used an alternate methodology to determine the 100-year flows for sizing the proposed culvert crossings. Nolte delineated the overall drainage basins discharging to each culvert. The basins were grouped together by relative size and slope. Then, a few select basins from each group were delineated and flows calculated using the Modified Rational Method described above. The results indicated a cfs per acre ranging from 1.8 to 2.0. Depending on the drainage basin group, Nolte applied the cfs/acre ratio to the other basins that contribute runoff to the proposed access road. These flows were used to size the culvert crossings.

4. SUMMARY

4.1 Substation Hydrology Results

Existing Conditions

The proposed Suncrest substation is located approximately six miles east of the unincorporated community of Alpine within San Diego County, California and impacts four undeveloped drainage basins. Basin 100 has a contributing area of 57.5 acres, Basin 200 has a contributing area of 89.1 acres, Basin 300 is 43.7 acres and Basin 400 is 6.0 acres. All four basins have natural drainage channels which drain in the southwest direction.

Proposed Conditions

Basin 100 is located on the north side of the proposed substation. Runoff from the proposed grading and the proposed access road within Basin 100 is routed through a series of storm drain pipes which is ultimately captured by a detention basin within Basin 100. The runoff from the remaining area of Basin 100, which will not be disturbed by the substation, drains naturally or is routed through a “bypass” storm drain system which discharges at a point downstream of the proposed detention basin.

Basin 200 is located on the southeast side of the proposed substation. Runoff from the proposed grading and substation pad within Basin 200 is also routed through a storm drain system which drains to a detention basin within Basin 200. The area of Basin 200 which is not affected by the proposed substation, will drain naturally.

Basin 300 is located at the center of the proposed substation, and similar to Basin 200, all runoff from the proposed site within Basin 300 is routed through a storm drain system into a detention basin within Basin 300. Runoff from the remaining portion of Basin 300 remains undisturbed.

Basin 400 is on the southwest side of the proposed substation and is minimally affected by grading for a proposed pad for a transmission tower. Basin 400 does not include a storm drain system and the entire basin drains naturally.

Table 1 summarizes the peak 100-year flows for existing and proposed conditions. The proposed flows in the table reflect post-detention conditions that are necessary to mitigate for increases in peak flows that discharge from the site.

Table 1: Basin Summary; 100-Year Discharge (Q) (cfs), and Area (acres)								
Site Condition	Basin 100		Basin 200		Basin 300		Basin 400	
	Q	Area	Q	Area	Q	Area	Q	Area
Existing	85.2	57.5	165.7	89.1	71.7	43.7	11.7	6.0
Proposed	84.7	63.2	165.7	91.0	63.1	41.3	11.3	5.8

4.2 Access Road Hydrologic Results

In addition to the substation site, a 2.8-mile road provides access to the proposed substation. This access road begins east of the proposed substation at Japatul Valley Road, and connects at the northeast corner of the proposed substation, impacting several major drainage basins that drain towards the proposed access road. Culverts will be installed at several locations along the access road to allow existing channels to drain under the proposed access road. Approximately 0.4 miles of this access road is part of Basin 100 and is included in Basin 100's hydrologic analysis. The Culvert and Rip Rap Sizing Summary Table located in Appendix B of this report provides the flows and tributary areas to size the proposed culvert crossings. Section 3.3 of this report explains the methodology used to determine the quantity of flows. Appendix B has the rational method calculations for the computed drainage basins (Basins 500, 600, 700, 800, 900, 1000). The Proposed Access Road Hydrology Map is located in Appendix C.

4.3 Substation Hydraulic Results

Hydrologic Basins 100, 200, and 300 each contain detention basins that capture runoff from the proposed site to attenuate downstream flow. Detention basins 100 and 300 have rectangular risers and detention basin 200 has a circular riser that discharge flow in a manner which maintains the natural drainage path. Additionally, each detention basin contains an emergency riser that is designed to help drain the basin in the event of failure of the primary riser outlet system.

The detention basin within Basin 100 is located near the northwest side of the proposed substation and captures runoff from approximately 22.5 acres. The detention basin located within Basin 200 is located on the southeast side of the substation and captures runoff from 2.7 acres of the proposed site. Finally, the third detention basin, located within Basin 300, is situated on the southwest side of the proposed site and has 30.0 acres of tributary drainage area.

The detention basin analyses are primarily comprised of three data types; the inflow hydrograph, stage-storage relationship of the basin, and the outflow rating curve. Using this input data, storage calculations were performed using Bentley Haestad PondPack, Version 10.0 (PondPack).

Detention Basin Analyses

Inflow Hydrograph

Peak discharge volumes and time of concentrations for runoff draining into each detention basin were calculated using AES (see Section 4.1). Inflow hydrographs were generated for each detention basin using the County of San Diego's RatHydro program. The RatHydro program requires the following input (obtained from Nolte's Rational Method analyses in AES) in order to generate a Rational Method Hydrograph:

- Rational Method Time of Concentration (min.)
- 6-Hour Rainfall (inches)
- Basin Area (acres)

- Rational Method Runoff Coefficient
- Peak Discharge (cfs)

Stage-Storage Relationship

Volumes are represented by specifying an area (acres) for each contour of the proposed detention basin. PondPack interpolates linearly between these contours to compute the available storage volume.

Outflow Rating Curve

As previously mentioned, the risers within detention basins 100 and 300 are rectangular and the riser within detention basin 200 is circular. A majority of the flow enters the risers via weir flow or orifice flow through the top. However, orifices along the side of the risers also serve to help drain the basin under low flow conditions. The risers in detention basin 100 and 300 have three six-inch orifices located at the invert of the riser and 1.5 and 2.5 feet above the invert. Detention basin 200 has a 2-inch orifice at the riser invert and a 6-inch orifice 2.5 feet above the invert. Table 3 summarizes the primary riser configurations for the proposed detention basins.

Table 3: Riser Configurations

Riser Data	Detention Basin 100	Detention Basin 200	Detention Basin 300
Top Dimensions	5' x 4'	2' Diameter	5' x 4'
Height (ft)	5.5	5.5	5.5
Perforation Area/Row(ft ²)	0.20	0.02/0.20	0.20

Both weir and orifice equations were used to calculate flow through the top of the riser. The more conservative discharge value (lowest value) was used in the rating curves.

The orifice equation was used to determine the flow through the perforations at given water surface elevations (WSEL) and for flow through the top of the riser for circumstances when the WSEL exceeds the top of the riser:

Orifice Equation:
$$Q = Ca\sqrt{2gh}$$

where:

C Orifice Coefficient, 0.6

a Total open area per row of perforations or through top of riser (ft²)

g Acceleration due to gravity (32.2 ft/sec²)

h Total head above perforation row or top of riser (ft)

The weir equation is used for flow through the top of riser when the head is above the riser.

Weir Equation: $Q=CLH^{3/2}$

where:

C Weir Coefficient, varies depending upon H/R ratio

L Perimeter at top of riser (ft)

H Total head above top of riser (ft)

Table 4 summarizes the outflow (assuming 50% blockage), maximum ponded WSEL, and volume of each respective detention basin at the 100-year ponded WSEL. See Appendix B for summary input and output from the detention basin analyses.

Table 4: Detention Basin Summary

PondPack Results	Detention Basin 100	Detention Basin 200	Detention Basin 300
100-Year Outflow (cfs)	18.20	0.94	47.66
100-Year Peak WSEL (ft)	3016.12	3048.39	2965.78
Volume (ac-ft)	2.340	0.204	2.226
Volume (ft ³)	101,930	8,886	96,965

Culvert Calculations

It was also necessary to verify that the outlet pipe has greater hydraulic capacity than the riser. This was accomplished by analyzing the outlet pipe as a culvert and comparing the calculated headwater elevation of the culvert to the peak WSEL of the detention basin (determined by the detention analyses). All culverts were analyzed as circular HDPE pipes with a roughness coefficient of 0.012. Table 5 summarizes the culvert diameters and calculated headwater elevations and illustrates that the outlet pipes for each respective detention basin have adequate capacity to convey the design discharge.

Table 5: Culvert Calculations

Culvert Results	Detention Basin 100	Detention Basin 200	Detention Basin 300
Culvert Diameter (ft)	3	1.5	3
Headwater Elevation (ft)	3006.15	3038.53	2957.12

4.4 Access Road Hydraulic Results

Culvert Calculations

CulvertMaster was used to determine adequate culvert sizes for all but two culvert crossings under the proposed access road. Input criteria for culvert analyses consisted of the discharge of

the culvert (peak 100-year flows of each basin), maximum allowable headwater (the edge of the proposed access road at the culvert crossing which would not allow water to overtop the road), and the tailwater elevation (determined as previously mentioned in Substation Hydraulics).

Basin 600 has culvert crossings at two locations; stations 115+38 and 120+15. At station 120+15, there is a natural channel draining to the northwest that crosses the proposed access road. After crossing the access road, the channel changes direction to the west, where it confluences with an additional channel within Basin 600. Downstream of this confluence, the channel drains in the southwest direction to cross the proposed access road a second time at station 115+38. HEC-RAS (Version 4.0) models were created for each of these culvert crossings to determine adequate culvert sizes. HEC-RAS models for Basin 600 included cross sections at a minimum of 100 feet upstream and downstream from the culvert at approximately 50-foot intervals, a coefficient of roughness of 0.05, and the peak 100-year discharge values determined by Rational Method analyses. The Culvert and Rip Rap Sizing Summary Table located in Appendix B lists the culvert sizes for each proposed crossing.

Rip Rap Calculations

Rip Rap Energy Dissipators will be placed at each culvert outlet per San Diego Regional Standard Drawing Number D-40. The purpose is to reduce the chances of runoff eroding the soil at each outlet, which can cause the culvert to shift over time. Rip rap calculations for each culvert are included in Appendix B in the Culvert and Rip Rap Sizing Summary Table.

4.5 Hydromodification Results

The San Diego County Municipal Permit (Order No. R9-2007-0001) was updated in January 2007 to include hydromodification criteria. While the County of San Diego is in the process of developing a comprehensive plan, Interim Hydromodification Criteria (IHC) have been established for those projects disturbing 50 acres or more. The purpose of IHC is to prevent development-related changes in storm water runoff from causing, or further accelerating, stream channel erosion or other adverse impacts to beneficial stream uses. Hydromodification aims to reduce erosion caused, not by the 100-year storm event that is typically used as the basis of design, but rather the steady flows of the more common smaller storms. IHC requires the analysis of historical rainfall data, usually spanning 35 years or more. By utilizing historical rain gauge data from a minimum of 35 years in the region, the hydromodification goal is to maintain the flow between 20 percent of the 5-year flow event and the 10-year flow event to within a ten percent interval from the pre-project to the mitigated land scenario. Nolte used a continuous simulation modeling software known as San Diego Hydrology Model (SDHM) created by Clear Creek Solutions, Inc. in order to model the Suncrest Substation. The SDHM was used to support the sizing of the three detention basins and their outlet structures that will be part of the proposed development. The SDHM output file can be found in Appendix B in the Hydraulic Calculations.

5. REFERENCES

AES Advanced Engineering Software, Rational Method Hydrology System Model, April 2008.

Haestad Methods, Inc., StormCAD v8i, November 2008.

Haestad Methods, Inc., FlowMaster, August 2008.

Haestad Methods, Inc., CulvertMaster v3.2, August 2008.

Haestad Methods, Inc., PondPack, v10.0, August 2008.

County of San Diego and Rick Engineering Company, RatHydro Program, January 2001.

U.S. Army Corps of Engineers, HEC-RAS 4.0.0, March 2008.

County of San Diego, Department of Public Works, Flood Control Section, San Diego County Hydrology Manual, June 2003

County of San Diego, Department of Public Works, Flood Control Section, San Diego County Drainage Design Manual, May 2005

Clear Creek Solutions, Inc., San Diego Hydrology Model, January 2008.

6. DECLARATION OF RESPONSIBLE CHARGE

I, hereby declare that I am the civil engineer of work for this project, that I have exercised responsible charge over the design of the project as defined in section 6703 of the Business and Professions Code, and that the design is consistent with current design.

I understand that the check of project drawings and specifications by the County of San Diego is confined to a review only and does not relieve me, as engineer of work, of my responsibilities for project design.

Scott Berkebile
RCE 66153, Exp. 06/30/10

Date

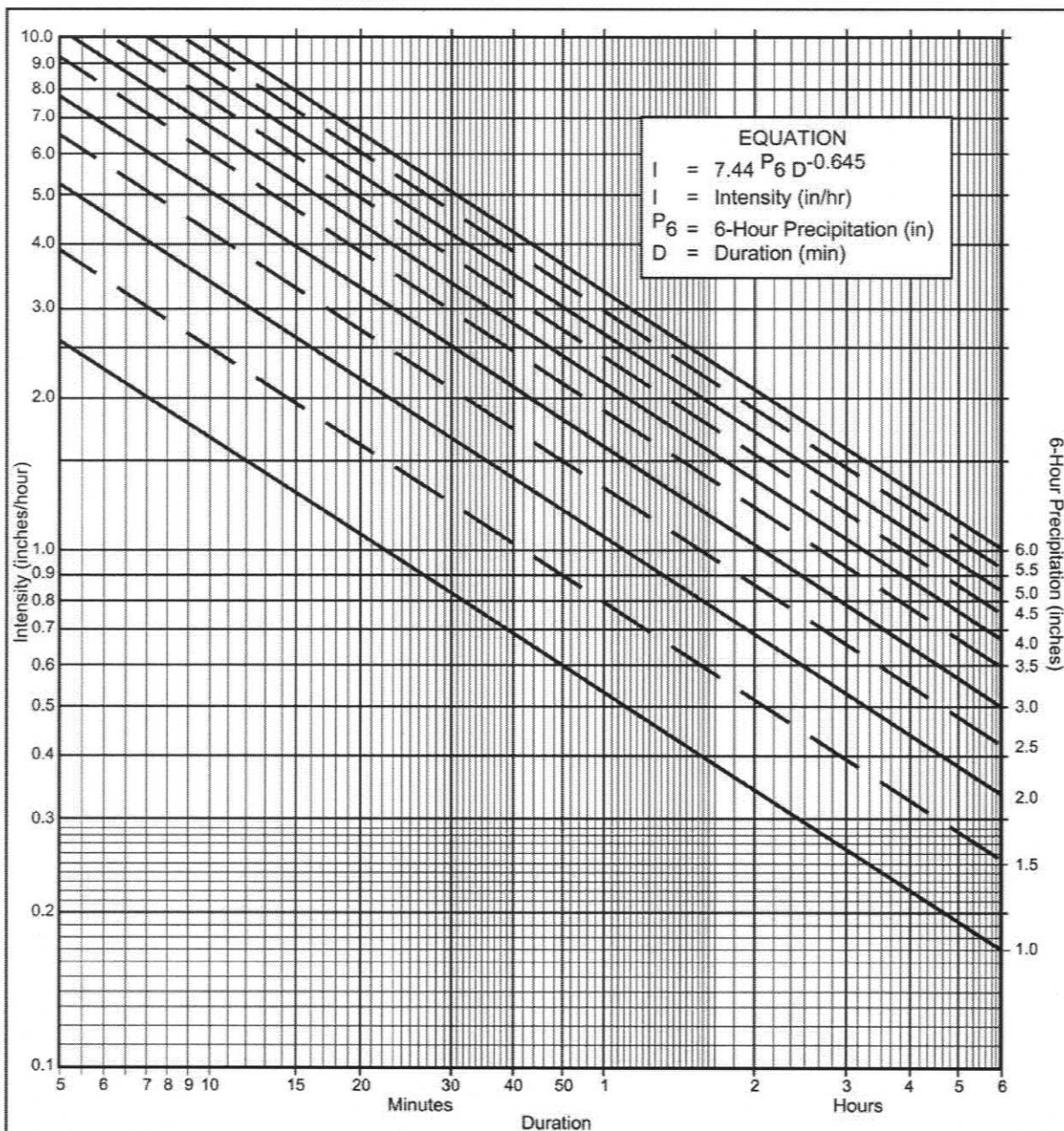
APPENDIX A: RATIONAL METHOD SUPPORTING TABLES AND FIGURES

Rainfall Intensity Duration Frequency Curves

Precipitation Map – 100-Year 6-Hour Isopluvials

Soil Hydrologic Groups Map

Runoff Coefficients



Directions for Application:

- (1) From precipitation maps determine 6 hr and 24 hr amounts for the selected frequency. These maps are included in the County Hydrology Manual (10, 50, and 100 yr maps included in the Design and Procedure Manual).
- (2) Adjust 6 hr precipitation (if necessary) so that it is within the range of 45% to 65% of the 24 hr precipitation (not applicable to Desert).
- (3) Plot 6 hr precipitation on the right side of the chart.
- (4) Draw a line through the point parallel to the plotted lines.
- (5) This line is the intensity-duration curve for the location being analyzed.

Application Form:

- (a) Selected frequency _____ year
- (b) $P_6 =$ _____ in., $P_{24} =$ _____, $\frac{P_6}{P_{24}} =$ _____ %⁽²⁾
- (c) Adjusted $P_6^{(2)} =$ _____ in.
- (d) $t_x =$ _____ min.
- (e) $I =$ _____ in./hr.

Note: This chart replaces the Intensity-Duration-Frequency curves used since 1965.

P6	1	1.5	2	2.5	3	3.5	4	4.5	5	5.5	6
Duration	I	I	I	I	I	I	I	I	I	I	I
5	2.63	3.95	5.27	6.59	7.90	9.22	10.54	11.86	13.17	14.49	15.81
7	2.12	3.18	4.24	5.30	6.36	7.42	8.48	9.54	10.60	11.66	12.72
10	1.68	2.53	3.37	4.21	5.05	5.90	6.74	7.58	8.42	9.27	10.11
15	1.30	1.95	2.59	3.24	3.89	4.54	5.19	5.84	6.49	7.13	7.78
20	1.08	1.62	2.15	2.69	3.23	3.77	4.31	4.85	5.39	5.93	6.46
25	0.93	1.40	1.87	2.33	2.80	3.27	3.73	4.20	4.67	5.13	5.60
30	0.83	1.24	1.66	2.07	2.49	2.90	3.32	3.73	4.15	4.56	4.98
40	0.69	1.03	1.38	1.72	2.07	2.41	2.76	3.10	3.45	3.79	4.13
50	0.60	0.90	1.19	1.49	1.79	2.09	2.39	2.69	2.98	3.28	3.58
60	0.53	0.80	1.06	1.33	1.59	1.86	2.12	2.39	2.65	2.92	3.18
90	0.41	0.61	0.82	1.02	1.23	1.43	1.63	1.84	2.04	2.25	2.45
120	0.34	0.51	0.68	0.85	1.02	1.19	1.36	1.53	1.70	1.87	2.04
150	0.29	0.44	0.59	0.73	0.88	1.03	1.18	1.32	1.47	1.62	1.76
180	0.26	0.39	0.52	0.65	0.78	0.91	1.04	1.18	1.31	1.44	1.57
240	0.22	0.33	0.43	0.54	0.65	0.76	0.87	0.98	1.08	1.19	1.30
300	0.19	0.28	0.38	0.47	0.56	0.66	0.75	0.85	0.94	1.03	1.13
360	0.17	0.25	0.33	0.42	0.50	0.58	0.67	0.75	0.84	0.92	1.00

Intensity-Duration Design Chart - Template

FIGURE

3-1

**Table 3-1
RUNOFF COEFFICIENTS FOR URBAN AREAS**

Land Use		Runoff Coefficient "C"				
NRCS Elements	County Elements	% IMPER.	Soil Type			
			A	B	C	D
Undisturbed Natural Terrain (Natural)	Permanent Open Space	0*	0.20	0.25	0.30	0.35
Low Density Residential (LDR)	Residential, 1.0 DU/A or less	10	0.27	0.32	0.36	0.41
Low Density Residential (LDR)	Residential, 2.0 DU/A or less	20	0.34	0.38	0.42	0.46
Low Density Residential (LDR)	Residential, 2.9 DU/A or less	25	0.38	0.41	0.45	0.49
Medium Density Residential (MDR)	Residential, 4.3 DU/A or less	30	0.41	0.45	0.48	0.52
Medium Density Residential (MDR)	Residential, 7.3 DU/A or less	40	0.48	0.51	0.54	0.57
Medium Density Residential (MDR)	Residential, 10.9 DU/A or less	45	0.52	0.54	0.57	0.60
Medium Density Residential (MDR)	Residential, 14.5 DU/A or less	50	0.55	0.58	0.60	0.63
High Density Residential (HDR)	Residential, 24.0 DU/A or less	65	0.66	0.67	0.69	0.71
High Density Residential (HDR)	Residential, 43.0 DU/A or less	80	0.76	0.77	0.78	0.79
Commercial/Industrial (N. Com)	Neighborhood Commercial	80	0.76	0.77	0.78	0.79
Commercial/Industrial (G. Com)	General Commercial	85	0.80	0.80	0.81	0.82
Commercial/Industrial (O.P. Com)	Office Professional/Commercial	90	0.83	0.84	0.84	0.85
Commercial/Industrial (Limited I.)	Limited Industrial	90	0.83	0.84	0.84	0.85
Commercial/Industrial (General I.)	General Industrial	95	0.87	0.87	0.87	0.87

*The values associated with 0% impervious may be used for direct calculation of the runoff coefficient as described in Section 3.1.2 (representing the pervious runoff coefficient, C_p , for the soil type), or for areas that will remain undisturbed in perpetuity. Justification must be given that the area will remain natural forever (e.g., the area is located in Cleveland National Forest).

DU/A = dwelling units per acre

NRCS = National Resources Conservation Service

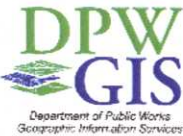
County of San Diego Hydrology Manual



Soil Hydrologic Groups

Legend

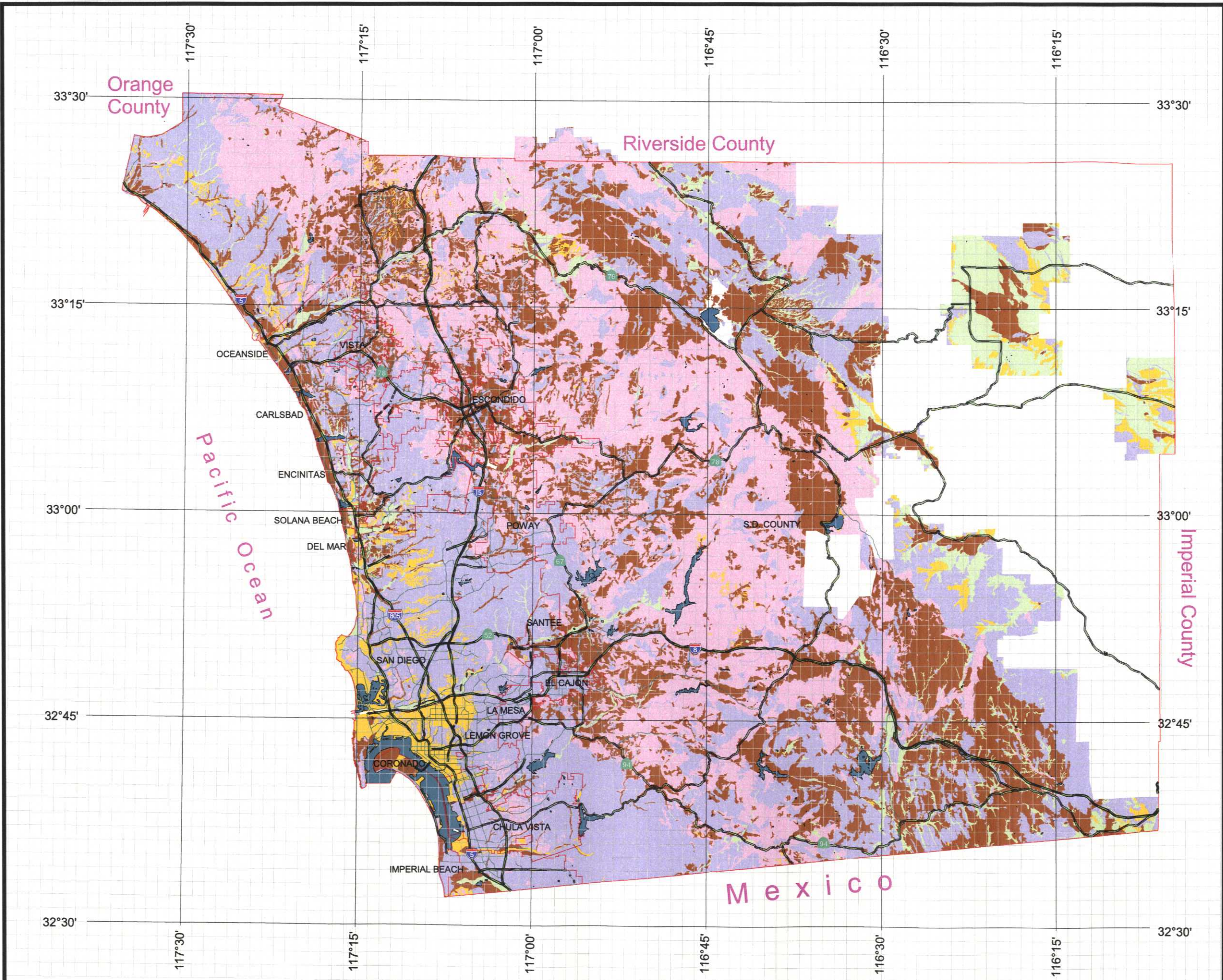
Soil Groups	
	Group A
	Group B
	Group C
	Group D
	Undetermined
	Data Unavailable



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100 Year Rainfall Event - 6 Hours

----- Isopluvial (inches)



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3 0 3 Miles

County of San Diego Hydrology Manual



Rainfall Isopluvials

100 Year Rainfall Event - 24 Hours

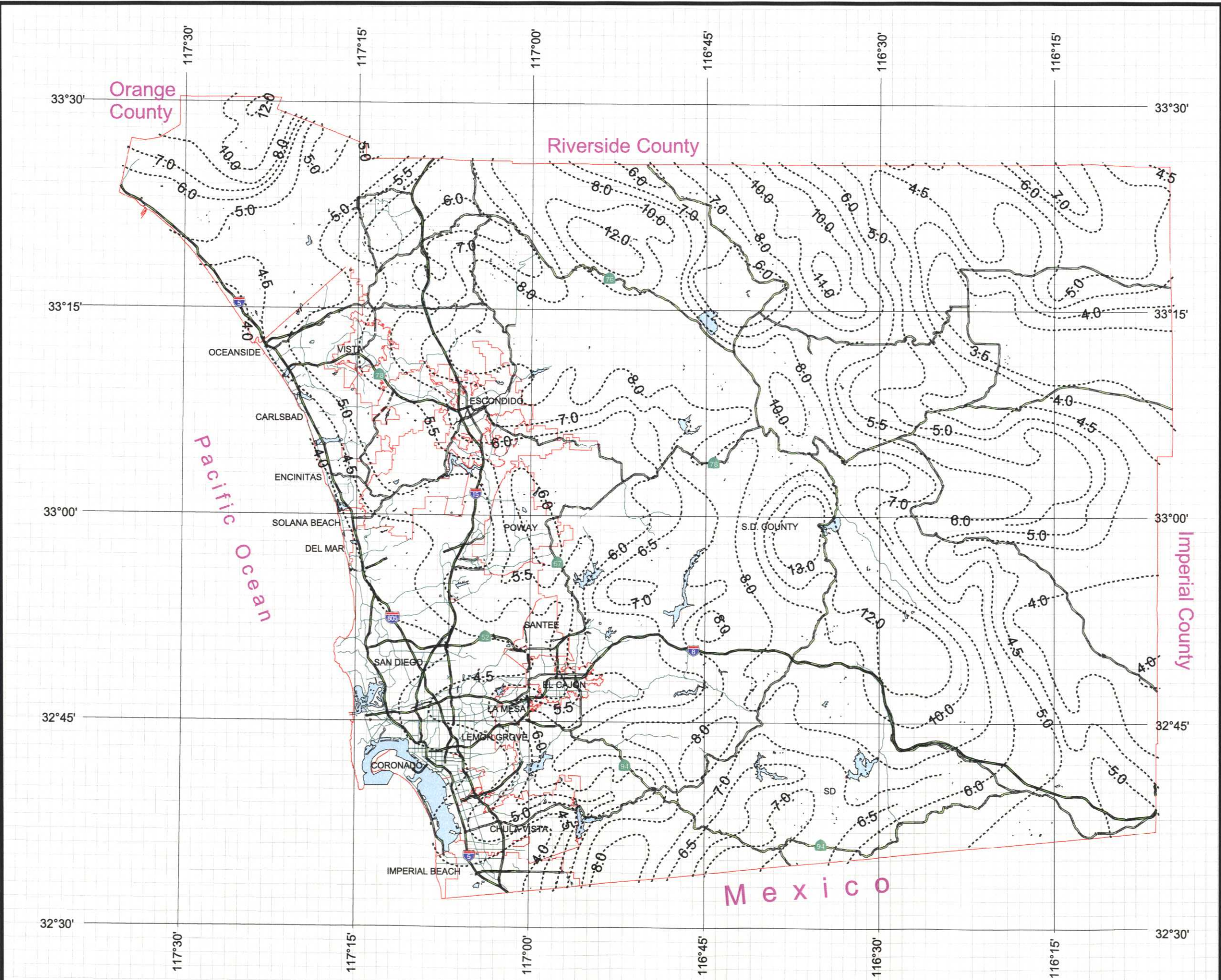
----- Isopluvial (inches)



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APPENDIX B: CALCULATIONS

Weighted C Value Calculation Excel Spreadsheet

AES Rational Method Calculations

Hydraulic Calculations

HEC-RAS Calculations

Culvert and Rip Rap Sizing Summary Table

WEIGHTED C VALUE CALCULATION

Weighted C-Value Calculations

	<u>Description</u>	<u>C-Value</u>
A	Paved Street	0.95
B	Proposed Slope/Newly Graded Area	0.40
C	Substation Pad (Class II Base)	0.45
D	Natural (Undisturbed) Terrain	0.35

Basin 100							
Nodes		Total Sub-Area (Acres)	A (Acres)	B (Acres)	C (Acres)	D (Acres)	C _{Average}
U/S	D/S						
100	101	0.10				0.1	0.35
101	102	0.88				0.88	0.35
102	103	0.86				0.86	0.35
103	104	1.83				1.83	0.35
105	105	0.26	0.09			0.17	0.56
151	151	0.88		0.22		0.66	0.36
156	151	3.01		0.67		2.34	0.36
160	161	0.07	0.07				0.95
161	162	0.06	0.06				0.95
162	163	0.19	0.19				0.95
163	164	0.40	0.40				0.95
164	165	0.08	0.08				0.95
165	167	0.32	0.25	0.07			0.83
166	166	1.59		1.59			0.40
167	168	N/A					N/A
170	171	0.04		0.04			0.40
171	172	0.10			0.10		0.45
172	173	0.34			0.34		0.45
173	174	N/A					N/A
175	174	0.51		0.05	0.46		0.45
176	176	0.24		0.01	0.23		0.45
176	169	N/A					N/A
169	178	0.69	0.34	0.17	0.18		0.68
178	179	0.84	0.12	0.23	0.49		0.51
179	180	0.61	0.19		0.42		0.61
180.5	180	1.00	0.11		0.89		0.51
180	181	0.62	0.11	0.02	0.49		0.54
181.5	181	0.97			0.97		0.45
181	182	0.33	0.10		0.23		0.60
183.5	183	0.57	0.11		0.46		0.55
183	184	N/A					N/A

	<u>Description</u>	<u>C-Value</u>
A	Paved Street	0.95
B	Proposed Slope/Newly Graded Area	0.40
C	Substation Pad (Class II Base)	0.45
D	Natural (Undisturbed) Terrain	0.35

Basin 100							
Nodes		Total Sub-Area (Acres)	A (Acres)	B (Acres)	C (Acres)	D (Acres)	C _{Average}
U/S	D/S						
185	186	0.11			0.11		0.45
186	187	0.12			0.12		0.45
187	188	0.77			0.77		0.45
188	189	0.56	0.35		0.21		0.76
189	190	0.85	0.25		0.60		0.60
190.5	190	0.79			0.79		0.45
190	191	1.51	0.13		1.38		0.49
191	192	1.51	0.21		1.30		0.52
192	193	1.46	0.12		1.34		0.49
193	184	N/A					N/A
194	195	0.15	0.03	0.05	0.07		0.53
195	196	0.23	0.04		0.20		0.53
196	197	0.36	0.07	0.29			0.51
197	198	N/A					N/A
198	199	N/A					N/A
199.1	199.0	0.86	0.28		0.58		0.61
199	184.0	N/A					N/A
184	199.2	N/A					N/A
199.2	199.3	N/A					N/A
199.3	199.4	N/A					N/A
199.4	199.4	1.37		1.37			0.40
199.5	199.5	0.34		0.34			0.40

	<u>Description</u>	<u>C-Value</u>
A	Paved Street	0.95
B	Proposed Slope/Newly Graded Area	0.40
C	Substation Pad (Class II Base)	0.45
D	Natural (Undisturbed) Terrain	0.35

Basin 200							
Nodes		Total Sub-Area (Acres)	A (Acres)	B (Acres)	C (Acres)	D (Acres)	C _{Average}
U/S	D/S						
208	210	0.11			0.11		0.45
210	212	0.17			0.17		0.45
212	218	0.77		0.23	0.54		0.44
218	220	0.92		0.12	0.80		0.44
224	226	0.05		0.05			0.40
226	228	0.09		0.02	0.07		0.44
228	230	0.22			0.22		0.45
232	234	0.36	0.03	0.33			0.45

	Description	C-Value
A	Paved Street	0.95
B	Proposed Slope/Newly Graded Area	0.40
C	Substation Pad (Class II Base)	0.45
D	Natural (Undisturbed) Terrain	0.35

Basin 300							
Nodes		Total Sub-Area (Acres)	A (Acres)	B (Acres)	C (Acres)	D (Acres)	C _{Average}
U/S	D/S						
300	301	0.11		0.11			0.40
301	302	0.04		0.02	0.02		0.43
302	303	0.38		0.30	0.08		0.41
303	304	0.79		0.06	0.73		0.45
304	305	N/A					N/A
306	305	0.49		0.49			0.40
305	307	0.78	0.05	0.23	0.5		0.47
307	308	0.95	0.20		0.75		0.56
308.5	308	0.68	0.14		0.54		0.55
308	309	1.24	0.21		1.03		0.53
310	311	0.11			0.11		0.45
311	312	0.12			0.12		0.45
312	313	1.00	0.11		0.89		0.51
313	309	N/A					N/A
309	314	1.23	0.21		1.02		0.54
315	316	0.11			0.11		0.45
316	317	0.13			0.13		0.45
317	318	0.98			0.98		0.45
318	319	1.22			1.22		0.45
320	319	1.17			1.17		0.45
319	314	N/A					N/A
322	323	0.11			0.11		0.45
323	324	0.12			0.12		0.45
324	325	1.00	0.11		0.89		0.51
325	314	N/A					N/A
314	326	0.76	0.20	0.21	0.35		0.57
328	329	0.08		0.08			0.40
329	330	0.14		0.14			0.40
330	331	N/A					N/A
331	326	N/A					N/A
326	332	0.78	0.11	0.20	0.47		0.51
332	333	N/A					N/A
333	335	0.55	0.10	0.03	0.42		0.54
335	336	0.46	0.09		0.37		0.55
336	337	0.46	0.08		0.38		0.54
345	346	0.11			0.11		0.45
346	347	0.21			0.21		0.45

	Description	C-Value
A	Paved Street	0.95
B	Proposed Slope/Newly Graded Area	0.40
C	Substation Pad (Class II Base)	0.45
D	Natural (Undisturbed) Terrain	0.35

Basin 300							
Nodes		Total Sub-Area (Acres)	A (Acres)	B (Acres)	C (Acres)	D (Acres)	C _{Average}
U/S	D/S						
347	348	1.19			1.19		0.45
348	349	1.56			1.56		0.45
349	350	N/A					N/A
351	350	0.70		0.02	0.68		0.45
350	354	N/A					N/A
355	354	0.73	0.13		0.60		0.54
358	359	0.10			0.10		0.45
359	360	0.18	0.02		0.16		0.51
360	361	0.81	0.09		0.72		0.51
361	362	0.99	0.09		0.90		0.50
362	354	N/A					N/A
355	354	0.73	0.12		0.61		0.53
354	363	N/A					N/A
364	363	0.73	0.19		0.54		0.58
365	367	0.10			0.10		0.45
367	368	0.18	0.02		0.16		0.51
368	369	0.81	0.08		0.73		0.50
369	370	0.99	0.09		0.9		0.50
370	363	N/A					N/A
363	337	N/A					N/A
337	371	N/A					N/A
371	372	N/A					N/A
372	373	0.78		0.78			0.40
373	374	1.17		1.17			0.40
374	376	N/A					N/A
376	378	2.14		2.14			0.40

AES RATIONAL METHOD CALCULATIONS

 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
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 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

Nolte Associates, Inc.
 15070 Avenue of Science
 Suite 100
 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * 11-11-2009 *
 * EXISTING HYDROLOGY BASIN 100 *
 * *

FILE NAME: EX100.DAT
 TIME/DATE OF STUDY: 10:59 11/11/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3395.00
 DOWNSTREAM ELEVATION(FEET) = 3357.00
 ELEVATION DIFFERENCE(FEET) = 38.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.29
 TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.29

 FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3357.00 DOWNSTREAM(FEET) = 3276.00

```

                                EX100.RES
CHANNEL LENGTH THRU SUBAREA(FEET) = 198.00 CHANNEL SLOPE = 0.4091
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2151 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.29
FLOW VELOCITY(FEET/SEC) = 2.60 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.27 Tc(MIN.) = 7.54
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 102.00 = 298.00 FEET.

*****
FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.279
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.88 SUBAREA RUNOFF(CFS) = 2.24
TOTAL AREA(ACRES) = 1.0 TOTAL RUNOFF(CFS) = 2.50
TC(MIN.) = 7.54

*****
FLOW PROCESS FROM NODE 102.00 TO NODE 103.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3276.00 DOWNSTREAM(FEET) = 3169.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 400.00 CHANNEL SLOPE = 0.2675
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1858 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 2.50
FLOW VELOCITY(FEET/SEC) = 3.27 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 2.04 Tc(MIN.) = 9.57
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 103.00 = 698.00 FEET.

*****
FLOW PROCESS FROM NODE 102.00 TO NODE 103.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.239
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.86 SUBAREA RUNOFF(CFS) = 1.88
TOTAL AREA(ACRES) = 1.8 TOTAL RUNOFF(CFS) = 4.02
TC(MIN.) = 9.57

*****
FLOW PROCESS FROM NODE 103.00 TO NODE 104.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3169.00 DOWNSTREAM(FEET) = 3136.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 350.00 CHANNEL SLOPE = 0.0943
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 5.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.547
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 7.11
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.05
AVERAGE FLOW DEPTH(FEET) = 0.21 TRAVEL TIME(MIN.) = 1.91
Tc(MIN.) = 11.49
SUBAREA AREA(ACRES) = 3.18 SUBAREA RUNOFF(CFS) = 6.17
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 5.0 PEAK FLOW RATE(CFS) = 9.75

```

EX100.RES

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.25 FLOW VELOCITY(FEET/SEC.) = 3.40
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 104.00 = 1048.00 FEET.

 FLOW PROCESS FROM NODE 104.00 TO NODE 105.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3136.00	DOWNSTREAM(FEET) =	3116.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	259.00	CHANNEL SLOPE =	0.0772
CHANNEL BASE(FEET) =	10.00	"Z" FACTOR =	5.000
MANNING'S FACTOR =	0.050	MAXIMUM DEPTH(FEET) =	1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	5.210		
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500		
SOIL CLASSIFICATION IS "D"			
S.C.S. CURVE NUMBER (AMC II) =	88		
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =	14.93		
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =	3.68		
AVERAGE FLOW DEPTH(FEET) =	0.35	TRAVEL TIME(MIN.) =	1.17
Tc(MIN.) =	12.66		
SUBAREA AREA(ACRES) =	5.68	SUBAREA RUNOFF(CFS) =	10.36
AREA-AVERAGE RUNOFF COEFFICIENT =	0.350		
TOTAL AREA(ACRES) =	10.7	PEAK FLOW RATE(CFS) =	19.51

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.40 FLOW VELOCITY(FEET/SEC.) = 4.05
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 105.00 = 1307.00 FEET.

 FLOW PROCESS FROM NODE 105.00 TO NODE 106.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3116.00	DOWNSTREAM(FEET) =	3034.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	864.00	CHANNEL SLOPE =	0.0949
CHANNEL BASE(FEET) =	10.00	"Z" FACTOR =	5.000
MANNING'S FACTOR =	0.050	MAXIMUM DEPTH(FEET) =	1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	4.606		
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500		
SOIL CLASSIFICATION IS "D"			
S.C.S. CURVE NUMBER (AMC II) =	88		
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =	38.75		
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =	5.41		
AVERAGE FLOW DEPTH(FEET) =	0.56	TRAVEL TIME(MIN.) =	2.66
Tc(MIN.) =	15.32		
SUBAREA AREA(ACRES) =	23.80	SUBAREA RUNOFF(CFS) =	38.37
AREA-AVERAGE RUNOFF COEFFICIENT =	0.350		
TOTAL AREA(ACRES) =	34.5	PEAK FLOW RATE(CFS) =	55.62

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.68 FLOW VELOCITY(FEET/SEC.) = 6.06
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 106.00 = 2171.00 FEET.

 FLOW PROCESS FROM NODE 106.00 TO NODE 107.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3034.00	DOWNSTREAM(FEET) =	2962.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	822.00	CHANNEL SLOPE =	0.0876
CHANNEL BASE(FEET) =	10.00	"Z" FACTOR =	5.000
MANNING'S FACTOR =	0.050	MAXIMUM DEPTH(FEET) =	1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	4.235		
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500		
SOIL CLASSIFICATION IS "D"			
S.C.S. CURVE NUMBER (AMC II) =	88		
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =	72.64		
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =	6.43		
AVERAGE FLOW DEPTH(FEET) =	0.81	TRAVEL TIME(MIN.) =	2.13
Tc(MIN.) =	17.45		

EX100.RES
SUBAREA AREA(ACRES) = 22.97 SUBAREA RUNOFF(CFS) = 34.05
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 57.5 PEAK FLOW RATE(CFS) = 85.18

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.88 FLOW VELOCITY(FEET/SEC.) = 6.72
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 107.00 = 2993.00 FEET.

=====

END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 57.5 TC(MIN.) = 17.45
PEAK FLOW RATE(CFS) = 85.18

=====

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END OF RATIONAL METHOD ANALYSIS

□

 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
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 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

Nolte Associates, Inc.
 15070 Avenue of Science
 Suite 100
 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * 11-11-2009 *
 * EXISTING HYDROLOGY BASIN 200 *
 * *

FILE NAME: EX200.DAT
 TIME/DATE OF STUDY: 10:56 11/11/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 200.00 TO NODE 201.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3277.00
 DOWNSTREAM ELEVATION(FEET) = 3262.00
 ELEVATION DIFFERENCE(FEET) = 15.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.72
 TOTAL AREA(ACRES) = 0.25 TOTAL RUNOFF(CFS) = 0.72

 FLOW PROCESS FROM NODE 201.00 TO NODE 202.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3262.00 DOWNSTREAM(FEET) = 3207.00

```

                                EX200.RES
CHANNEL LENGTH THRU SUBAREA(FEET) = 148.00 CHANNEL SLOPE = 0.3716
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2090 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.72
FLOW VELOCITY(FEET/SEC) = 2.56 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.96 Tc(MIN.) = 7.23
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 202.00 = 248.00 FEET.

*****
FLOW PROCESS FROM NODE 201.00 TO NODE 202.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.477
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.63 SUBAREA RUNOFF(CFS) = 1.65
TOTAL AREA(ACRES) = 0.9 TOTAL RUNOFF(CFS) = 2.30
TC(MIN.) = 7.23

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3207.00 DOWNSTREAM(FEET) = 3170.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 215.00 CHANNEL SLOPE = 0.1721
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1460 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 2.30
FLOW VELOCITY(FEET/SEC) = 2.83 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.27 Tc(MIN.) = 8.50
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 203.00 = 463.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.737
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 1.42 SUBAREA RUNOFF(CFS) = 3.35
TOTAL AREA(ACRES) = 2.3 TOTAL RUNOFF(CFS) = 5.42
TC(MIN.) = 8.50

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 204.00 IS CODE = 52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3170.00 DOWNSTREAM(FEET) = 3132.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 208.00 CHANNEL SLOPE = 0.1827
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 5.42
FLOW VELOCITY(FEET/SEC) = 6.80 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.51 Tc(MIN.) = 9.01
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 204.00 = 671.00 FEET.

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 204.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.488
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"

```

EX200.RES

S.C.S. CURVE NUMBER (AMC II) = 88
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
 SUBAREA AREA(ACRES) = 2.36 SUBAREA RUNOFF(CFS) = 5.36
 TOTAL AREA(ACRES) = 4.7 TOTAL RUNOFF(CFS) = 10.58
 TC(MIN.) = 9.01

 FLOW PROCESS FROM NODE 204.00 TO NODE 205.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3132.00	DOWNSTREAM(FEET) =	3050.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	335.00	CHANNEL SLOPE =	0.2448
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION			
CHANNEL FLOW THRU SUBAREA(CFS) =	10.58		
FLOW VELOCITY(FEET/SEC) =	8.02	(PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)	
TRAVEL TIME(MIN.) =	0.70	Tc(MIN.) =	9.70
LONGEST FLOWPATH FROM NODE	200.00	TO NODE	205.00 = 1006.00 FEET.

 FLOW PROCESS FROM NODE 204.00 TO NODE 205.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	6.184
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500
SOIL CLASSIFICATION IS	"D"
S.C.S. CURVE NUMBER (AMC II) =	88
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3500
SUBAREA AREA(ACRES) =	10.68 SUBAREA RUNOFF(CFS) = 23.12
TOTAL AREA(ACRES) =	15.3 TOTAL RUNOFF(CFS) = 33.20
TC(MIN.) =	9.70

 FLOW PROCESS FROM NODE 205.00 TO NODE 206.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3050.00	DOWNSTREAM(FEET) =	3002.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	431.00	CHANNEL SLOPE =	0.1114
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION			
CHANNEL FLOW THRU SUBAREA(CFS) =	33.20		
FLOW VELOCITY(FEET/SEC) =	10.89	(PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)	
TRAVEL TIME(MIN.) =	0.66	Tc(MIN.) =	10.36
LONGEST FLOWPATH FROM NODE	200.00	TO NODE	206.00 = 1437.00 FEET.

 FLOW PROCESS FROM NODE 205.00 TO NODE 206.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	5.927
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500
SOIL CLASSIFICATION IS	"D"
S.C.S. CURVE NUMBER (AMC II) =	88
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3500
SUBAREA AREA(ACRES) =	26.24 SUBAREA RUNOFF(CFS) = 54.44
TOTAL AREA(ACRES) =	41.6 TOTAL RUNOFF(CFS) = 86.26
TC(MIN.) =	10.36

 FLOW PROCESS FROM NODE 206.00 TO NODE 207.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3002.00	DOWNSTREAM(FEET) =	2952.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	430.00	CHANNEL SLOPE =	0.1163
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION			
CHANNEL FLOW THRU SUBAREA(CFS) =	86.26		
FLOW VELOCITY(FEET/SEC) =	14.36	(PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)	
TRAVEL TIME(MIN.) =	0.50	Tc(MIN.) =	10.86

```

                                EX200.RES
LONGEST FLOWPATH FROM NODE    200.00 TO NODE    207.00 =    1867.00 FEET.
*****
FLOW PROCESS FROM NODE    206.00 TO NODE    207.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    5.750
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) =    14.07    SUBAREA RUNOFF(CFS) =    28.32
TOTAL AREA(ACRES) =    55.7    TOTAL RUNOFF(CFS) =    112.00
TC(MIN.) =    10.86
*****
FLOW PROCESS FROM NODE    207.00 TO NODE    208.00 IS CODE =    52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    2952.00    DOWNSTREAM(FEET) =    2540.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    1318.00    CHANNEL SLOPE =    0.3126
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) =    112.00
FLOW VELOCITY(FEET/SEC) =    15.53 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    1.41    Tc(MIN.) =    12.28
LONGEST FLOWPATH FROM NODE    200.00 TO NODE    208.00 =    3185.00 FEET.
*****
FLOW PROCESS FROM NODE    207.00 TO NODE    208.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    5.314
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) =    33.43    SUBAREA RUNOFF(CFS) =    62.17
TOTAL AREA(ACRES) =    89.1    TOTAL RUNOFF(CFS) =    165.67
TC(MIN.) =    12.28
=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES) =    89.1    TC(MIN.) =    12.28
PEAK FLOW RATE(CFS) =    165.67
=====
END OF RATIONAL METHOD ANALYSIS

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 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
 (c) Copyright 1982-2008 Advanced Engineering Software (aes)
 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

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 Suite 100
 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * EXISTING HYDROLOGY BASIN 300 *
 * *

FILE NAME: EX300.DAT
 TIME/DATE OF STUDY: 16:09 11/11/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 300.00 TO NODE 301.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3302.00
 DOWNSTREAM ELEVATION(FEET) = 3258.00
 ELEVATION DIFFERENCE(FEET) = 44.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.17
 TOTAL AREA(ACRES) = 0.06 TOTAL RUNOFF(CFS) = 0.17

 FLOW PROCESS FROM NODE 301.00 TO NODE 302.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3258.00 DOWNSTREAM(FEET) = 3169.00


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                                EX300.RES
CHANNEL LENGTH THRU SUBAREA(FEET) = 215.00 CHANNEL SLOPE = 0.4140
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2157 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.17
FLOW VELOCITY(FEET/SEC) = 2.60 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.38 Tc(MIN.) = 7.64
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 302.00 = 315.00 FEET.

*****
FLOW PROCESS FROM NODE 301.00 TO NODE 302.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.213
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.17 SUBAREA RUNOFF(CFS) = 0.43
TOTAL AREA(ACRES) = 0.2 TOTAL RUNOFF(CFS) = 0.58
TC(MIN.) = 7.64

*****
FLOW PROCESS FROM NODE 302.00 TO NODE 303.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3169.00 DOWNSTREAM(FEET) = 3138.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 101.00 CHANNEL SLOPE = 0.3069
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1967 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.58
FLOW VELOCITY(FEET/SEC) = 2.48 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.68 Tc(MIN.) = 8.32
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 303.00 = 416.00 FEET.

*****
FLOW PROCESS FROM NODE 302.00 TO NODE 303.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.828
RESIDENTIAL (24. DU/AC OR LESS) RUNOFF COEFFICIENT = .7100
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 92
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5743
SUBAREA AREA(ACRES) = 0.38 SUBAREA RUNOFF(CFS) = 1.84
TOTAL AREA(ACRES) = 0.6 TOTAL RUNOFF(CFS) = 2.39
TC(MIN.) = 8.32

*****
FLOW PROCESS FROM NODE 303.00 TO NODE 304.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3138.00 DOWNSTREAM(FEET) = 3079.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 378.00 CHANNEL SLOPE = 0.1561
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1380 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 2.39
FLOW VELOCITY(FEET/SEC) = 2.78 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 2.26 Tc(MIN.) = 10.59
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 304.00 = 794.00 FEET.

*****
FLOW PROCESS FROM NODE 303.00 TO NODE 304.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.847

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                                EX300.RES
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4309
SUBAREA AREA(ACRES) = 1.08 SUBAREA RUNOFF(CFS) = 2.21
TOTAL AREA(ACRES) = 1.7 TOTAL RUNOFF(CFS) = 4.26
TC(MIN.) = 10.59

*****
FLOW PROCESS FROM NODE 304.00 TO NODE 305.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3079.00 DOWNSTREAM(FEET) = 3062.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 222.00 CHANNEL SLOPE = 0.0766
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .0766 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 4.26
FLOW VELOCITY(FEET/SEC) = 2.51 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.47 Tc(MIN.) = 12.06
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 305.00 = 1016.00 FEET.

*****
FLOW PROCESS FROM NODE 304.00 TO NODE 305.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.375
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3790
SUBAREA AREA(ACRES) = 3.02 SUBAREA RUNOFF(CFS) = 5.68
TOTAL AREA(ACRES) = 4.7 TOTAL RUNOFF(CFS) = 9.60
TC(MIN.) = 12.06

*****
FLOW PROCESS FROM NODE 305.00 TO NODE 306.00 IS CODE = 52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3062.00 DOWNSTREAM(FEET) = 3044.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 230.00 CHANNEL SLOPE = 0.0783
CHANNEL FLOW THRU SUBAREA(CFS) = 9.60
FLOW VELOCITY(FEET/SEC) = 6.92 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.55 Tc(MIN.) = 12.61
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 306.00 = 1246.00 FEET.

*****
FLOW PROCESS FROM NODE 305.00 TO NODE 306.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.222
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3611
SUBAREA AREA(ACRES) = 7.57 SUBAREA RUNOFF(CFS) = 13.83
TOTAL AREA(ACRES) = 12.3 TOTAL RUNOFF(CFS) = 23.16
TC(MIN.) = 12.61

*****
FLOW PROCESS FROM NODE 306.00 TO NODE 307.00 IS CODE = 52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3044.00 DOWNSTREAM(FEET) = 2974.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 621.00 CHANNEL SLOPE = 0.1127
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 23.16

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                                EX300.RES
FLOW VELOCITY(FEET/SEC) =    9.86 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    1.05   Tc(MIN.) =    13.66
LONGEST FLOWPATH FROM NODE    300.00 TO NODE    307.00 =    1867.00 FEET.

*****
FLOW PROCESS FROM NODE    306.00 TO NODE    307.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    4.959
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3564
SUBAREA AREA(ACRES) =    9.25   SUBAREA RUNOFF(CFS) =    16.06
TOTAL AREA(ACRES) =    21.5   TOTAL RUNOFF(CFS) =    38.05
TC(MIN.) =    13.66

*****
FLOW PROCESS FROM NODE    307.00 TO NODE    308.00 IS CODE =    52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    2974.00   DOWNSTREAM(FEET) =    2916.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    656.00   CHANNEL SLOPE =    0.0884
CHANNEL FLOW THRU SUBAREA(CFS) =    38.05
FLOW VELOCITY(FEET/SEC) =    10.64 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    1.03   Tc(MIN.) =    14.69
LONGEST FLOWPATH FROM NODE    300.00 TO NODE    308.00 =    2523.00 FEET.

*****
FLOW PROCESS FROM NODE    307.00 TO NODE    308.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    4.733
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3539
SUBAREA AREA(ACRES) =    13.18   SUBAREA RUNOFF(CFS) =    21.83
TOTAL AREA(ACRES) =    34.7   TOTAL RUNOFF(CFS) =    58.14
TC(MIN.) =    14.69

*****
FLOW PROCESS FROM NODE    308.00 TO NODE    309.00 IS CODE =    52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    2916.00   DOWNSTREAM(FEET) =    2868.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    311.00   CHANNEL SLOPE =    0.1543
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) =    58.14
FLOW VELOCITY(FEET/SEC) =    12.79 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    0.41   Tc(MIN.) =    15.10
LONGEST FLOWPATH FROM NODE    300.00 TO NODE    309.00 =    2834.00 FEET.

*****
FLOW PROCESS FROM NODE    308.00 TO NODE    309.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    4.650
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3531
SUBAREA AREA(ACRES) =    8.99   SUBAREA RUNOFF(CFS) =    14.63
TOTAL AREA(ACRES) =    43.7   TOTAL RUNOFF(CFS) =    71.76
TC(MIN.) =    15.10
=====
END OF STUDY SUMMARY:

```

EX300.RES
TOTAL AREA(ACRES) = 43.7 TC(MIN.) = 15.10
PEAK FLOW RATE(CFS) = 71.76

=====

END OF RATIONAL METHOD ANALYSIS

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 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
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 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

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***** DESCRIPTION OF STUDY *****
 * 01-09-2009 *
 * EXISTING HYDROLOGY BASIN 400 *
 * *

FILE NAME: SDGEE400.DAT
 TIME/DATE OF STUDY: 08:03 01/12/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
 1. Relative Flow-Depth = 0.00 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 400.00 TO NODE 401.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
 =====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 2979.00
 DOWNSTREAM ELEVATION(FEET) = 2962.00
 ELEVATION DIFFERENCE(FEET) = 17.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.26
 TOTAL AREA(ACRES) = 0.09 TOTAL RUNOFF(CFS) = 0.26

 FLOW PROCESS FROM NODE 401.00 TO NODE 402.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 2962.00 DOWNSTREAM(FEET) = 2924.00

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                                EX400.RES
CHANNEL LENGTH THRU SUBAREA(FEET) = 147.00 CHANNEL SLOPE = 0.2585
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1828 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.26
FLOW VELOCITY(FEET/SEC) = 2.39 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.02 Tc(MIN.) = 7.29
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 402.00 = 247.00 FEET.

*****
FLOW PROCESS FROM NODE 401.00 TO NODE 402.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.437
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.28 SUBAREA RUNOFF(CFS) = 0.73
TOTAL AREA(ACRES) = 0.4 TOTAL RUNOFF(CFS) = 0.96
TC(MIN.) = 7.29

*****
FLOW PROCESS FROM NODE 402.00 TO NODE 403.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 2924.00 DOWNSTREAM(FEET) = 2784.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 243.00 CHANNEL SLOPE = 0.5761
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2288 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.96
FLOW VELOCITY(FEET/SEC) = 2.68 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.51 Tc(MIN.) = 8.80
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 403.00 = 490.00 FEET.

*****
FLOW PROCESS FROM NODE 402.00 TO NODE 403.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.586
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 1.03 SUBAREA RUNOFF(CFS) = 2.37
TOTAL AREA(ACRES) = 1.4 TOTAL RUNOFF(CFS) = 3.23
TC(MIN.) = 8.80

*****
FLOW PROCESS FROM NODE 403.00 TO NODE 404.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 2784.00 DOWNSTREAM(FEET) = 2671.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 177.00 CHANNEL SLOPE = 0.6384
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2300 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 3.23
FLOW VELOCITY(FEET/SEC) = 3.97 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.74 Tc(MIN.) = 9.55
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 404.00 = 667.00 FEET.

*****
FLOW PROCESS FROM NODE 403.00 TO NODE 404.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.250

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                                EX400.RES
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 1.90 SUBAREA RUNOFF(CFS) = 4.16
TOTAL AREA(ACRES) = 3.3 TOTAL RUNOFF(CFS) = 7.22
TC(MIN.) = 9.55

*****
FLOW PROCESS FROM NODE 404.00 TO NODE 405.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 2671.00 DOWNSTREAM(FEET) = 2510.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 516.00 CHANNEL SLOPE = 0.3120
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1980 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 7.22
FLOW VELOCITY(FEET/SEC) = 4.81 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.79 Tc(MIN.) = 11.33
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 405.00 = 1183.00 FEET.

*****
FLOW PROCESS FROM NODE 404.00 TO NODE 405.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.596
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 2.67 SUBAREA RUNOFF(CFS) = 5.23
TOTAL AREA(ACRES) = 6.0 TOTAL RUNOFF(CFS) = 11.69
TC(MIN.) = 11.33
=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 6.0 TC(MIN.) = 11.33
PEAK FLOW RATE(CFS) = 11.69
=====
END OF RATIONAL METHOD ANALYSIS

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Analysis prepared by:

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***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 11-13-2009 *
 * FINAL HYDROLOGY: BASIN 100 OUTFALL *

FILE NAME: PR100A.DAT
 TIME/DATE OF STUDY: 09:53 11/13/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
 1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3395.00
 DOWNSTREAM ELEVATION(FEET) = 3357.00
 ELEVATION DIFFERENCE(FEET) = 38.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.29
 TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.29

 FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3357.00 DOWNSTREAM(FEET) = 3276.00

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                                PRI00A.RES
CHANNEL LENGTH THRU SUBAREA(FEET) = 198.00 CHANNEL SLOPE = 0.4091
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2151 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.29
FLOW VELOCITY(FEET/SEC) = 2.60 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.27 Tc(MIN.) = 7.54
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 102.00 = 298.00 FEET.

*****
FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.279
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.88 SUBAREA RUNOFF(CFS) = 2.24
TOTAL AREA(ACRES) = 1.0 TOTAL RUNOFF(CFS) = 2.50
TC(MIN.) = 7.54

*****
FLOW PROCESS FROM NODE 102.00 TO NODE 103.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3276.00 DOWNSTREAM(FEET) = 3169.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 400.00 CHANNEL SLOPE = 0.2675
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1858 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 2.50
FLOW VELOCITY(FEET/SEC) = 3.27 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 2.04 Tc(MIN.) = 9.57
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 103.00 = 698.00 FEET.

*****
FLOW PROCESS FROM NODE 102.00 TO NODE 103.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.239
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.86 SUBAREA RUNOFF(CFS) = 1.88
TOTAL AREA(ACRES) = 1.8 TOTAL RUNOFF(CFS) = 4.02
TC(MIN.) = 9.57

*****
FLOW PROCESS FROM NODE 103.00 TO NODE 104.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3169.00 DOWNSTREAM(FEET) = 3146.24
CHANNEL LENGTH THRU SUBAREA(FEET) = 332.80 CHANNEL SLOPE = 0.0684
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .0684 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 4.02
FLOW VELOCITY(FEET/SEC) = 2.33 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 2.38 Tc(MIN.) = 11.96
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 104.00 = 1030.80 FEET.

*****
FLOW PROCESS FROM NODE 103.00 TO NODE 104.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.405
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500

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PRI00A.RES

SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
 SUBAREA AREA(ACRES) = 1.83 SUBAREA RUNOFF(CFS) = 3.46
 TOTAL AREA(ACRES) = 3.7 TOTAL RUNOFF(CFS) = 6.94
 TC(MIN.) = 11.96

 FLOW PROCESS FROM NODE 105.00 TO NODE 105.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
 =====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.405
 *USER SPECIFIED(SUBAREA):
 NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .5600
 S.C.S. CURVE NUMBER (AMC II) = 88
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.3639
 SUBAREA AREA(ACRES) = 0.26 SUBAREA RUNOFF(CFS) = 0.79
 TOTAL AREA(ACRES) = 3.9 TOTAL RUNOFF(CFS) = 7.73
 TC(MIN.) = 11.96

 FLOW PROCESS FROM NODE 104.00 TO NODE 106.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 3140.33 DOWNSTREAM(FEET) = 3127.66
 FLOW LENGTH(FEET) = 432.50 MANNING'S N = 0.012
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.78
 ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 7.73
 PIPE TRAVEL TIME(MIN.) = 0.74 Tc(MIN.) = 12.69
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 106.00 = 1463.30 FEET.

 FLOW PROCESS FROM NODE 104.00 TO NODE 106.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
 =====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.201
 NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.3569
 SUBAREA AREA(ACRES) = 4.02 SUBAREA RUNOFF(CFS) = 7.32
 TOTAL AREA(ACRES) = 7.9 TOTAL RUNOFF(CFS) = 14.75
 TC(MIN.) = 12.69

 FLOW PROCESS FROM NODE 106.00 TO NODE 107.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 3127.66 DOWNSTREAM(FEET) = 3107.98
 FLOW LENGTH(FEET) = 452.90 MANNING'S N = 0.012
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.8 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 13.28
 ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 14.75
 PIPE TRAVEL TIME(MIN.) = 0.57 Tc(MIN.) = 13.26
 LONGEST FLOWPATH FROM NODE 100.00 TO NODE 107.00 = 1916.20 FEET.

 FLOW PROCESS FROM NODE 107.00 TO NODE 107.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
 =====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.056
 NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"

PRI00A.RES

S.C.S. CURVE NUMBER (AMC II) = 88
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.3540
 SUBAREA AREA(ACRES) = 5.73 SUBAREA RUNOFF(CFS) = 10.14
 TOTAL AREA(ACRES) = 13.7 TOTAL RUNOFF(CFS) = 24.48
 TC(MIN.) = 13.26

 FLOW PROCESS FROM NODE 107.00 TO NODE 108.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3107.98	DOWNSTREAM(FEET) =	3090.99
FLOW LENGTH(FEET) =	296.10	MANNING'S N =	0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS	14.4 INCHES		
PIPE-FLOW VELOCITY(FEET/SEC.) =	16.21		
ESTIMATED PIPE DIAMETER(INCH) =	18.00	NUMBER OF PIPES =	1
PIPE-FLOW(CFS) =	24.48		
PIPE TRAVEL TIME(MIN.) =	0.30	Tc(MIN.) =	13.57
LONGEST FLOWPATH FROM NODE	100.00 TO NODE	108.00 =	2212.30 FEET.

 FLOW PROCESS FROM NODE 108.00 TO NODE 108.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	4.982
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500
SOIL CLASSIFICATION IS "D"	
S.C.S. CURVE NUMBER (AMC II) =	88
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3529
SUBAREA AREA(ACRES) =	5.13 SUBAREA RUNOFF(CFS) = 8.95
TOTAL AREA(ACRES) =	18.8 TOTAL RUNOFF(CFS) = 33.07
TC(MIN.) =	13.57

 FLOW PROCESS FROM NODE 108.00 TO NODE 109.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3090.99	DOWNSTREAM(FEET) =	3076.41
FLOW LENGTH(FEET) =	299.50	MANNING'S N =	0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS	16.3 INCHES		
PIPE-FLOW VELOCITY(FEET/SEC.) =	16.51		
ESTIMATED PIPE DIAMETER(INCH) =	21.00	NUMBER OF PIPES =	1
PIPE-FLOW(CFS) =	33.07		
PIPE TRAVEL TIME(MIN.) =	0.30	Tc(MIN.) =	13.87
LONGEST FLOWPATH FROM NODE	100.00 TO NODE	109.00 =	2511.80 FEET.

 FLOW PROCESS FROM NODE 109.00 TO NODE 109.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	4.912
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500
SOIL CLASSIFICATION IS "D"	
S.C.S. CURVE NUMBER (AMC II) =	88
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3524
SUBAREA AREA(ACRES) =	3.58 SUBAREA RUNOFF(CFS) = 6.15
TOTAL AREA(ACRES) =	22.4 TOTAL RUNOFF(CFS) = 38.76
TC(MIN.) =	13.87

 FLOW PROCESS FROM NODE 109.00 TO NODE 110.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3076.41	DOWNSTREAM(FEET) =	3061.41
FLOW LENGTH(FEET) =	34.20	MANNING'S N =	0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO	18.000		
DEPTH OF FLOW IN 18.0 INCH PIPE IS	9.6 INCHES		

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                                PR100A.RES
PIPE-FLOW VELOCITY(FEET/SEC.) = 40.39
ESTIMATED PIPE DIAMETER(INCH) = 18.00      NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 38.76
PIPE TRAVEL TIME(MIN.) = 0.01      Tc(MIN.) = 13.88
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 110.00 = 2546.00 FEET.

*****
FLOW PROCESS FROM NODE 110.00 TO NODE 110.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.909
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3523
SUBAREA AREA(ACRES) = 1.00      SUBAREA RUNOFF(CFS) = 1.72
TOTAL AREA(ACRES) = 23.4      TOTAL RUNOFF(CFS) = 40.45
TC(MIN.) = 13.88

*****
FLOW PROCESS FROM NODE 110.00 TO NODE 110.50 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3061.41      DOWNSTREAM(FEET) = 3052.85
FLOW LENGTH(FEET) = 19.60      MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 9.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 40.74
ESTIMATED PIPE DIAMETER(INCH) = 18.00      NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 40.45
PIPE TRAVEL TIME(MIN.) = 0.01      Tc(MIN.) = 13.89
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 110.50 = 2565.60 FEET.

*****
FLOW PROCESS FROM NODE 110.50 TO NODE 110.50 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.907
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3527
SUBAREA AREA(ACRES) = 0.18      SUBAREA RUNOFF(CFS) = 0.35
TOTAL AREA(ACRES) = 23.6      TOTAL RUNOFF(CFS) = 40.79
TC(MIN.) = 13.89

*****
FLOW PROCESS FROM NODE 110.50 TO NODE 111.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3052.85      DOWNSTREAM(FEET) = 3048.64
FLOW LENGTH(FEET) = 13.70      MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 35.62
ESTIMATED PIPE DIAMETER(INCH) = 18.00      NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 40.79
PIPE TRAVEL TIME(MIN.) = 0.01      Tc(MIN.) = 13.90
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 111.00 = 2579.30 FEET.

*****
FLOW PROCESS FROM NODE 111.00 TO NODE 111.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====
*****
FLOW PROCESS FROM NODE 111.00 TO NODE 111.00 IS CODE = 13
-----

```

>>>>CLEAR THE MAIN-STREAM MEMORY<<<<

```
*****
FLOW PROCESS FROM NODE      112.00 TO NODE      113.00 IS CODE =  21
-----
```

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

```
=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3332.00
DOWNSTREAM ELEVATION(FEET) = 3308.00
ELEVATION DIFFERENCE(FEET) = 24.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
SUBAREA RUNOFF(CFS) = 0.14
TOTAL AREA(ACRES) = 0.05 TOTAL RUNOFF(CFS) = 0.14
```

```
*****
FLOW PROCESS FROM NODE      113.00 TO NODE      114.00 IS CODE =  53
-----
```

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3308.00 DOWNSTREAM(FEET) = 3212.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 189.00 CHANNEL SLOPE = 0.5079
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2254 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.14
FLOW VELOCITY(FEET/SEC) = 2.66 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.18 Tc(MIN.) = 7.45
LONGEST FLOWPATH FROM NODE      112.00 TO NODE      114.00 = 289.00 FEET.
```

```
*****
FLOW PROCESS FROM NODE      113.00 TO NODE      114.00 IS CODE =  81
-----
```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

```
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.333
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.17 SUBAREA RUNOFF(CFS) = 0.44
TOTAL AREA(ACRES) = 0.2 TOTAL RUNOFF(CFS) = 0.56
TC(MIN.) = 7.45
```

```
*****
FLOW PROCESS FROM NODE      114.00 TO NODE      115.00 IS CODE =  53
-----
```

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3212.00 DOWNSTREAM(FEET) = 3174.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 112.00 CHANNEL SLOPE = 0.3393
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2034 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.56
FLOW VELOCITY(FEET/SEC) = 2.53 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.74 Tc(MIN.) = 8.19
LONGEST FLOWPATH FROM NODE      112.00 TO NODE      115.00 = 401.00 FEET.
```

```
*****
FLOW PROCESS FROM NODE      114.00 TO NODE      115.00 IS CODE =  81
-----
```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

```
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.899
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
```


PRI00A.RES

SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
 SUBAREA AREA(ACRES) = 0.24 SUBAREA RUNOFF(CFS) = 0.58
 TOTAL AREA(ACRES) = 0.5 TOTAL RUNOFF(CFS) = 1.11
 TC(MIN.) = 8.19

 FLOW PROCESS FROM NODE 115.00 TO NODE 116.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3174.00	DOWNSTREAM(FEET) =	3155.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	154.60	CHANNEL SLOPE =	0.1229
SLOPE ADJUSTMENT CURVE USED:			
EFFECTIVE SLOPE =	.1169	(PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)	
CHANNEL FLOW THRU SUBAREA(CFS) =	1.11		
FLOW VELOCITY(FEET/SEC) =	1.98	(PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)	
TRAVEL TIME(MIN.) =	1.30	Tc(MIN.) =	9.49
LONGEST FLOWPATH FROM NODE	112.00 TO NODE	116.00 =	555.60 FEET.

 FLOW PROCESS FROM NODE 115.00 TO NODE 116.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	6.274
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500
SOIL CLASSIFICATION IS "D"	
S.C.S. CURVE NUMBER (AMC II) =	88
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3500
SUBAREA AREA(ACRES) =	1.23 SUBAREA RUNOFF(CFS) = 2.70
TOTAL AREA(ACRES) =	1.7 TOTAL RUNOFF(CFS) = 3.71
TC(MIN.) =	9.49

 FLOW PROCESS FROM NODE 116.00 TO NODE 117.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3155.00	DOWNSTREAM(FEET) =	3105.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	652.90	CHANNEL SLOPE =	0.0766
CHANNEL BASE(FEET) =	2.50	"Z" FACTOR =	2.000
MANNING'S FACTOR =	0.015	MAXIMUM DEPTH(FEET) =	3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	5.868		
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500		
SOIL CLASSIFICATION IS "D"			
S.C.S. CURVE NUMBER (AMC II) =	88		
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =	9.25		
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =	10.48		
AVERAGE FLOW DEPTH(FEET) =	0.29	TRAVEL TIME(MIN.) =	1.04
Tc(MIN.) =	10.53		
SUBAREA AREA(ACRES) =	5.39	SUBAREA RUNOFF(CFS) =	11.07
AREA-AVERAGE RUNOFF COEFFICIENT =	0.350		
TOTAL AREA(ACRES) =	7.1	PEAK FLOW RATE(CFS) =	14.54

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.37 FLOW VELOCITY(FEET/SEC.) = 12.09
 LONGEST FLOWPATH FROM NODE 112.00 TO NODE 117.00 = 1208.50 FEET.

 FLOW PROCESS FROM NODE 117.00 TO NODE 117.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	5.868
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500
SOIL CLASSIFICATION IS "D"	
S.C.S. CURVE NUMBER (AMC II) =	88
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3500
SUBAREA AREA(ACRES) =	0.69 SUBAREA RUNOFF(CFS) = 1.42
TOTAL AREA(ACRES) =	7.8 TOTAL RUNOFF(CFS) = 15.96

TC(MIN.) = 10.53

```

*****
FLOW PROCESS FROM NODE      117.00 TO NODE      118.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3102.75  DOWNSTREAM(FEET) =  3056.40
CHANNEL LENGTH THRU SUBAREA(FEET) =   83.40  CHANNEL SLOPE =  0.5558
CHANNEL BASE(FEET) =   2.50  "Z" FACTOR =  2.000
MANNING'S FACTOR = 0.015  MAXIMUM DEPTH(FEET) =  1.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  5.847
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) =  88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =   16.76
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =  24.41
AVERAGE FLOW DEPTH(FEET) =  0.23  TRAVEL TIME(MIN.) =  0.06
Tc(MIN.) =  10.59
SUBAREA AREA(ACRES) =   0.69  SUBAREA RUNOFF(CFS) =   1.61
AREA-AVERAGE RUNOFF COEFFICIENT =  0.354
TOTAL AREA(ACRES) =   8.5  PEAK FLOW RATE(CFS) =   17.52

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =  0.23  FLOW VELOCITY(FEET/SEC.) =  25.26
LONGEST FLOWPATH FROM NODE      112.00 TO NODE      118.00 =   1291.90 FEET.

*****
FLOW PROCESS FROM NODE      118.00 TO NODE      119.00 IS CODE =  31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3048.42  DOWNSTREAM(FEET) =  3047.02
FLOW LENGTH(FEET) =  101.70  MANNING'S N =  0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 16.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  8.78
ESTIMATED PIPE DIAMETER(INCH) =  21.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =   17.52
PIPE TRAVEL TIME(MIN.) =  0.19  Tc(MIN.) =  10.78
LONGEST FLOWPATH FROM NODE      112.00 TO NODE      119.00 =   1393.60 FEET.

*****
FLOW PROCESS FROM NODE      119.00 TO NODE      120.00 IS CODE =  31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3047.02  DOWNSTREAM(FEET) =  3046.20
FLOW LENGTH(FEET) =  107.20  MANNING'S N =  0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 17.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  7.10
ESTIMATED PIPE DIAMETER(INCH) =  24.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =   17.52
PIPE TRAVEL TIME(MIN.) =  0.25  Tc(MIN.) =  11.03
LONGEST FLOWPATH FROM NODE      112.00 TO NODE      120.00 =   1500.80 FEET.

*****
FLOW PROCESS FROM NODE      120.00 TO NODE      121.00 IS CODE =  31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3046.20  DOWNSTREAM(FEET) =  3044.48
FLOW LENGTH(FEET) =  288.60  MANNING'S N =  0.012
DEPTH OF FLOW IN 27.0 INCH PIPE IS 17.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  6.56
ESTIMATED PIPE DIAMETER(INCH) =  27.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =   17.52
PIPE TRAVEL TIME(MIN.) =  0.73  Tc(MIN.) =  11.76
LONGEST FLOWPATH FROM NODE      112.00 TO NODE      121.00 =   1789.40 FEET.

*****

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```

                                PR100A.RES
FLOW PROCESS FROM NODE      121.00 TO NODE      111.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3044.48  DOWNSTREAM(FEET) = 3041.50
FLOW LENGTH(FEET) = 298.50  MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.94
ESTIMATED PIPE DIAMETER(INCH) = 24.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 17.52
PIPE TRAVEL TIME(MIN.) = 0.63  Tc(MIN.) = 12.39
LONGEST FLOWPATH FROM NODE 112.00 TO NODE 111.00 = 2087.90 FEET.

*****
FLOW PROCESS FROM NODE      111.00 TO NODE      111.00 IS CODE =  11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM  RUNOFF  Tc  INTENSITY  AREA
NUMBER  (CFS)  (MIN.)  (INCH/HR)  (ACRE)
1       17.52  12.39   5.283     8.46
LONGEST FLOWPATH FROM NODE 112.00 TO NODE 111.00 = 2087.90 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM  RUNOFF  Tc  INTENSITY  AREA
NUMBER  (CFS)  (MIN.)  (INCH/HR)  (ACRE)
1       40.79  13.90   4.905    23.57
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 111.00 = 2579.30 FEET.

** PEAK FLOW RATE TABLE **
STREAM  RUNOFF  Tc  INTENSITY
NUMBER  (CFS)  (MIN.)  (INCH/HR)
1       53.88  12.39   5.283
2       57.06  13.90   4.905

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 57.06  Tc(MIN.) = 13.90
TOTAL AREA(ACRES) = 32.0

*****
FLOW PROCESS FROM NODE      111.00 TO NODE      111.00 IS CODE =  12
-----
>>>>CLEAR MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE      111.00 TO NODE      122.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3041.50  DOWNSTREAM(FEET) = 3040.03
FLOW LENGTH(FEET) = 239.21  MANNING'S N = 0.012
DEPTH OF FLOW IN 39.0 INCH PIPE IS 28.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.80
ESTIMATED PIPE DIAMETER(INCH) = 39.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 57.06
PIPE TRAVEL TIME(MIN.) = 0.45  Tc(MIN.) = 14.35
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 122.00 = 2818.51 FEET.

*****
FLOW PROCESS FROM NODE      122.00 TO NODE      124.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3040.03  DOWNSTREAM(FEET) = 2998.17
FLOW LENGTH(FEET) = 157.30  MANNING'S N = 0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 12.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 36.71
ESTIMATED PIPE DIAMETER(INCH) = 21.00  NUMBER OF PIPES = 1

```

```

                                PRI00A.RES
PIPE-FLOW(CFS) =          57.06
PIPE TRAVEL TIME(MIN.) =    0.07      Tc(MIN.) =    14.42
LONGEST FLOWPATH FROM NODE    100.00 TO NODE    124.00 =    2975.81 FEET.

*****
FLOW PROCESS FROM NODE    124.00 TO NODE    124.00 IS CODE =    81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    4.790
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT =    0.3539
SUBAREA AREA(ACRES) =    0.58      SUBAREA RUNOFF(CFS) =    1.11
TOTAL AREA(ACRES) =    32.6      TOTAL RUNOFF(CFS) =    57.06
TC(MIN.) =    14.42
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

*****
FLOW PROCESS FROM NODE    124.00 TO NODE    126.00 IS CODE =    31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    2998.17  DOWNSTREAM(FEET) =    2978.56
FLOW LENGTH(FEET) =    73.70  MANNING'S N =    0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 12.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    36.71
ESTIMATED PIPE DIAMETER(INCH) =    21.00      NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =    57.06
PIPE TRAVEL TIME(MIN.) =    0.03      Tc(MIN.) =    14.46
LONGEST FLOWPATH FROM NODE    100.00 TO NODE    126.00 =    3049.51 FEET.

*****
FLOW PROCESS FROM NODE    126.00 TO NODE    126.00 IS CODE =    81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    4.782
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT =    0.3544
SUBAREA AREA(ACRES) =    0.34      SUBAREA RUNOFF(CFS) =    0.65
TOTAL AREA(ACRES) =    33.0      TOTAL RUNOFF(CFS) =    57.06
TC(MIN.) =    14.46
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

*****
FLOW PROCESS FROM NODE    126.00 TO NODE    128.00 IS CODE =    31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    2978.56  DOWNSTREAM(FEET) =    2962.03
FLOW LENGTH(FEET) =    65.80  MANNING'S N =    0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    35.88
ESTIMATED PIPE DIAMETER(INCH) =    21.00      NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =    57.06
PIPE TRAVEL TIME(MIN.) =    0.03      Tc(MIN.) =    14.49
LONGEST FLOWPATH FROM NODE    100.00 TO NODE    128.00 =    3115.31 FEET.

*****
FLOW PROCESS FROM NODE    128.00 TO NODE    130.00 IS CODE =    31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    2962.03  DOWNSTREAM(FEET) =    2961.94
FLOW LENGTH(FEET) =    21.50  MANNING'S N =    0.012
DEPTH OF FLOW IN 42.0 INCH PIPE IS 30.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    7.62

```

```

                                PR100A.RES
ESTIMATED PIPE DIAMETER(INCH) = 42.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 57.06
PIPE TRAVEL TIME(MIN.) = 0.05    Tc(MIN.) = 14.53
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 130.00 = 3136.81 FEET.

*****
FLOW PROCESS FROM NODE 130.00 TO NODE 130.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE 130.00 TO NODE 130.00 IS CODE = 13
-----
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<
=====

*****
FLOW PROCESS FROM NODE 199.00 TO NODE 151.00 IS CODE = 7
-----
>>>>USER SPECIFIED HYDROLOGY INFORMATION AT NODE<<<<
=====
USER-SPECIFIED VALUES ARE AS FOLLOWS:
TC(MIN) = 18.20    RAIN INTENSITY(INCH/HOUR) = 4.12
TOTAL AREA(ACRES) = 22.50    TOTAL RUNOFF(CFS) = 18.20

*****
FLOW PROCESS FROM NODE 151.00 TO NODE 151.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1          18.20    18.20    4.122    22.50
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 151.00 = 3136.81 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1          57.06    14.53    4.766    32.95
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 151.00 = 3136.81 FEET.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF    Tc    INTENSITY
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)
1          71.59    14.53    4.766
2          67.55    18.20    4.122

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 71.59    Tc(MIN.) = 14.53
TOTAL AREA(ACRES) = 55.5

*****
FLOW PROCESS FROM NODE 151.00 TO NODE 151.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE 151.00 TO NODE 151.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE 151.00 TO NODE 151.00 IS CODE = 13
-----
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<
=====

*****
FLOW PROCESS FROM NODE 152.00 TO NODE 153.00 IS CODE = 21

```

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
 =====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3390.00
 DOWNSTREAM ELEVATION(FEET) = 3336.00
 ELEVATION DIFFERENCE(FEET) = 54.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.17
 TOTAL AREA(ACRES) = 0.06 TOTAL RUNOFF(CFS) = 0.17

 FLOW PROCESS FROM NODE 153.00 TO NODE 154.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 3336.00 DOWNSTREAM(FEET) = 3257.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 154.00 CHANNEL SLOPE = 0.5130
 SLOPE ADJUSTMENT CURVE USED:
 EFFECTIVE SLOPE = .2256 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
 NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
 CHANNEL FLOW THRU SUBAREA(CFS) = 0.17
 FLOW VELOCITY(FEET/SEC) = 2.66 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
 TRAVEL TIME(MIN.) = 0.96 Tc(MIN.) = 7.23
 LONGEST FLOWPATH FROM NODE 152.00 TO NODE 154.00 = 254.00 FEET.

 FLOW PROCESS FROM NODE 153.00 TO NODE 154.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
 =====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.476
 NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
 SUBAREA AREA(ACRES) = 0.15 SUBAREA RUNOFF(CFS) = 0.39
 TOTAL AREA(ACRES) = 0.2 TOTAL RUNOFF(CFS) = 0.55
 TC(MIN.) = 7.23

 FLOW PROCESS FROM NODE 154.00 TO NODE 155.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 3257.00 DOWNSTREAM(FEET) = 3073.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 419.00 CHANNEL SLOPE = 0.4391
 SLOPE ADJUSTMENT CURVE USED:
 EFFECTIVE SLOPE = .2187 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
 NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
 CHANNEL FLOW THRU SUBAREA(CFS) = 0.55
 FLOW VELOCITY(FEET/SEC) = 2.62 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
 TRAVEL TIME(MIN.) = 2.67 Tc(MIN.) = 9.90
 LONGEST FLOWPATH FROM NODE 152.00 TO NODE 155.00 = 673.00 FEET.

 FLOW PROCESS FROM NODE 154.00 TO NODE 155.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
 =====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.106
 NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
 SUBAREA AREA(ACRES) = 1.23 SUBAREA RUNOFF(CFS) = 2.63
 TOTAL AREA(ACRES) = 1.4 TOTAL RUNOFF(CFS) = 3.08

TC(MIN.) = 9.90

```
*****
FLOW PROCESS FROM NODE      155.00 TO NODE      156.00 IS CODE =  53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3073.00 DOWNSTREAM(FEET) =  3065.50
CHANNEL LENGTH THRU SUBAREA(FEET) =  157.00 CHANNEL SLOPE =  0.0478
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .0478 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) =      3.08
FLOW VELOCITY(FEET/SEC) =  1.78 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =  1.47 Tc(MIN.) =  11.37
LONGEST FLOWPATH FROM NODE  152.00 TO NODE  156.00 =      830.00 FEET.

*****
FLOW PROCESS FROM NODE      155.00 TO NODE      156.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  5.584
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =  88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) =  2.39 SUBAREA RUNOFF(CFS) =  4.67
TOTAL AREA(ACRES) =  3.8 TOTAL RUNOFF(CFS) =  7.49
TC(MIN.) =  11.37

*****
FLOW PROCESS FROM NODE      156.00 TO NODE      151.00 IS CODE =  53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3065.50 DOWNSTREAM(FEET) =  2963.00
CHANNEL LENGTH THRU SUBAREA(FEET) =  486.00 CHANNEL SLOPE =  0.2109
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1655 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) =  7.49
FLOW VELOCITY(FEET/SEC) =  4.45 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =  1.82 Tc(MIN.) =  13.19
LONGEST FLOWPATH FROM NODE  152.00 TO NODE  151.00 =  1316.00 FEET.

*****
FLOW PROCESS FROM NODE      156.00 TO NODE      151.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  5.074
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3600
S.C.S. CURVE NUMBER (AMC II) =  88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3544
SUBAREA AREA(ACRES) =  3.03 SUBAREA RUNOFF(CFS) =  5.53
TOTAL AREA(ACRES) =  6.9 TOTAL RUNOFF(CFS) =  12.34
TC(MIN.) =  13.19

*****
FLOW PROCESS FROM NODE      151.00 TO NODE      151.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  5.074
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3600
S.C.S. CURVE NUMBER (AMC II) =  88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3551
SUBAREA AREA(ACRES) =  0.88 SUBAREA RUNOFF(CFS) =  1.61
TOTAL AREA(ACRES) =  7.7 TOTAL RUNOFF(CFS) =  13.94
TC(MIN.) =  13.19
```

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                                PR100A.RES
*****
FLOW PROCESS FROM NODE      151.00 TO NODE      151.00 IS CODE =  11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/ HOUR)  (ACRE)
   1         13.94     13.19      5.074         7.74
LONGEST FLOWPATH FROM NODE      152.00 TO NODE      151.00 =      1316.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/ HOUR)  (ACRE)
   1         71.59     14.53      4.766        55.45
LONGEST FLOWPATH FROM NODE      100.00 TO NODE      151.00 =      3136.81 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/ HOUR)
   1         78.90     13.19      5.074
   2         84.69     14.53      4.766

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      84.69   Tc(MIN.) =    14.53
TOTAL AREA(ACRES) =      63.2
=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES)   =      63.2   TC(MIN.) =    14.53
PEAK FLOW RATE(CFS) =      84.69
=====
END OF RATIONAL METHOD ANALYSIS

```

□

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
 (c) Copyright 1982-2008 Advanced Engineering Software (aes)
 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

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 15070 Avenue of Science
 Suite 100
 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 11-6-2009 *
 * PROPOSED FINAL HYDROLOGY *

FILE NAME: PR100B.DAT
 TIME/DATE OF STUDY: 08:21 11/06/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 160.00 TO NODE 161.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
 =====

*USER SPECIFIED(SUBAREA):
 STREETS & ROADS (CURBS/STORM DRAINS) RUNOFF COEFFICIENT = .9500
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3164.50
 DOWNSTREAM ELEVATION(FEET) = 3163.00
 ELEVATION DIFFERENCE(FEET) = 1.50
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 1.902
 WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
 THE MAXIMUM OVERLAND FLOW LENGTH = 65.00
 (Reference: Table 3-1B of Hydrology Manual)
 THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 9.485
 NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
 SUBAREA RUNOFF(CFS) = 0.54
 TOTAL AREA(ACRES) = 0.06 TOTAL RUNOFF(CFS) = 0.54

 FLOW PROCESS FROM NODE 161.00 TO NODE 162.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3163.00 DOWNSTREAM(FEET) = 3150.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 181.60 CHANNEL SLOPE = 0.0716
CHANNEL BASE(FEET) = 1.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.016 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 9.485
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
*USER SPECIFIED(SUBAREA):
STREETS & ROADS (CURBS/STORM DRAINS) RUNOFF COEFFICIENT = .9500
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.95
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.89
AVERAGE FLOW DEPTH(FEET) = 0.07 TRAVEL TIME(MIN.) = 1.05
Tc(MIN.) = 2.95
SUBAREA AREA(ACRES) = 0.09 SUBAREA RUNOFF(CFS) = 0.81
AREA-AVERAGE RUNOFF COEFFICIENT = 0.950
TOTAL AREA(ACRES) = 0.2 PEAK FLOW RATE(CFS) = 1.35
```

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.08 FLOW VELOCITY(FEET/SEC.) = 3.30
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 162.00 = 281.60 FEET.

FLOW PROCESS FROM NODE 162.00 TO NODE 163.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3150.00 DOWNSTREAM(FEET) = 3122.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 281.80 CHANNEL SLOPE = 0.0994
CHANNEL BASE(FEET) = 1.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.016 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 9.485
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
*USER SPECIFIED(SUBAREA):
STREETS & ROADS (CURBS/STORM DRAINS) RUNOFF COEFFICIENT = .9500
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.21
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.09
AVERAGE FLOW DEPTH(FEET) = 0.09 TRAVEL TIME(MIN.) = 1.15
Tc(MIN.) = 4.10
SUBAREA AREA(ACRES) = 0.19 SUBAREA RUNOFF(CFS) = 1.71
AREA-AVERAGE RUNOFF COEFFICIENT = 0.950
TOTAL AREA(ACRES) = 0.3 PEAK FLOW RATE(CFS) = 3.06
```

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.11 FLOW VELOCITY(FEET/SEC.) = 4.31
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 163.00 = 563.40 FEET.

FLOW PROCESS FROM NODE 163.00 TO NODE 164.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3122.00 DOWNSTREAM(FEET) = 3066.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 574.30 CHANNEL SLOPE = 0.0975
CHANNEL BASE(FEET) = 1.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.016 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.420
*USER SPECIFIED(SUBAREA):
STREETS & ROADS (CURBS/STORM DRAINS) RUNOFF COEFFICIENT = .9500
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.65
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.99
AVERAGE FLOW DEPTH(FEET) = 0.13 TRAVEL TIME(MIN.) = 1.92
Tc(MIN.) = 6.01
SUBAREA AREA(ACRES) = 0.40 SUBAREA RUNOFF(CFS) = 3.20
AREA-AVERAGE RUNOFF COEFFICIENT = 0.950
TOTAL AREA(ACRES) = 0.7 PEAK FLOW RATE(CFS) = 5.92
```

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

```

                                PRI00B.RES
DEPTH(FEET) = 0.14   FLOW VELOCITY(FEET/SEC.) = 5.14
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 164.00 = 1137.70 FEET.

*****
FLOW PROCESS FROM NODE 164.00 TO NODE 165.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3066.00 DOWNSTREAM(FEET) = 3058.64
CHANNEL LENGTH THRU SUBAREA(FEET) = 121.90 CHANNEL SLOPE = 0.0604
CHANNEL BASE(FEET) = 1.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.016 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.033
*USER SPECIFIED(SUBAREA):
STREETS & ROADS (CURBS/STORM DRAINS) RUNOFF COEFFICIENT = .9500
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.22
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.46
AVERAGE FLOW DEPTH(FEET) = 0.16 TRAVEL TIME(MIN.) = 0.46
Tc(MIN.) = 6.47
SUBAREA AREA(ACRES) = 0.08 SUBAREA RUNOFF(CFS) = 0.61
AREA-AVERAGE RUNOFF COEFFICIENT = 0.950
TOTAL AREA(ACRES) = 0.8 PEAK FLOW RATE(CFS) = 6.26

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.16 FLOW VELOCITY(FEET/SEC.) = 4.48
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 165.00 = 1259.60 FEET.

*****
FLOW PROCESS FROM NODE 165.00 TO NODE 165.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.033
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6168
SUBAREA AREA(ACRES) = 1.26 SUBAREA RUNOFF(CFS) = 4.05
TOTAL AREA(ACRES) = 2.1 TOTAL RUNOFF(CFS) = 10.31
TC(MIN.) = 6.47

*****
FLOW PROCESS FROM NODE 165.00 TO NODE 167.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3051.56 DOWNSTREAM(FEET) = 3050.86
FLOW LENGTH(FEET) = 84.70 MANNING'S N = 0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.50
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 10.31
PIPE TRAVEL TIME(MIN.) = 0.22 Tc(MIN.) = 6.69
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 167.00 = 1344.30 FEET.

*****
FLOW PROCESS FROM NODE 167.00 TO NODE 167.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.863
*USER SPECIFIED(SUBAREA):
STREETS & ROADS (CURBS/STORM DRAINS) RUNOFF COEFFICIENT = .8300
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6452
SUBAREA AREA(ACRES) = 0.32 SUBAREA RUNOFF(CFS) = 2.09
TOTAL AREA(ACRES) = 2.4 TOTAL RUNOFF(CFS) = 12.18
TC(MIN.) = 6.69

*****
FLOW PROCESS FROM NODE 166.00 TO NODE 166.00 IS CODE = 81

```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

```
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.863
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5475
SUBAREA AREA(ACRES) = 1.59 SUBAREA RUNOFF(CFS) = 5.00
TOTAL AREA(ACRES) = 4.0 TOTAL RUNOFF(CFS) = 17.18
TC(MIN.) = 6.69
```

```
*****
FLOW PROCESS FROM NODE 167.00 TO NODE 169.00 IS CODE = 31
-----
```

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3050.86 DOWNSTREAM(FEET) = 3050.23
FLOW LENGTH(FEET) = 69.90 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 16.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.58
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 17.18
PIPE TRAVEL TIME(MIN.) = 0.15 Tc(MIN.) = 6.84
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 169.00 = 1414.20 FEET.
```

```
*****
FLOW PROCESS FROM NODE 169.00 TO NODE 169.00 IS CODE = 10
-----
```

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<

```
*****
FLOW PROCESS FROM NODE 169.00 TO NODE 169.00 IS CODE = 13
-----
```

>>>>CLEAR THE MAIN-STREAM MEMORY<<<<

```
*****
FLOW PROCESS FROM NODE 170.00 TO NODE 171.00 IS CODE = 21
-----
```

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

```
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) = 88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3190.00
DOWNSTREAM ELEVATION(FEET) = 3060.00
ELEVATION DIFFERENCE(FEET) = 130.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.849
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.573
SUBAREA RUNOFF(CFS) = 0.14
TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.14
```

```
*****
FLOW PROCESS FROM NODE 171.00 TO NODE 172.00 IS CODE = 51
-----
```

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3060.00 DOWNSTREAM(FEET) = 3053.37
CHANNEL LENGTH THRU SUBAREA(FEET) = 48.10 CHANNEL SLOPE = 0.1378
CHANNEL BASE(FEET) = 2.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.251
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.32
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.24
AVERAGE FLOW DEPTH(FEET) = 0.06 TRAVEL TIME(MIN.) = 0.36
```

```

                                PRI00B.RES

Tc(MIN.) =      6.21
SUBAREA AREA(ACRES) =      0.10      SUBAREA RUNOFF(CFS) =      0.37
AREA-AVERAGE RUNOFF COEFFICIENT =  0.436
TOTAL AREA(ACRES) =      0.1      PEAK FLOW RATE(CFS) =      0.50

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.07  FLOW VELOCITY(FEET/SEC.) =  2.59
LONGEST FLOWPATH FROM NODE  170.00 TO NODE  172.00 =  148.10 FEET.

*****
FLOW PROCESS FROM NODE  172.00 TO NODE  173.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3053.37  DOWNSTREAM(FEET) =  3052.57
CHANNEL LENGTH THRU SUBAREA(FEET) =  163.70  CHANNEL SLOPE =  0.0049
CHANNEL BASE(FEET) =  2.00  "Z" FACTOR =  10.000
MANNING'S FACTOR = 0.030  MAXIMUM DEPTH(FEET) =  1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  6.510
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) =  88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =  1.00
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =  0.99
AVERAGE FLOW DEPTH(FEET) =  0.23  TRAVEL TIME(MIN.) =  2.76
Tc(MIN.) =  8.96
SUBAREA AREA(ACRES) =  0.34      SUBAREA RUNOFF(CFS) =  1.00
AREA-AVERAGE RUNOFF COEFFICIENT =  0.446
TOTAL AREA(ACRES) =  0.5      PEAK FLOW RATE(CFS) =  1.39

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.27  FLOW VELOCITY(FEET/SEC.) =  1.09
LONGEST FLOWPATH FROM NODE  170.00 TO NODE  173.00 =  311.80 FEET.

*****
FLOW PROCESS FROM NODE  173.00 TO NODE  174.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3053.31  DOWNSTREAM(FEET) =  3052.90
FLOW LENGTH(FEET) =  22.90  MANNING'S N =  0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS  3.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  5.06
ESTIMATED PIPE DIAMETER(INCH) =  18.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =  1.39
PIPE TRAVEL TIME(MIN.) =  0.08  Tc(MIN.) =  9.04
LONGEST FLOWPATH FROM NODE  170.00 TO NODE  174.00 =  334.70 FEET.

*****
FLOW PROCESS FROM NODE  174.00 TO NODE  174.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  6.475
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) =  88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4480
SUBAREA AREA(ACRES) =  0.51  SUBAREA RUNOFF(CFS) =  1.49
TOTAL AREA(ACRES) =  1.0  TOTAL RUNOFF(CFS) =  2.87
TC(MIN.) =  9.04

*****
FLOW PROCESS FROM NODE  174.00 TO NODE  176.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3052.90  DOWNSTREAM(FEET) =  3051.75
FLOW LENGTH(FEET) =  133.60  MANNING'S N =  0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000

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                                PRI00B.RES
DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.79
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.87
PIPE TRAVEL TIME(MIN.) = 0.46    Tc(MIN.) = 9.50
LONGEST FLOWPATH FROM NODE 170.00 TO NODE 176.00 = 468.30 FEET.

*****
FLOW PROCESS FROM NODE 176.00 TO NODE 176.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.269
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4484
SUBAREA AREA(ACRES) = 0.24    SUBAREA RUNOFF(CFS) = 0.68
TOTAL AREA(ACRES) = 1.2    TOTAL RUNOFF(CFS) = 3.46
TC(MIN.) = 9.50

*****
FLOW PROCESS FROM NODE 176.00 TO NODE 169.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3051.75    DOWNSTREAM(FEET) = 3050.30
FLOW LENGTH(FEET) = 73.50    MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.81
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.46
PIPE TRAVEL TIME(MIN.) = 0.18    Tc(MIN.) = 9.68
LONGEST FLOWPATH FROM NODE 170.00 TO NODE 169.00 = 541.80 FEET.

*****
FLOW PROCESS FROM NODE 169.00 TO NODE 169.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1          3.46    9.68    6.193    1.23
LONGEST FLOWPATH FROM NODE 170.00 TO NODE 169.00 = 541.80 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1          17.18    6.84    7.749    3.99
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 169.00 = 1414.20 FEET.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF    Tc    INTENSITY
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)
1          19.62    6.84    7.749
2          17.19    9.68    6.193

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 19.62    Tc(MIN.) = 6.84
TOTAL AREA(ACRES) = 5.2

*****
FLOW PROCESS FROM NODE 169.00 TO NODE 169.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE 169.00 TO NODE 178.00 IS CODE = 31
-----

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                                PRI00B.RES
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3051.75  DOWNSTREAM(FEET) = 3048.57
FLOW LENGTH(FEET) = 136.00  MANNING'S N = 0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 14.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.20
ESTIMATED PIPE DIAMETER(INCH) = 21.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 19.62
PIPE TRAVEL TIME(MIN.) = 0.20  Tc(MIN.) = 7.04
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 178.00 = 1550.20 FEET.

*****
FLOW PROCESS FROM NODE 178.00 TO NODE 178.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.605
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .6800
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5424
SUBAREA AREA(ACRES) = 0.69  SUBAREA RUNOFF(CFS) = 3.57
TOTAL AREA(ACRES) = 5.9  TOTAL RUNOFF(CFS) = 24.37
TC(MIN.) = 7.04

*****
FLOW PROCESS FROM NODE 178.00 TO NODE 179.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3048.57  DOWNSTREAM(FEET) = 3047.34
FLOW LENGTH(FEET) = 153.70  MANNING'S N = 0.012
DEPTH OF FLOW IN 27.0 INCH PIPE IS 19.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.85
ESTIMATED PIPE DIAMETER(INCH) = 27.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 24.37
PIPE TRAVEL TIME(MIN.) = 0.33  Tc(MIN.) = 7.37
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 179.00 = 1703.90 FEET.

*****
FLOW PROCESS FROM NODE 179.00 TO NODE 179.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.386
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5100
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5393
SUBAREA AREA(ACRES) = 0.61  SUBAREA RUNOFF(CFS) = 2.30
TOTAL AREA(ACRES) = 6.5  TOTAL RUNOFF(CFS) = 25.97
TC(MIN.) = 7.37

*****
FLOW PROCESS FROM NODE 179.50 TO NODE 179.50 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.386
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5231
SUBAREA AREA(ACRES) = 0.86  SUBAREA RUNOFF(CFS) = 2.54
TOTAL AREA(ACRES) = 7.4  TOTAL RUNOFF(CFS) = 28.51
TC(MIN.) = 7.37

*****
FLOW PROCESS FROM NODE 179.00 TO NODE 180.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

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PRI00B.RES

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=====
ELEVATION DATA: UPSTREAM(FEET) = 3047.34  DOWNSTREAM(FEET) = 3045.98
FLOW LENGTH(FEET) = 250.80  MANNING'S N = 0.012
DEPTH OF FLOW IN 30.0 INCH PIPE IS 23.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.99
ESTIMATED PIPE DIAMETER(INCH) = 30.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 28.51
PIPE TRAVEL TIME(MIN.) = 0.60  Tc(MIN.) = 7.97
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 180.00 = 1954.70 FEET.

*****
FLOW PROCESS FROM NODE 180.00 TO NODE 180.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.023
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .6100
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5297
SUBAREA AREA(ACRES) = 0.61  SUBAREA RUNOFF(CFS) = 2.61
TOTAL AREA(ACRES) = 8.0  TOTAL RUNOFF(CFS) = 29.72
TC(MIN.) = 7.97

*****
FLOW PROCESS FROM NODE 180.50 TO NODE 180.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.023
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5100
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5275
SUBAREA AREA(ACRES) = 1.00  SUBAREA RUNOFF(CFS) = 3.58
TOTAL AREA(ACRES) = 9.0  TOTAL RUNOFF(CFS) = 33.31
TC(MIN.) = 7.97

*****
FLOW PROCESS FROM NODE 180.00 TO NODE 181.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3045.98  DOWNSTREAM(FEET) = 3044.66
FLOW LENGTH(FEET) = 209.00  MANNING'S N = 0.012
DEPTH OF FLOW IN 33.0 INCH PIPE IS 22.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.84
ESTIMATED PIPE DIAMETER(INCH) = 33.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 33.31
PIPE TRAVEL TIME(MIN.) = 0.44  Tc(MIN.) = 8.41
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 181.00 = 2163.70 FEET.

*****
FLOW PROCESS FROM NODE 181.00 TO NODE 181.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.781
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5400
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5283
SUBAREA AREA(ACRES) = 0.62  SUBAREA RUNOFF(CFS) = 2.27
TOTAL AREA(ACRES) = 9.6  TOTAL RUNOFF(CFS) = 34.43
TC(MIN.) = 8.41

*****
FLOW PROCESS FROM NODE 181.50 TO NODE 181.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.781
*USER SPECIFIED(SUBAREA):

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                                PRI00B.RES
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5212
SUBAREA AREA(ACRES) = 0.97 SUBAREA RUNOFF(CFS) = 2.96
TOTAL AREA(ACRES) = 10.6 TOTAL RUNOFF(CFS) = 37.39
TC(MIN.) = 8.41

*****
FLOW PROCESS FROM NODE 181.00 TO NODE 182.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3044.66 DOWNSTREAM(FEET) = 3043.58
FLOW LENGTH(FEET) = 161.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 33.0 INCH PIPE IS 23.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.19
ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 37.39
PIPE TRAVEL TIME(MIN.) = 0.33 Tc(MIN.) = 8.74
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 182.00 = 2324.70 FEET.

*****
FLOW PROCESS FROM NODE 182.00 TO NODE 182.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.616
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .6000
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5235
SUBAREA AREA(ACRES) = 0.33 SUBAREA RUNOFF(CFS) = 1.31
TOTAL AREA(ACRES) = 10.9 TOTAL RUNOFF(CFS) = 37.79
TC(MIN.) = 8.74

*****
FLOW PROCESS FROM NODE 182.00 TO NODE 183.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3043.58 DOWNSTREAM(FEET) = 3042.32
FLOW LENGTH(FEET) = 197.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 33.0 INCH PIPE IS 24.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.04
ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 37.79
PIPE TRAVEL TIME(MIN.) = 0.41 Tc(MIN.) = 9.15
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 183.00 = 2521.70 FEET.

*****
FLOW PROCESS FROM NODE 183.50 TO NODE 183.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.424
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5500
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5249
SUBAREA AREA(ACRES) = 0.57 SUBAREA RUNOFF(CFS) = 2.01
TOTAL AREA(ACRES) = 11.5 TOTAL RUNOFF(CFS) = 38.71
TC(MIN.) = 9.15

*****
FLOW PROCESS FROM NODE 183.00 TO NODE 184.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3042.32 DOWNSTREAM(FEET) = 3041.22
FLOW LENGTH(FEET) = 222.50 MANNING'S N = 0.012
DEPTH OF FLOW IN 36.0 INCH PIPE IS 25.0 INCHES

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                                PRI00B.RES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.40
ESTIMATED PIPE DIAMETER(INCH) = 36.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 38.71
PIPE TRAVEL TIME(MIN.) = 0.50    Tc(MIN.) = 9.65
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 184.00 = 2744.20 FEET.

*****
FLOW PROCESS FROM NODE 184.00 TO NODE 184.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE 184.00 TO NODE 184.00 IS CODE = 13
-----
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<
=====

*****
FLOW PROCESS FROM NODE 185.00 TO NODE 186.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3057.70
DOWNSTREAM ELEVATION(FEET) = 3056.70
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.063
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
         THE MAXIMUM OVERLAND FLOW LENGTH = 60.00
         (Reference: Table 3-1B of Hydrology Manual)
         THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.463
SUBAREA RUNOFF(CFS) = 0.32
TOTAL AREA(ACRES) = 0.11    TOTAL RUNOFF(CFS) = 0.32

*****
FLOW PROCESS FROM NODE 186.00 TO NODE 187.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.70    DOWNSTREAM(FEET) = 3056.64
CHANNEL LENGTH THRU SUBAREA(FEET) = 42.20    CHANNEL SLOPE = 0.0014
CHANNEL BASE(FEET) = 3.00    "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030    MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.815
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.48
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.44
AVERAGE FLOW DEPTH(FEET) = 0.17    TRAVEL TIME(MIN.) = 1.61
Tc(MIN.) = 10.68
SUBAREA AREA(ACRES) = 0.12    SUBAREA RUNOFF(CFS) = 0.31
AREA-AVERAGE RUNOFF COEFFICIENT = 0.450
TOTAL AREA(ACRES) = 0.2    PEAK FLOW RATE(CFS) = 0.60

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.19    FLOW VELOCITY(FEET/SEC.) = 0.45
LONGEST FLOWPATH FROM NODE 185.00 TO NODE 187.00 = 142.20 FEET.

*****
FLOW PROCESS FROM NODE 187.00 TO NODE 188.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.64    DOWNSTREAM(FEET) = 3056.22
CHANNEL LENGTH THRU SUBAREA(FEET) = 6.00    CHANNEL SLOPE = 0.0700
CHANNEL BASE(FEET) = 3.00    "Z" FACTOR = 20.000

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                                PRI00B.RES
MANNING'S FACTOR = 0.030    MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.801
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.61
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.50
AVERAGE FLOW DEPTH(FEET) = 0.12    TRAVEL TIME(MIN.) = 0.04
Tc(MIN.) = 10.72
SUBAREA AREA(ACRES) = 0.77    SUBAREA RUNOFF(CFS) = 2.01
AREA-AVERAGE RUNOFF COEFFICIENT = 0.450
TOTAL AREA(ACRES) = 1.0    PEAK FLOW RATE(CFS) = 2.61

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.15    FLOW VELOCITY(FEET/SEC.) = 2.91
LONGEST FLOWPATH FROM NODE 185.00 TO NODE 188.00 = 148.20 FEET.

*****
FLOW PROCESS FROM NODE 188.00 TO NODE 189.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3049.72    DOWNSTREAM(FEET) = 3048.60
FLOW LENGTH(FEET) = 162.00    MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.31
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.61
PIPE TRAVEL TIME(MIN.) = 0.63    Tc(MIN.) = 11.34
LONGEST FLOWPATH FROM NODE 185.00 TO NODE 189.00 = 310.20 FEET.

*****
FLOW PROCESS FROM NODE 189.00 TO NODE 189.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.592
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .7600
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5613
SUBAREA AREA(ACRES) = 0.56    SUBAREA RUNOFF(CFS) = 2.38
TOTAL AREA(ACRES) = 1.6    TOTAL RUNOFF(CFS) = 4.90
TC(MIN.) = 11.34

*****
FLOW PROCESS FROM NODE 189.00 TO NODE 190.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3048.60    DOWNSTREAM(FEET) = 3047.22
FLOW LENGTH(FEET) = 222.50    MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.87
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.90
PIPE TRAVEL TIME(MIN.) = 0.76    Tc(MIN.) = 12.10
LONGEST FLOWPATH FROM NODE 185.00 TO NODE 190.00 = 532.70 FEET.

*****
FLOW PROCESS FROM NODE 190.00 TO NODE 190.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.363
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .6000
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5749
SUBAREA AREA(ACRES) = 0.85    SUBAREA RUNOFF(CFS) = 2.74

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                                PRI00B.RES
TOTAL AREA(ACRES) =          2.4   TOTAL RUNOFF(CFS) =          7.43
TC(MIN.) =      12.10

*****
FLOW PROCESS FROM NODE      190.50 TO NODE      190.00 IS CODE =   81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =   5.363
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) =   88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5441
SUBAREA AREA(ACRES) =      0.79   SUBAREA RUNOFF(CFS) =      1.91
TOTAL AREA(ACRES) =      3.2     TOTAL RUNOFF(CFS) =      9.34
TC(MIN.) =      12.10

*****
FLOW PROCESS FROM NODE      190.00 TO NODE      191.00 IS CODE =   31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3047.22  DOWNSTREAM(FEET) = 3046.13
FLOW LENGTH(FEET) = 162.30   MANNING'S N = 0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.86
ESTIMATED PIPE DIAMETER(INCH) = 21.00   NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.34
PIPE TRAVEL TIME(MIN.) = 0.46   Tc(MIN.) = 12.57
LONGEST FLOWPATH FROM NODE      185.00 TO NODE      191.00 = 695.00 FEET.

*****
FLOW PROCESS FROM NODE      191.00 TO NODE      191.00 IS CODE =   81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =   5.235
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4900
S.C.S. CURVE NUMBER (AMC II) =   88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5268
SUBAREA AREA(ACRES) =      1.51   SUBAREA RUNOFF(CFS) =      3.87
TOTAL AREA(ACRES) =      4.7     TOTAL RUNOFF(CFS) =     12.99
TC(MIN.) =      12.57

*****
FLOW PROCESS FROM NODE      191.00 TO NODE      192.00 IS CODE =   31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3046.13  DOWNSTREAM(FEET) = 3044.57
FLOW LENGTH(FEET) = 215.70   MANNING'S N = 0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 16.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.37
ESTIMATED PIPE DIAMETER(INCH) = 21.00   NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.99
PIPE TRAVEL TIME(MIN.) = 0.56   Tc(MIN.) = 13.13
LONGEST FLOWPATH FROM NODE      185.00 TO NODE      192.00 = 910.70 FEET.

*****
FLOW PROCESS FROM NODE      192.00 TO NODE      192.00 IS CODE =   81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =   5.089
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5200
S.C.S. CURVE NUMBER (AMC II) =   88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5251
SUBAREA AREA(ACRES) =      1.51   SUBAREA RUNOFF(CFS) =      4.00
TOTAL AREA(ACRES) =      6.2     TOTAL RUNOFF(CFS) =     16.62
TC(MIN.) =      13.13

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PRI00B.RES

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*****
FLOW PROCESS FROM NODE      192.00 TO NODE      193.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3044.57  DOWNSTREAM(FEET) = 3042.53
FLOW LENGTH(FEET) = 211.30  MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.75
ESTIMATED PIPE DIAMETER(INCH) = 24.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 16.62
PIPE TRAVEL TIME(MIN.) = 0.45  Tc(MIN.) = 13.58
LONGEST FLOWPATH FROM NODE      185.00 TO NODE      193.00 = 1122.00 FEET.

*****
FLOW PROCESS FROM NODE      193.00 TO NODE      193.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.978
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4900
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5184
SUBAREA AREA(ACRES) = 1.46  SUBAREA RUNOFF(CFS) = 3.56
TOTAL AREA(ACRES) = 7.7  TOTAL RUNOFF(CFS) = 19.82
TC(MIN.) = 13.58

*****
FLOW PROCESS FROM NODE      193.00 TO NODE      184.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3042.53  DOWNSTREAM(FEET) = 3040.92
FLOW LENGTH(FEET) = 158.10  MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 17.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.17
ESTIMATED PIPE DIAMETER(INCH) = 24.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 19.82
PIPE TRAVEL TIME(MIN.) = 0.32  Tc(MIN.) = 13.91
LONGEST FLOWPATH FROM NODE      185.00 TO NODE      184.00 = 1280.10 FEET.

*****
FLOW PROCESS FROM NODE      184.00 TO NODE      184.00 IS CODE =  11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1         19.82    13.91     4.903         7.68
LONGEST FLOWPATH FROM NODE      185.00 TO NODE      184.00 = 1280.10 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM    RUNOFF    Tc    INTENSITY    AREA
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)    (ACRE)
1         38.71     9.65     6.207        11.48
LONGEST FLOWPATH FROM NODE      160.00 TO NODE      184.00 = 2744.20 FEET.

** PEAK FLOW RATE TABLE **
STREAM    RUNOFF    Tc    INTENSITY
NUMBER    (CFS)    (MIN.)    (INCH/HOUR)
1         52.46     9.65     6.207
2         50.40    13.91     4.903

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 52.46  Tc(MIN.) = 9.65
TOTAL AREA(ACRES) = 19.2

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                                PRI00B.RES
FLOW PROCESS FROM NODE      184.00 TO NODE      184.00 IS CODE =  12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====
*****
FLOW PROCESS FROM NODE      184.00 TO NODE      184.00 IS CODE =  10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====
*****
FLOW PROCESS FROM NODE      184.00 TO NODE      184.00 IS CODE =  13
-----
>>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<
=====
*****
FLOW PROCESS FROM NODE      194.00 TO NODE      195.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5300
S.C.S. CURVE NUMBER (AMC II) = 88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 66.80
UPSTREAM ELEVATION(FEET) = 3057.70
DOWNSTREAM ELEVATION(FEET) = 3056.90
ELEVATION DIFFERENCE(FEET) = 0.80
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 7.606
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
          THE MAXIMUM OVERLAND FLOW LENGTH = 61.98
          (Reference: Table 3-1B of Hydrology Manual)
          THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.237
SUBAREA RUNOFF(CFS) = 0.58
TOTAL AREA(ACRES) = 0.15 TOTAL RUNOFF(CFS) = 0.58
*****
FLOW PROCESS FROM NODE      195.00 TO NODE      196.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.90 DOWNSTREAM(FEET) = 3056.26
CHANNEL LENGTH THRU SUBAREA(FEET) = 53.00 CHANNEL SLOPE = 0.0121
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.792
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5300
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.99
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.12
AVERAGE FLOW DEPTH(FEET) = 0.15 TRAVEL TIME(MIN.) = 0.79
Tc(MIN.) = 8.39
SUBAREA AREA(ACRES) = 0.23 SUBAREA RUNOFF(CFS) = 0.83
AREA-AVERAGE RUNOFF COEFFICIENT = 0.530
TOTAL AREA(ACRES) = 0.4 PEAK FLOW RATE(CFS) = 1.37

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.17 FLOW VELOCITY(FEET/SEC.) = 1.25
LONGEST FLOWPATH FROM NODE 194.00 TO NODE 196.00 = 119.80 FEET.
*****
FLOW PROCESS FROM NODE      196.00 TO NODE      197.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.26 DOWNSTREAM(FEET) = 3056.14
CHANNEL LENGTH THRU SUBAREA(FEET) = 6.00 CHANNEL SLOPE = 0.0200
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 0.50

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                                PRI00B.RES
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.761
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5100
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.99
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(Feet/Sec.) = 1.67
AVERAGE FLOW DEPTH(Feet) = 0.18 TRAVEL TIME(MIN.) = 0.06
Tc(MIN.) = 8.45
SUBAREA AREA(ACRES) = 0.36 SUBAREA RUNOFF(CFS) = 1.24
AREA-AVERAGE RUNOFF COEFFICIENT = 0.520
TOTAL AREA(ACRES) = 0.7 PEAK FLOW RATE(CFS) = 2.60

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(Feet) = 0.20 FLOW VELOCITY(Feet/Sec.) = 1.81
LONGEST FLOWPATH FROM NODE 194.00 TO NODE 197.00 = 125.80 FEET.

*****
FLOW PROCESS FROM NODE 197.00 TO NODE 198.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(Feet) = 3051.64 DOWNSTREAM(Feet) = 3050.19
FLOW LENGTH(Feet) = 41.50 MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 4.4 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 7.70
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.60
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 8.54
LONGEST FLOWPATH FROM NODE 194.00 TO NODE 198.00 = 167.30 FEET.

*****
FLOW PROCESS FROM NODE 198.00 TO NODE 199.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(Feet) = 3050.19 DOWNSTREAM(Feet) = 3048.58
FLOW LENGTH(Feet) = 264.80 MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.0 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 4.11
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.60
PIPE TRAVEL TIME(MIN.) = 1.07 Tc(MIN.) = 9.61
LONGEST FLOWPATH FROM NODE 194.00 TO NODE 199.00 = 432.10 FEET.

*****
FLOW PROCESS FROM NODE 199.10 TO NODE 199.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.221
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .6100
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5685
SUBAREA AREA(ACRES) = 0.86 SUBAREA RUNOFF(CFS) = 3.26
TOTAL AREA(ACRES) = 1.6 TOTAL RUNOFF(CFS) = 5.66
TC(MIN.) = 9.61

*****
FLOW PROCESS FROM NODE 199.00 TO NODE 184.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(Feet) = 3048.58 DOWNSTREAM(Feet) = 3048.39
FLOW LENGTH(Feet) = 42.20 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 12.2 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 4.43
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.66

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                                PRI00B.RES
PIPE TRAVEL TIME(MIN.) = 0.16      Tc(MIN.) = 9.77
LONGEST FLOWPATH FROM NODE 194.00 TO NODE 184.00 = 474.30 FEET.

*****
FLOW PROCESS FROM NODE 184.00 TO NODE 184.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/ HOUR)  (ACRE)
1           5.66      9.77      6.156      1.60
LONGEST FLOWPATH FROM NODE 194.00 TO NODE 184.00 = 474.30 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/ HOUR)  (ACRE)
1           52.46      9.65      6.207      19.16
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 184.00 = 2744.20 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/ HOUR)
1           58.05      9.65      6.207
2           57.69      9.77      6.156

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 58.05      Tc(MIN.) = 9.65
TOTAL AREA(ACRES) = 20.8

*****
FLOW PROCESS FROM NODE 184.00 TO NODE 184.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE 184.00 TO NODE 199.20 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3040.92 DOWNSTREAM(FEET) = 3040.87
FLOW LENGTH(FEET) = 18.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 45.0 INCH PIPE IS 33.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.52
ESTIMATED PIPE DIAMETER(INCH) = 45.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 58.05
PIPE TRAVEL TIME(MIN.) = 0.05      Tc(MIN.) = 9.70
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 199.20 = 2762.20 FEET.

*****
FLOW PROCESS FROM NODE 199.20 TO NODE 199.30 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3034.55 DOWNSTREAM(FEET) = 3011.11
FLOW LENGTH(FEET) = 112.90 MANNING'S N = 0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 14.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 33.32
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 58.05
PIPE TRAVEL TIME(MIN.) = 0.06      Tc(MIN.) = 9.75
LONGEST FLOWPATH FROM NODE 160.00 TO NODE 199.30 = 2875.10 FEET.

*****
FLOW PROCESS FROM NODE 199.30 TO NODE 199.40 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3011.11 DOWNSTREAM(FEET) = 3011.00

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                                PRI00B.RES
FLOW LENGTH(FEET) =      24.80  MANNING'S N = 0.012
DEPTH OF FLOW IN  42.0 INCH PIPE IS  30.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    7.83
ESTIMATED PIPE DIAMETER(INCH) =  42.00  NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =      58.05
PIPE TRAVEL TIME(MIN.) =    0.05  Tc(MIN.) =    9.80
LONGEST FLOWPATH FROM NODE    160.00 TO NODE    199.40 =    2899.90 FEET.

*****
FLOW PROCESS FROM NODE    199.40 TO NODE    199.40 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  6.143
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) =  88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5181
SUBAREA AREA(ACRES) =    1.37  SUBAREA RUNOFF(CFS) =    3.37
TOTAL AREA(ACRES) =    22.1  TOTAL RUNOFF(CFS) =    70.43
TC(MIN.) =    9.80

*****
FLOW PROCESS FROM NODE    199.50 TO NODE    199.50 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  6.143
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) =  88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5163
SUBAREA AREA(ACRES) =    0.34  SUBAREA RUNOFF(CFS) =    0.84
TOTAL AREA(ACRES) =    22.5  TOTAL RUNOFF(CFS) =    71.27
TC(MIN.) =    9.80

=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES) =    22.5  TC(MIN.) =    9.80
PEAK FLOW RATE(CFS) =    71.27
=====
END OF RATIONAL METHOD ANALYSIS

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□

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
 (c) Copyright 1982-2008 Advanced Engineering Software (aes)
 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

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 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * 07-15-2009 *
 * PROPOSED HYDROLOGY BASIN 200 *
 * *

FILE NAME: PR200A.DAT
 TIME/DATE OF STUDY: 14:48 07/22/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP HIKE (FT) (FT) (FT)	MANNING FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313 0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
 1. Relative Flow-Depth = 0.00 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 208.00 TO NODE 210.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
 =====

*USER SPECIFIED(SUBAREA):
 URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4500
 S.C.S. CURVE NUMBER (AMC II) = 0
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3057.70
 DOWNSTREAM ELEVATION(FEET) = 3056.70
 ELEVATION DIFFERENCE(FEET) = 1.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.433
 WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
 THE MAXIMUM OVERLAND FLOW LENGTH = 65.00
 (Reference: Table 3-1B of Hydrology Manual)
 THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.298
 SUBAREA RUNOFF(CFS) = 0.31
 TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.31

 FLOW PROCESS FROM NODE 210.00 TO NODE 212.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

```

PR200A.RES
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.70 DOWNSTREAM(FEET) = 3056.09
CHANNEL LENGTH THRU SUBAREA(FEET) = 61.30 CHANNEL SLOPE = 0.0099
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.804
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.53
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.80
AVERAGE FLOW DEPTH(FEET) = 0.07 TRAVEL TIME(MIN.) = 1.27
Tc(MIN.) = 10.71
SUBAREA AREA(ACRES) = 0.17 SUBAREA RUNOFF(CFS) = 0.44
AREA-AVERAGE RUNOFF COEFFICIENT = 0.450
TOTAL AREA(ACRES) = 0.3 PEAK FLOW RATE(CFS) = 0.73

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.08 FLOW VELOCITY(FEET/SEC.) = 0.91
LONGEST FLOWPATH FROM NODE 208.00 TO NODE 212.00 = 161.30 FEET.

*****
FLOW PROCESS FROM NODE 212.00 TO NODE 218.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.09 DOWNSTREAM(FEET) = 3056.04
CHANNEL LENGTH THRU SUBAREA(FEET) = 5.00 CHANNEL SLOPE = 0.0100
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.781
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4400
S.C.S. CURVE NUMBER (AMC II) = 0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.71
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.26
AVERAGE FLOW DEPTH(FEET) = 0.13 TRAVEL TIME(MIN.) = 0.07
Tc(MIN.) = 10.77
SUBAREA AREA(ACRES) = 0.77 SUBAREA RUNOFF(CFS) = 1.96
AREA-AVERAGE RUNOFF COEFFICIENT = 0.443
TOTAL AREA(ACRES) = 1.0 PEAK FLOW RATE(CFS) = 2.69

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.17 FLOW VELOCITY(FEET/SEC.) = 1.50
LONGEST FLOWPATH FROM NODE 208.00 TO NODE 218.00 = 166.30 FEET.

*****
FLOW PROCESS FROM NODE 218.00 TO NODE 220.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3049.54 DOWNSTREAM(FEET) = 3047.87
FLOW LENGTH(FEET) = 277.80 MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.30
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.69
PIPE TRAVEL TIME(MIN.) = 1.08 Tc(MIN.) = 11.85
LONGEST FLOWPATH FROM NODE 208.00 TO NODE 220.00 = 444.10 FEET.

*****
FLOW PROCESS FROM NODE 220.00 TO NODE 220.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.436
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4400
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4414

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                                PR200A.RES
SUBAREA AREA(ACRES) =      0.92  SUBAREA RUNOFF(CFS) =      2.20
TOTAL AREA(ACRES) =      2.0    TOTAL RUNOFF(CFS) =      4.73
TC(MIN.) =      11.85

*****
FLOW PROCESS FROM NODE      220.00 TO NODE      220.00 IS CODE =  10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE      220.00 TO NODE      220.00 IS CODE =  13
-----
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<
=====

*****
FLOW PROCESS FROM NODE      224.00 TO NODE      226.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) =  0
INITIAL SUBAREA FLOW-LENGTH(FEET) =      18.30
UPSTREAM ELEVATION(FEET) =      3070.00
DOWNSTREAM ELEVATION(FEET) =      3058.00
ELEVATION DIFFERENCE(FEET) =      12.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =      2.502
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  9.485
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
SUBAREA RUNOFF(CFS) =      0.19
TOTAL AREA(ACRES) =      0.05  TOTAL RUNOFF(CFS) =      0.19

*****
FLOW PROCESS FROM NODE      226.00 TO NODE      228.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =      3058.00  DOWNSTREAM(FEET) =      3057.12
CHANNEL LENGTH THRU SUBAREA(FEET) =      80.60  CHANNEL SLOPE =  0.0109
CHANNEL BASE(FEET) =      2.00  "Z" FACTOR =  1.000
MANNING'S FACTOR =  0.015  MAXIMUM DEPTH(FEET) =  0.75
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  9.485
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4400
S.C.S. CURVE NUMBER (AMC II) =  0
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      0.38
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =  2.04
AVERAGE FLOW DEPTH(FEET) =  0.09  TRAVEL TIME(MIN.) =  0.66
Tc(MIN.) =  3.16
SUBAREA AREA(ACRES) =      0.09  SUBAREA RUNOFF(CFS) =      0.38
AREA-AVERAGE RUNOFF COEFFICIENT =  0.426
TOTAL AREA(ACRES) =      0.1    PEAK FLOW RATE(CFS) =      0.57

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =  0.12  FLOW VELOCITY(FEET/SEC.) =  2.28
LONGEST FLOWPATH FROM NODE      224.00 TO NODE      228.00 =      98.90 FEET.

*****
FLOW PROCESS FROM NODE      228.00 TO NODE      230.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =      3057.12  DOWNSTREAM(FEET) =      3056.53
CHANNEL LENGTH THRU SUBAREA(FEET) =      51.50  CHANNEL SLOPE =  0.0115
CHANNEL BASE(FEET) =      2.00  "Z" FACTOR =  1.000
MANNING'S FACTOR =  0.015  MAXIMUM DEPTH(FEET) =  0.75
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  9.485
NOTE: RAINFALL INTENSITY IS BASED ON Tc = 5-MINUTE.

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PR200A.RES

*USER SPECIFIED(SUBAREA):

GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500

S.C.S. CURVE NUMBER (AMC II) = 0

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.03

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(Feet/Sec.) = 2.90

AVERAGE FLOW DEPTH(Feet) = 0.16 TRAVEL TIME(Min.) = 0.30

Tc(Min.) = 3.46

SUBAREA AREA(ACRES) = 0.22 SUBAREA RUNOFF(CFS) = 0.94

AREA-AVERAGE RUNOFF COEFFICIENT = 0.441

TOTAL AREA(ACRES) = 0.4 PEAK FLOW RATE(CFS) = 1.50

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(Feet) = 0.20 FLOW VELOCITY(Feet/Sec.) = 3.36

LONGEST FLOWPATH FROM NODE 224.00 TO NODE 230.00 = 150.40 FEET.

 FLOW PROCESS FROM NODE 230.00 TO NODE 220.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

ELEVATION DATA: UPSTREAM(Feet) = 3049.85 DOWNSTREAM(Feet) = 3048.88

FLOW LENGTH(Feet) = 198.70 MANNING'S N = 0.012

ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000

DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.4 INCHES

PIPE-FLOW VELOCITY(Feet/Sec.) = 3.39

ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1

PIPE-FLOW(CFS) = 1.50

PIPE TRAVEL TIME(Min.) = 0.98 Tc(Min.) = 4.43

LONGEST FLOWPATH FROM NODE 224.00 TO NODE 220.00 = 349.10 FEET.

 FLOW PROCESS FROM NODE 220.00 TO NODE 220.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (Min.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.50	4.43	9.485	0.36

LONGEST FLOWPATH FROM NODE 224.00 TO NODE 220.00 = 349.10 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (Min.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	4.73	11.85	5.436	1.97

LONGEST FLOWPATH FROM NODE 208.00 TO NODE 220.00 = 444.10 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (Min.)	INTENSITY (INCH/HOUR)
1	3.27	4.43	9.485
2	5.59	11.85	5.436

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 5.59 Tc(Min.) = 11.85

TOTAL AREA(ACRES) = 2.3

 FLOW PROCESS FROM NODE 220.00 TO NODE 220.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<

 FLOW PROCESS FROM NODE 220.00 TO NODE 232.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

ELEVATION DATA: UPSTREAM(Feet) = 3047.87 DOWNSTREAM(Feet) = 3043.45

FLOW LENGTH(Feet) = 14.90 MANNING'S N = 0.012

ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000

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                                PR200A.RES
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 21.37
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.59
PIPE TRAVEL TIME(MIN.) = 0.01    Tc(MIN.) = 11.86
LONGEST FLOWPATH FROM NODE 208.00 TO NODE 232.00 = 459.00 FEET.

*****
FLOW PROCESS FROM NODE 232.00 TO NODE 234.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3043.45 DOWNSTREAM(FEET) = 3043.00
FLOW LENGTH(FEET) = 81.80 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.00
ESTIMATED PIPE DIAMETER(INCH) = 18.00    NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.59
PIPE TRAVEL TIME(MIN.) = 0.27    Tc(MIN.) = 12.13
LONGEST FLOWPATH FROM NODE 208.00 TO NODE 234.00 = 540.80 FEET.

*****
FLOW PROCESS FROM NODE 234.00 TO NODE 234.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.354
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4425
SUBAREA AREA(ACRES) = 0.36 SUBAREA RUNOFF(CFS) = 0.87
TOTAL AREA(ACRES) = 2.7 TOTAL RUNOFF(CFS) = 6.37
TC(MIN.) = 12.13
=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 2.7 TC(MIN.) = 12.13
PEAK FLOW RATE(CFS) = 6.37
=====
END OF RATIONAL METHOD ANALYSIS

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□

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
 (c) Copyright 1982-2008 Advanced Engineering Software (aes)
 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

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 15070 Avenue of Science
 Suite 100
 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 11-11-2009 *
 * FINAL HYDROLOGY: BASIN 200 OUTFALL *

FILE NAME: PR200B.DAT
 TIME/DATE OF STUDY: 11:50 11/11/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
 1. Relative Flow-Depth = 0.00 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 200.00 TO NODE 201.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3277.00
 DOWNSTREAM ELEVATION(FEET) = 3262.00
 ELEVATION DIFFERENCE(FEET) = 15.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.72
 TOTAL AREA(ACRES) = 0.25 TOTAL RUNOFF(CFS) = 0.72

 FLOW PROCESS FROM NODE 201.00 TO NODE 202.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3262.00 DOWNSTREAM(FEET) = 3207.00

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                                PR200B.RES
CHANNEL LENGTH THRU SUBAREA(FEET) = 148.00 CHANNEL SLOPE = 0.3716
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2090 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.72
FLOW VELOCITY(FEET/SEC) = 2.56 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.96 Tc(MIN.) = 7.23
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 202.00 = 248.00 FEET.

*****
FLOW PROCESS FROM NODE 201.00 TO NODE 202.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.477
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.63 SUBAREA RUNOFF(CFS) = 1.65
TOTAL AREA(ACRES) = 0.9 TOTAL RUNOFF(CFS) = 2.30
TC(MIN.) = 7.23

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3207.00 DOWNSTREAM(FEET) = 3170.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 215.00 CHANNEL SLOPE = 0.1721
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1460 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 2.30
FLOW VELOCITY(FEET/SEC) = 2.83 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.27 Tc(MIN.) = 8.50
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 203.00 = 463.00 FEET.

*****
FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.737
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 1.42 SUBAREA RUNOFF(CFS) = 3.35
TOTAL AREA(ACRES) = 2.3 TOTAL RUNOFF(CFS) = 5.42
TC(MIN.) = 8.50

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 204.00 IS CODE = 52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3170.00 DOWNSTREAM(FEET) = 3132.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 208.00 CHANNEL SLOPE = 0.1827
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 5.42
FLOW VELOCITY(FEET/SEC) = 6.80 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.51 Tc(MIN.) = 9.01
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 204.00 = 671.00 FEET.

*****
FLOW PROCESS FROM NODE 203.00 TO NODE 204.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.488
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"

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PR200B.RES

S.C.S. CURVE NUMBER (AMC II) = 88
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
 SUBAREA AREA(ACRES) = 2.36 SUBAREA RUNOFF(CFS) = 5.36
 TOTAL AREA(ACRES) = 4.7 TOTAL RUNOFF(CFS) = 10.58
 TC(MIN.) = 9.01

 FLOW PROCESS FROM NODE 204.00 TO NODE 205.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3132.00	DOWNSTREAM(FEET) =	3050.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	335.00	CHANNEL SLOPE =	0.2448
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION			
CHANNEL FLOW THRU SUBAREA(CFS) =	10.58		
FLOW VELOCITY(FEET/SEC) =	8.02	(PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)	
TRAVEL TIME(MIN.) =	0.70	Tc(MIN.) =	9.70
LONGEST FLOWPATH FROM NODE	200.00 TO NODE	205.00 =	1006.00 FEET.

 FLOW PROCESS FROM NODE 204.00 TO NODE 205.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	6.184
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500
SOIL CLASSIFICATION IS	"D"
S.C.S. CURVE NUMBER (AMC II) =	88
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3500
SUBAREA AREA(ACRES) =	10.68 SUBAREA RUNOFF(CFS) = 23.12
TOTAL AREA(ACRES) =	15.3 TOTAL RUNOFF(CFS) = 33.20
TC(MIN.) =	9.70

 FLOW PROCESS FROM NODE 205.00 TO NODE 206.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3050.00	DOWNSTREAM(FEET) =	3002.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	431.00	CHANNEL SLOPE =	0.1114
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION			
CHANNEL FLOW THRU SUBAREA(CFS) =	33.20		
FLOW VELOCITY(FEET/SEC) =	10.89	(PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)	
TRAVEL TIME(MIN.) =	0.66	Tc(MIN.) =	10.36
LONGEST FLOWPATH FROM NODE	200.00 TO NODE	206.00 =	1437.00 FEET.

 FLOW PROCESS FROM NODE 205.00 TO NODE 206.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	5.927
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500
SOIL CLASSIFICATION IS	"D"
S.C.S. CURVE NUMBER (AMC II) =	88
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3500
SUBAREA AREA(ACRES) =	26.24 SUBAREA RUNOFF(CFS) = 54.44
TOTAL AREA(ACRES) =	41.6 TOTAL RUNOFF(CFS) = 86.26
TC(MIN.) =	10.36

 FLOW PROCESS FROM NODE 206.00 TO NODE 207.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3002.00	DOWNSTREAM(FEET) =	2952.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	430.00	CHANNEL SLOPE =	0.1163
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION			
CHANNEL FLOW THRU SUBAREA(CFS) =	86.26		
FLOW VELOCITY(FEET/SEC) =	14.36	(PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)	
TRAVEL TIME(MIN.) =	0.50	Tc(MIN.) =	10.86

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                                PR200B.RES
LONGEST FLOWPATH FROM NODE    200.00 TO NODE    207.00 =    1867.00 FEET.

*****
FLOW PROCESS FROM NODE    206.00 TO NODE    207.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    5.750
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) =    14.23    SUBAREA RUNOFF(CFS) =    28.64
TOTAL AREA(ACRES) =    55.8    TOTAL RUNOFF(CFS) =    112.32
TC(MIN.) =    10.86

*****
FLOW PROCESS FROM NODE    207.00 TO NODE    207.00 IS CODE =    10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE    207.00 TO NODE    207.00 IS CODE =    13
-----
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<
=====

*****
FLOW PROCESS FROM NODE    236.00 TO NODE    236.00 IS CODE =    7
-----
>>>>USER SPECIFIED HYDROLOGY INFORMATION AT NODE<<<<
=====
USER-SPECIFIED VALUES ARE AS FOLLOWS:
TC(MIN) =    22.93    RAIN INTENSITY(INCH/HOUR) =    3.55
TOTAL AREA(ACRES) =    2.70    TOTAL RUNOFF(CFS) =    0.94

*****
FLOW PROCESS FROM NODE    236.00 TO NODE    207.00 IS CODE =    53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    3040.00    DOWNSTREAM(FEET) =    2952.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    231.00    CHANNEL SLOPE =    0.3810
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2107 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) =    0.94
FLOW VELOCITY(FEET/SEC) =    2.57 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    1.50    Tc(MIN.) =    24.43
LONGEST FLOWPATH FROM NODE    200.00 TO NODE    207.00 =    2098.00 FEET.

*****
FLOW PROCESS FROM NODE    207.00 TO NODE    207.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    3.409
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.1225
SUBAREA AREA(ACRES) =    0.29    SUBAREA RUNOFF(CFS) =    0.35
TOTAL AREA(ACRES) =    3.0    TOTAL RUNOFF(CFS) =    1.25
TC(MIN.) =    24.43

*****
FLOW PROCESS FROM NODE    207.00 TO NODE    207.00 IS CODE =    11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **

```

PR200B.RES

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.25	24.43	3.409	2.99

LONGEST FLOWPATH FROM NODE 200.00 TO NODE 207.00 = 2098.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	112.32	10.86	5.750	55.81

LONGEST FLOWPATH FROM NODE 200.00 TO NODE 207.00 = 1867.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	112.88	10.86	5.750
2	67.85	24.43	3.409

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 112.88 Tc(MIN.) = 10.86
 TOTAL AREA(ACRES) = 58.8

 FLOW PROCESS FROM NODE 207.00 TO NODE 250.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 2952.00 DOWNSTREAM(FEET) = 2540.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 1318.00 CHANNEL SLOPE = 0.3126
 NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
 CHANNEL FLOW THRU SUBAREA(CFS) = 112.88
 FLOW VELOCITY(FEET/SEC) = 15.57 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
 TRAVEL TIME(MIN.) = 1.41 Tc(MIN.) = 12.27
 LONGEST FLOWPATH FROM NODE 200.00 TO NODE 250.00 = 3416.00 FEET.

 FLOW PROCESS FROM NODE 207.00 TO NODE 250.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.315
 *USER SPECIFIED(SUBAREA):
 NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 S.C.S. CURVE NUMBER (AMC II) = 88
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.3425
 SUBAREA AREA(ACRES) = 32.20 SUBAREA RUNOFF(CFS) = 59.90
 TOTAL AREA(ACRES) = 91.0 TOTAL RUNOFF(CFS) = 165.66
 TC(MIN.) = 12.27

=====

END OF STUDY SUMMARY:
 TOTAL AREA(ACRES) = 91.0 TC(MIN.) = 12.27
 PEAK FLOW RATE(CFS) = 165.66

=====

END OF RATIONAL METHOD ANALYSIS

□

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
 (c) Copyright 1982-2008 Advanced Engineering Software (aes)
 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

Nolte Associates, Inc.
 15070 Avenue of Science
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 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 11-24-2009 *
 * FINAL HYDROLOGY: BASIN 300 OUTFALL *

FILE NAME: PR300A.DAT
 TIME/DATE OF STUDY: 09:56 11/24/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	11.0	5.5	0.020/0.020/0.020	0.08	0.10 0.0100	0.010	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
 1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 380.00 TO NODE 380.00 IS CODE = 7

>>>>USER SPECIFIED HYDROLOGY INFORMATION AT NODE<<<<

=====

USER-SPECIFIED VALUES ARE AS FOLLOWS:
 TC(MIN) = 20.52 RAIN INTENSITY(INCH/HOUR) = 3.82
 TOTAL AREA(ACRES) = 30.00 TOTAL RUNOFF(CFS) = 47.66

 FLOW PROCESS FROM NODE 380.00 TO NODE 382.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 2953.23 DOWNSTREAM(FEET) = 2915.05
 FLOW LENGTH(FEET) = 121.00 MANNING'S N = 0.012
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 8.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 36.32
 GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 47.66
 PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 20.58
 LONGEST FLOWPATH FROM NODE 0.00 TO NODE 382.00 = 121.00 FEET.

```

PR300A.RES
FLOW PROCESS FROM NODE      382.00 TO NODE      382.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  3.809
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) =  0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4162
SUBAREA AREA(ACRES) =  0.44 SUBAREA RUNOFF(CFS) =  0.67
TOTAL AREA(ACRES) =  30.4 TOTAL RUNOFF(CFS) =  48.25
TC(MIN.) =  20.58

*****
FLOW PROCESS FROM NODE      382.00 TO NODE      386.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 2915.05 DOWNSTREAM(FEET) = 2889.67
FLOW LENGTH(FEET) =  91.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 36.0 INCH PIPE IS 9.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 34.88
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =  48.25
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) =  20.62
LONGEST FLOWPATH FROM NODE      0.00 TO NODE      386.00 =  212.00 FEET.

*****
FLOW PROCESS FROM NODE      386.00 TO NODE      386.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  3.803
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) =  0
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4158
SUBAREA AREA(ACRES) =  0.70 SUBAREA RUNOFF(CFS) =  1.06
TOTAL AREA(ACRES) =  31.1 TOTAL RUNOFF(CFS) =  49.25
TC(MIN.) =  20.62

*****
FLOW PROCESS FROM NODE      386.00 TO NODE      392.00 IS CODE =  52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 2887.90 DOWNSTREAM(FEET) = 2868.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 138.00 CHANNEL SLOPE = 0.1442
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) =  49.25
FLOW VELOCITY(FEET/SEC) = 12.19 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.19 Tc(MIN.) =  20.81
LONGEST FLOWPATH FROM NODE      0.00 TO NODE      392.00 =  350.00 FEET.

*****
FLOW PROCESS FROM NODE      392.00 TO NODE      392.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  3.781
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =  88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3990
SUBAREA AREA(ACRES) =  10.69 SUBAREA RUNOFF(CFS) =  14.15
TOTAL AREA(ACRES) =  41.8 TOTAL RUNOFF(CFS) =  63.11
TC(MIN.) =  20.81

=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES) =  41.8 TC(MIN.) =  20.81
PEAK FLOW RATE(CFS) =  63.11
=====

```

PR300A.RES

=====

END OF RATIONAL METHOD ANALYSIS

□

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
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 (c) Copyright 1982-2008 Advanced Engineering Software (aes)
 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

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 15070 Avenue of Science
 Suite 100
 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 11-6-2009 *
 * FINAL HYDROLOGY *

FILE NAME: PR300B.DAT
 TIME/DATE OF STUDY: 10:25 11/06/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	11.0	5.5	0.020/0.020/0.020	0.08	0.10 0.0100	0.010	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
 1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 300.00 TO NODE 301.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

*USER SPECIFIED(SUBAREA):
 URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
 S.C.S. CURVE NUMBER (AMC II) = 94
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3190.00
 DOWNSTREAM ELEVATION(FEET) = 3060.00
 ELEVATION DIFFERENCE(FEET) = 130.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.849
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.573
 SUBAREA RUNOFF(CFS) = 0.14
 TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.14

 FLOW PROCESS FROM NODE 301.00 TO NODE 302.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3060.00 DOWNSTREAM(FEET) = 3053.40

```

                                PR300B.RES
CHANNEL LENGTH THRU SUBAREA(FEET) = 53.00 CHANNEL SLOPE = 0.1245
CHANNEL BASE(FEET) = 2.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.228
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4300
S.C.S. CURVE NUMBER (AMC II) = 94
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.33
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.30
AVERAGE FLOW DEPTH(FEET) = 0.06 TRAVEL TIME(MIN.) = 0.38
Tc(MIN.) = 6.23
SUBAREA AREA(ACRES) = 0.11 SUBAREA RUNOFF(CFS) = 0.39
AREA-AVERAGE RUNOFF COEFFICIENT = 0.422
TOTAL AREA(ACRES) = 0.2 PEAK FLOW RATE(CFS) = 0.52

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.07 FLOW VELOCITY(FEET/SEC.) = 2.59
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 302.00 = 153.00 FEET.

*****
FLOW PROCESS FROM NODE 302.00 TO NODE 303.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3053.40 DOWNSTREAM(FEET) = 3052.40
CHANNEL LENGTH THRU SUBAREA(FEET) = 195.10 CHANNEL SLOPE = 0.0051
CHANNEL BASE(FEET) = 2.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 10.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.280
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4100
S.C.S. CURVE NUMBER (AMC II) = 94
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.02
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.00
AVERAGE FLOW DEPTH(FEET) = 0.23 TRAVEL TIME(MIN.) = 3.24
Tc(MIN.) = 9.48
SUBAREA AREA(ACRES) = 0.38 SUBAREA RUNOFF(CFS) = 0.98
AREA-AVERAGE RUNOFF COEFFICIENT = 0.413
TOTAL AREA(ACRES) = 0.5 PEAK FLOW RATE(CFS) = 1.38

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.27 FLOW VELOCITY(FEET/SEC.) = 1.08
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 303.00 = 348.10 FEET.

*****
FLOW PROCESS FROM NODE 303.00 TO NODE 304.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3052.40 DOWNSTREAM(FEET) = 3050.81
FLOW LENGTH(FEET) = 68.80 MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.52
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.38
PIPE TRAVEL TIME(MIN.) = 0.21 Tc(MIN.) = 9.68
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 304.00 = 416.90 FEET.

*****
FLOW PROCESS FROM NODE 304.00 TO NODE 304.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.192
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 94
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4353
SUBAREA AREA(ACRES) = 0.79 SUBAREA RUNOFF(CFS) = 2.20
TOTAL AREA(ACRES) = 1.3 TOTAL RUNOFF(CFS) = 3.56
TC(MIN.) = 9.68

```



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*****
FLOW PROCESS FROM NODE      304.00 TO NODE      305.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3050.81  DOWNSTREAM(FEET) = 3050.65
FLOW LENGTH(FEET) = 83.70  MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 2.87
ESTIMATED PIPE DIAMETER(INCH) = 18.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.56
PIPE TRAVEL TIME(MIN.) = 0.49  Tc(MIN.) = 10.17
LONGEST FLOWPATH FROM NODE      300.00 TO NODE      305.00 = 500.60 FEET.

*****
FLOW PROCESS FROM NODE      305.00 TO NODE      305.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE      305.00 TO NODE      305.00 IS CODE = 13
-----
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<
=====

*****
FLOW PROCESS FROM NODE      305.20 TO NODE      305.40 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3302.00
DOWNSTREAM ELEVATION(FEET) = 3266.00
ELEVATION DIFFERENCE(FEET) = 36.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
SUBAREA RUNOFF(CFS) = 0.15
TOTAL AREA(ACRES) = 0.05  TOTAL RUNOFF(CFS) = 0.15

*****
FLOW PROCESS FROM NODE      305.40 TO NODE      305.60 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3266.00  DOWNSTREAM(FEET) = 3218.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 140.00  CHANNEL SLOPE = 0.3429
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2040 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.15
FLOW VELOCITY(FEET/SEC) = 2.53 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.92  Tc(MIN.) = 7.19
LONGEST FLOWPATH FROM NODE      305.20 TO NODE      305.60 = 240.00 FEET.

*****
FLOW PROCESS FROM NODE      305.40 TO NODE      305.40 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.504
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.14  SUBAREA RUNOFF(CFS) = 0.35
TOTAL AREA(ACRES) = 0.2  TOTAL RUNOFF(CFS) = 0.49

```

TC(MIN.) = 7.19

```

*****
FLOW PROCESS FROM NODE      305.60 TO NODE      306.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3218.00  DOWNSTREAM(FEET) =  3056.50
CHANNEL LENGTH THRU SUBAREA(FEET) =  412.00  CHANNEL SLOPE =  0.3920
CHANNEL BASE(FEET) =  8.00  "Z" FACTOR =  10.000
MANNING'S FACTOR = 0.030  MAXIMUM DEPTH(FEET) =  0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  6.495
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) =  88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =  1.57
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =  3.80
AVERAGE FLOW DEPTH(FEET) =  0.05  TRAVEL TIME(MIN.) =  1.80
Tc(MIN.) =  8.99
SUBAREA AREA(ACRES) =  0.83  SUBAREA RUNOFF(CFS) =  2.16
AREA-AVERAGE RUNOFF COEFFICIENT =  0.391
TOTAL AREA(ACRES) =  1.0  PEAK FLOW RATE(CFS) =  2.58

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =  0.06  FLOW VELOCITY(FEET/SEC.) =  4.68
LONGEST FLOWPATH FROM NODE      305.20 TO NODE      306.00 =  652.00 FEET.

*****
FLOW PROCESS FROM NODE      306.00 TO NODE      305.00 IS CODE =  41
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3051.50  DOWNSTREAM(FEET) =  3050.65
FLOW LENGTH(FEET) =  24.70  MANNING'S N =  0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS  4.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  7.64
GIVEN PIPE DIAMETER(INCH) =  18.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =  2.58
PIPE TRAVEL TIME(MIN.) =  0.05  Tc(MIN.) =  9.05
LONGEST FLOWPATH FROM NODE      305.20 TO NODE      305.00 =  676.70 FEET.

*****
FLOW PROCESS FROM NODE      305.00 TO NODE      305.00 IS CODE =  11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1           2.58       9.05      6.470      1.02
LONGEST FLOWPATH FROM NODE      305.20 TO NODE      305.00 =  676.70 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1           3.56      10.17      6.000      1.32
LONGEST FLOWPATH FROM NODE      300.00 TO NODE      305.00 =  500.60 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)
1           5.75       9.05      6.470
2           5.95      10.17      6.000

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =  5.95  Tc(MIN.) =  10.17
TOTAL AREA(ACRES) =  2.3

*****
FLOW PROCESS FROM NODE      305.00 TO NODE      305.00 IS CODE =  12
-----

```

>>>>CLEAR MEMORY BANK # 1 <<<<

```
*****
FLOW PROCESS FROM NODE      305.00 TO NODE      307.00 IS CODE =  31
-----
```

```
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
```

```
ELEVATION DATA: UPSTREAM(FEET) = 3050.65  DOWNSTREAM(FEET) = 3049.49
FLOW LENGTH(FEET) = 122.80  MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 9.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.99
ESTIMATED PIPE DIAMETER(INCH) = 18.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.95
PIPE TRAVEL TIME(MIN.) = 0.34  Tc(MIN.) = 10.51
LONGEST FLOWPATH FROM NODE      305.20 TO NODE      307.00 = 799.50 FEET.
```

```
*****
FLOW PROCESS FROM NODE      307.00 TO NODE      307.00 IS CODE =  81
-----
```

```
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
```

```
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.873
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4700
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4295
SUBAREA AREA(ACRES) = 0.78  SUBAREA RUNOFF(CFS) = 2.15
TOTAL AREA(ACRES) = 3.1  TOTAL RUNOFF(CFS) = 7.86
TC(MIN.) = 10.51
```

```
*****
FLOW PROCESS FROM NODE      307.00 TO NODE      308.00 IS CODE =  31
-----
```

```
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
```

```
ELEVATION DATA: UPSTREAM(FEET) = 3049.49  DOWNSTREAM(FEET) = 3048.21
FLOW LENGTH(FEET) = 162.50  MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 12.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.91
ESTIMATED PIPE DIAMETER(INCH) = 18.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.86
PIPE TRAVEL TIME(MIN.) = 0.46  Tc(MIN.) = 10.97
LONGEST FLOWPATH FROM NODE      305.20 TO NODE      308.00 = 962.00 FEET.
```

```
*****
FLOW PROCESS FROM NODE      308.00 TO NODE      308.00 IS CODE =  81
-----
```

```
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
```

```
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.714
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5600
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4600
SUBAREA AREA(ACRES) = 0.95  SUBAREA RUNOFF(CFS) = 3.04
TOTAL AREA(ACRES) = 4.1  TOTAL RUNOFF(CFS) = 10.69
TC(MIN.) = 10.97
```

```
*****
FLOW PROCESS FROM NODE      308.50 TO NODE      308.00 IS CODE =  81
-----
```

```
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
```

```
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.714
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5500
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4729
SUBAREA AREA(ACRES) = 0.68  SUBAREA RUNOFF(CFS) = 2.14
TOTAL AREA(ACRES) = 4.7  TOTAL RUNOFF(CFS) = 12.83
```

TC(MIN.) = 10.97

```

*****
FLOW PROCESS FROM NODE      308.00 TO NODE      309.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3048.21  DOWNSTREAM(FEET) = 3045.78
FLOW LENGTH(FEET) = 290.00  MANNING'S N = 0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 15.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.80
ESTIMATED PIPE DIAMETER(INCH) = 21.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.83
PIPE TRAVEL TIME(MIN.) = 0.71  Tc(MIN.) = 11.68
LONGEST FLOWPATH FROM NODE      305.20 TO NODE      309.00 = 1252.00 FEET.

*****
FLOW PROCESS FROM NODE      309.00 TO NODE      309.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.487
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5300
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4847
SUBAREA AREA(ACRES) = 1.24  SUBAREA RUNOFF(CFS) = 3.61
TOTAL AREA(ACRES) = 6.0  TOTAL RUNOFF(CFS) = 15.92
TC(MIN.) = 11.68

*****
FLOW PROCESS FROM NODE      309.00 TO NODE      309.00 IS CODE =  10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE      309.00 TO NODE      309.00 IS CODE =  13
-----
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<
=====

*****
FLOW PROCESS FROM NODE      310.00 TO NODE      311.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3057.70
DOWNSTREAM ELEVATION(FEET) = 3056.70
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.063
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
          THE MAXIMUM OVERLAND FLOW LENGTH = 60.00
          (Reference: Table 3-1B of Hydrology Manual)
          THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.463
SUBAREA RUNOFF(CFS) = 0.32
TOTAL AREA(ACRES) = 0.11  TOTAL RUNOFF(CFS) = 0.32

*****
FLOW PROCESS FROM NODE      311.00 TO NODE      312.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.70  DOWNSTREAM(FEET) = 3056.20
CHANNEL LENGTH THRU SUBAREA(FEET) = 44.00  CHANNEL SLOPE = 0.0114
CHANNEL BASE(FEET) = 3.00  "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030  MAXIMUM DEPTH(FEET) = 0.50

```

```

                                PR300B.RES
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.121
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.49
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(Feet/Sec.) = 0.92
AVERAGE FLOW DEPTH(Feet) = 0.10 TRAVEL TIME(MIN.) = 0.80
Tc(MIN.) = 9.86
SUBAREA AREA(ACRES) = 0.12 SUBAREA RUNOFF(CFS) = 0.33
AREA-AVERAGE RUNOFF COEFFICIENT = 0.450
TOTAL AREA(ACRES) = 0.2 PEAK FLOW RATE(CFS) = 0.63

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(Feet) = 0.12 FLOW VELOCITY(Feet/Sec.) = 0.99
LONGEST FLOWPATH FROM NODE 310.00 TO NODE 312.00 = 144.00 FEET.

*****
FLOW PROCESS FROM NODE 312.00 TO NODE 313.00 IS CODE = 51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(Feet) = 3056.20 DOWNSTREAM(Feet) = 3055.93
CHANNEL LENGTH THRU SUBAREA(Feet) = 11.40 CHANNEL SLOPE = 0.0237
CHANNEL BASE(Feet) = 3.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(Feet) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.080
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5100
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.18
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(Feet/Sec.) = 1.83
AVERAGE FLOW DEPTH(Feet) = 0.18 TRAVEL TIME(MIN.) = 0.10
Tc(MIN.) = 9.96
SUBAREA AREA(ACRES) = 1.00 SUBAREA RUNOFF(CFS) = 3.10
AREA-AVERAGE RUNOFF COEFFICIENT = 0.499
TOTAL AREA(ACRES) = 1.2 PEAK FLOW RATE(CFS) = 3.73

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(Feet) = 0.23 FLOW VELOCITY(Feet/Sec.) = 2.11
LONGEST FLOWPATH FROM NODE 310.00 TO NODE 313.00 = 155.40 FEET.

*****
FLOW PROCESS FROM NODE 313.00 TO NODE 309.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(Feet) = 3049.43 DOWNSTREAM(Feet) = 3046.47
FLOW LENGTH(Feet) = 216.20 MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.8 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 6.09
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.73
PIPE TRAVEL TIME(MIN.) = 0.59 Tc(MIN.) = 10.56
LONGEST FLOWPATH FROM NODE 310.00 TO NODE 309.00 = 371.60 FEET.

*****
FLOW PROCESS FROM NODE 309.00 TO NODE 309.00 IS CODE = 11
-----
>>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 3.73 10.56 5.858 1.23
LONGEST FLOWPATH FROM NODE 310.00 TO NODE 309.00 = 371.60 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 15.92 11.68 5.487 5.99

```

```

                                PR300B.RES
LONGEST FLOWPATH FROM NODE    305.20 TO NODE    309.00 =    1252.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)
  1         18.12      10.56      5.858
  2         19.42      11.68      5.487

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) =      19.42    Tc(MIN.) =    11.68
TOTAL AREA(ACRES) =      7.2

*****
FLOW PROCESS FROM NODE    309.00 TO NODE    309.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 1 <<<<<
=====

*****
FLOW PROCESS FROM NODE    309.00 TO NODE    314.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3045.78 DOWNSTREAM(FEET) = 3044.19
FLOW LENGTH(FEET) = 260.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 27.0 INCH PIPE IS 18.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.76
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 19.42
PIPE TRAVEL TIME(MIN.) = 0.64 Tc(MIN.) = 12.32
LONGEST FLOWPATH FROM NODE    305.20 TO NODE    314.00 =    1512.00 FEET.

*****
FLOW PROCESS FROM NODE    314.00 TO NODE    314.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.301
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5400
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4948
SUBAREA AREA(ACRES) = 1.23 SUBAREA RUNOFF(CFS) = 3.52
TOTAL AREA(ACRES) = 8.4 TOTAL RUNOFF(CFS) = 22.16
TC(MIN.) = 12.32

*****
FLOW PROCESS FROM NODE    314.00 TO NODE    314.00 IS CODE = 10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
=====

*****
FLOW PROCESS FROM NODE    315.00 TO NODE    316.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3057.70
DOWNSTREAM ELEVATION(FEET) = 3056.70
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.063
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
          THE MAXIMUM OVERLAND FLOW LENGTH = 60.00
          (Reference: Table 3-1B of Hydrology Manual)
          THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.463
SUBAREA RUNOFF(CFS) = 0.32
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.32

```

PR300B.RES

 FLOW PROCESS FROM NODE 316.00 TO NODE 317.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 3056.70 DOWNSTREAM(FEET) = 3056.20
 CHANNEL LENGTH THRU SUBAREA(FEET) = 45.69 CHANNEL SLOPE = 0.0109
 CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 20.000
 MANNING'S FACTOR = 0.012 MAXIMUM DEPTH(FEET) = 0.50
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.273
 *USER SPECIFIED(SUBAREA):
 GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
 S.C.S. CURVE NUMBER (AMC II) = 88
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.50
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.77
 AVERAGE FLOW DEPTH(FEET) = 0.07 TRAVEL TIME(MIN.) = 0.43
 Tc(MIN.) = 9.49
 SUBAREA AREA(ACRES) = 0.13 SUBAREA RUNOFF(CFS) = 0.37
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.450
 TOTAL AREA(ACRES) = 0.2 PEAK FLOW RATE(CFS) = 0.68

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.08 FLOW VELOCITY(FEET/SEC.) = 1.87
 LONGEST FLOWPATH FROM NODE 315.00 TO NODE 317.00 = 145.69 FEET.

 FLOW PROCESS FROM NODE 317.00 TO NODE 318.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 3056.20 DOWNSTREAM(FEET) = 3056.06
 CHANNEL LENGTH THRU SUBAREA(FEET) = 17.80 CHANNEL SLOPE = 0.0079
 CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 20.000
 MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 0.50
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.170
 *USER SPECIFIED(SUBAREA):
 GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
 S.C.S. CURVE NUMBER (AMC II) = 88
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.04
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.20
 AVERAGE FLOW DEPTH(FEET) = 0.23 TRAVEL TIME(MIN.) = 0.25
 Tc(MIN.) = 9.74
 SUBAREA AREA(ACRES) = 0.98 SUBAREA RUNOFF(CFS) = 2.72
 AREA-AVERAGE RUNOFF COEFFICIENT = 0.450
 TOTAL AREA(ACRES) = 1.2 PEAK FLOW RATE(CFS) = 3.39

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.29 FLOW VELOCITY(FEET/SEC.) = 1.36
 LONGEST FLOWPATH FROM NODE 315.00 TO NODE 318.00 = 163.49 FEET.

 FLOW PROCESS FROM NODE 318.00 TO NODE 319.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 3049.50 DOWNSTREAM(FEET) = 3048.10
 FLOW LENGTH(FEET) = 216.30 MANNING'S N = 0.012
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.51
 ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 3.39
 PIPE TRAVEL TIME(MIN.) = 0.80 Tc(MIN.) = 10.54
 LONGEST FLOWPATH FROM NODE 315.00 TO NODE 319.00 = 379.79 FEET.

 FLOW PROCESS FROM NODE 319.00 TO NODE 319.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
 =====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.864

```

*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4500
SUBAREA AREA(ACRES) = 1.22 SUBAREA RUNOFF(CFS) = 3.22
TOTAL AREA(ACRES) = 2.4 TOTAL RUNOFF(CFS) = 6.44
TC(MIN.) = 10.54

```

```

*****
FLOW PROCESS FROM NODE 320.00 TO NODE 319.00 IS CODE = 81
-----

```

```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====

```

```

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.864
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4500
SUBAREA AREA(ACRES) = 1.17 SUBAREA RUNOFF(CFS) = 3.09
TOTAL AREA(ACRES) = 3.6 TOTAL RUNOFF(CFS) = 9.53
TC(MIN.) = 10.54

```

```

*****
FLOW PROCESS FROM NODE 319.00 TO NODE 314.00 IS CODE = 31
-----

```

```

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====

```

```

ELEVATION DATA: UPSTREAM(FEET) = 3048.10 DOWNSTREAM(FEET) = 3044.19
FLOW LENGTH(FEET) = 268.70 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.87
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.53
PIPE TRAVEL TIME(MIN.) = 0.57 Tc(MIN.) = 11.11
LONGEST FLOWPATH FROM NODE 315.00 TO NODE 314.00 = 648.49 FEET.

```

```

*****
FLOW PROCESS FROM NODE 314.00 TO NODE 314.00 IS CODE = 11
-----

```

```

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

```

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	9.53	11.11	5.668	3.61

LONGEST FLOWPATH FROM NODE 315.00 TO NODE 314.00 = 648.49 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	22.16	12.32	5.301	8.45

LONGEST FLOWPATH FROM NODE 305.20 TO NODE 314.00 = 1512.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	29.50	11.11	5.668
2	31.07	12.32	5.301

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 31.07 Tc(MIN.) = 12.32
 TOTAL AREA(ACRES) = 12.1

```

*****
FLOW PROCESS FROM NODE 314.00 TO NODE 314.00 IS CODE = 12
-----

```

```

>>>>CLEAR MEMORY BANK # 1 <<<<
=====

```

```

*****
FLOW PROCESS FROM NODE 314.00 TO NODE 314.00 IS CODE = 10
-----

```



```

                                PR300B.RES
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====
*****
FLOW PROCESS FROM NODE      314.00 TO NODE      314.00 IS CODE =  13
-----
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<
=====
*****
FLOW PROCESS FROM NODE      322.00 TO NODE      323.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3057.70
DOWNSTREAM ELEVATION(FEET) = 3056.70
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.063
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
          THE MAXIMUM OVERLAND FLOW LENGTH = 60.00
          (Reference: Table 3-1B of Hydrology Manual)
          THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.463
SUBAREA RUNOFF(CFS) = 0.32
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.32
*****
FLOW PROCESS FROM NODE      323.00 TO NODE      324.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.70 DOWNSTREAM(FEET) = 3056.20
CHANNEL LENGTH THRU SUBAREA(FEET) = 44.00 CHANNEL SLOPE = 0.0114
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.121
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.49
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.92
AVERAGE FLOW DEPTH(FEET) = 0.10 TRAVEL TIME(MIN.) = 0.80
Tc(MIN.) = 9.86
SUBAREA AREA(ACRES) = 0.12 SUBAREA RUNOFF(CFS) = 0.33
AREA-AVERAGE RUNOFF COEFFICIENT = 0.450
TOTAL AREA(ACRES) = 0.2 PEAK FLOW RATE(CFS) = 0.63

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.12 FLOW VELOCITY(FEET/SEC.) = 0.99
LONGEST FLOWPATH FROM NODE 322.00 TO NODE 324.00 = 144.00 FEET.
*****
FLOW PROCESS FROM NODE      324.00 TO NODE      325.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.20 DOWNSTREAM(FEET) = 3055.92
CHANNEL LENGTH THRU SUBAREA(FEET) = 11.70 CHANNEL SLOPE = 0.0239
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.079
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5100
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.18
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.83
AVERAGE FLOW DEPTH(FEET) = 0.18 TRAVEL TIME(MIN.) = 0.11
Tc(MIN.) = 9.97

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```

PR300B.RES
SUBAREA AREA(ACRES) = 1.00 SUBAREA RUNOFF(CFS) = 3.10
AREA-AVERAGE RUNOFF COEFFICIENT = 0.499
TOTAL AREA(ACRES) = 1.2 PEAK FLOW RATE(CFS) = 3.73

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(Feet) = 0.23 FLOW VELOCITY(Feet/Sec.) = 2.11
LONGEST FLOWPATH FROM NODE 322.00 TO NODE 325.00 = 155.70 FEET.

*****
FLOW PROCESS FROM NODE 325.00 TO NODE 314.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(Feet) = 3049.42 DOWNSTREAM(Feet) = 3044.19
FLOW LENGTH(Feet) = 215.70 MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.8 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 7.49
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.73
PIPE TRAVEL TIME(Min.) = 0.48 Tc(Min.) = 10.45
LONGEST FLOWPATH FROM NODE 322.00 TO NODE 314.00 = 371.40 FEET.

*****
FLOW PROCESS FROM NODE 314.00 TO NODE 314.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (Min.) (Inch/Hour) (Acre)
1 3.73 10.45 5.897 1.23
LONGEST FLOWPATH FROM NODE 322.00 TO NODE 314.00 = 371.40 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (Min.) (Inch/Hour) (Acre)
1 31.07 12.32 5.301 12.06
LONGEST FLOWPATH FROM NODE 305.20 TO NODE 314.00 = 1512.00 FEET.

** PEAK FLOW RATE TABLE **
STREAM RUNOFF Tc INTENSITY
NUMBER (CFS) (Min.) (Inch/Hour)
1 30.07 10.45 5.897
2 34.42 12.32 5.301

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 34.42 Tc(Min.) = 12.32
TOTAL AREA(ACRES) = 13.3

*****
FLOW PROCESS FROM NODE 314.00 TO NODE 314.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE 314.00 TO NODE 326.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(Feet) = 3044.19 DOWNSTREAM(Feet) = 3041.87
FLOW LENGTH(Feet) = 239.50 MANNING'S N = 0.012
DEPTH OF FLOW IN 30.0 INCH PIPE IS 21.3 INCHES
PIPE-FLOW VELOCITY(Feet/Sec.) = 9.23
ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 34.42
PIPE TRAVEL TIME(Min.) = 0.43 Tc(Min.) = 12.75
LONGEST FLOWPATH FROM NODE 305.20 TO NODE 326.00 = 1751.50 FEET.

*****

```

```

PR300B.RES
FLOW PROCESS FROM NODE      326.00 TO NODE      326.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  5.185
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5700
S.C.S. CURVE NUMBER (AMC II) =  88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4878
SUBAREA AREA(ACRES) =  0.77   SUBAREA RUNOFF(CFS) =  2.28
TOTAL AREA(ACRES) =  14.1   TOTAL RUNOFF(CFS) =  35.55
TC(MIN.) =  12.75

*****
FLOW PROCESS FROM NODE      326.00 TO NODE      326.00 IS CODE =  10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====

*****
FLOW PROCESS FROM NODE      326.00 TO NODE      326.00 IS CODE =  13
-----
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<
=====

*****
FLOW PROCESS FROM NODE      328.00 TO NODE      329.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) =  88
INITIAL SUBAREA FLOW-LENGTH(FEET) =  98.00
UPSTREAM ELEVATION(FEET) =  3134.00
DOWNSTREAM ELEVATION(FEET) =  3088.79
ELEVATION DIFFERENCE(FEET) =  45.21
SUBAREA OVERLAND TIME OF FLOW(MIN.) =  5.790
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  8.629
SUBAREA RUNOFF(CFS) =  0.35
TOTAL AREA(ACRES) =  0.10   TOTAL RUNOFF(CFS) =  0.35

*****
FLOW PROCESS FROM NODE      329.00 TO NODE      330.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  3088.79   DOWNSTREAM(FEET) =  307.82
CHANNEL LENGTH THRU SUBAREA(FEET) =  110.80   CHANNEL SLOPE = 25.0990
CHANNEL BASE(FEET) =  2.00   "Z" FACTOR =  1.000
MANNING'S FACTOR = 0.015   MAXIMUM DEPTH(FEET) =  1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  8.571
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) =  88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =  0.76
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =  30.32
AVERAGE FLOW DEPTH(FEET) =  0.01   TRAVEL TIME(MIN.) =  0.06
Tc(MIN.) =  5.85
SUBAREA AREA(ACRES) =  0.24   SUBAREA RUNOFF(CFS) =  0.82
AREA-AVERAGE RUNOFF COEFFICIENT = 0.400
TOTAL AREA(ACRES) =  0.3   PEAK FLOW RATE(CFS) =  1.17

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =  0.02   FLOW VELOCITY(FEET/SEC.) =  31.88
LONGEST FLOWPATH FROM NODE      328.00 TO NODE      330.00 =  208.80 FEET.

*****
FLOW PROCESS FROM NODE      330.00 TO NODE      331.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

```

```

=====
ELEVATION DATA: UPSTREAM(FEET) = 3085.02  DOWNSTREAM(FEET) = 3050.52
FLOW LENGTH(FEET) = 59.00  MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.37
ESTIMATED PIPE DIAMETER(INCH) = 18.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.17
PIPE TRAVEL TIME(MIN.) = 0.06  Tc(MIN.) = 5.91
LONGEST FLOWPATH FROM NODE 328.00 TO NODE 331.00 = 267.80 FEET.

```

```

*****
FLOW PROCESS FROM NODE 331.00 TO NODE 326.00 IS CODE = 31
-----

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```

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====

```

```

ELEVATION DATA: UPSTREAM(FEET) = 3050.52  DOWNSTREAM(FEET) = 3041.87
FLOW LENGTH(FEET) = 21.10  MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 1.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.43
ESTIMATED PIPE DIAMETER(INCH) = 18.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.17
PIPE TRAVEL TIME(MIN.) = 0.02  Tc(MIN.) = 5.94
LONGEST FLOWPATH FROM NODE 328.00 TO NODE 326.00 = 288.90 FEET.

```

```

*****
FLOW PROCESS FROM NODE 326.00 TO NODE 326.00 IS CODE = 11
-----

```

```

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

```

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.17	5.94	8.492	0.34

LONGEST FLOWPATH FROM NODE 328.00 TO NODE 326.00 = 288.90 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	35.55	12.75	5.185	14.06

LONGEST FLOWPATH FROM NODE 305.20 TO NODE 326.00 = 1751.50 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	17.71	5.94	8.492
2	36.26	12.75	5.185

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 36.26 Tc(MIN.) = 12.75
TOTAL AREA(ACRES) = 14.4

```

*****
FLOW PROCESS FROM NODE 326.00 TO NODE 326.00 IS CODE = 12
-----

```

```

>>>>CLEAR MEMORY BANK # 1 <<<<
=====

```

```

*****
FLOW PROCESS FROM NODE 326.00 TO NODE 332.00 IS CODE = 31
-----

```

```

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====

```

```

ELEVATION DATA: UPSTREAM(FEET) = 3041.87  DOWNSTREAM(FEET) = 3040.79
FLOW LENGTH(FEET) = 216.00  MANNING'S N = 0.012
DEPTH OF FLOW IN 33.0 INCH PIPE IS 26.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.17
ESTIMATED PIPE DIAMETER(INCH) = 33.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 36.26
PIPE TRAVEL TIME(MIN.) = 0.50  Tc(MIN.) = 13.26

```

```

                                PR300B.RES
LONGEST FLOWPATH FROM NODE    305.20 TO NODE    332.00 =    1967.50 FEET.

*****
FLOW PROCESS FROM NODE    332.00 TO NODE    332.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    5.057
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5100
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4869
SUBAREA AREA(ACRES) =    0.78    SUBAREA RUNOFF(CFS) =    2.01
TOTAL AREA(ACRES) =    15.2    TOTAL RUNOFF(CFS) =    37.37
TC(MIN.) =    13.26

*****
FLOW PROCESS FROM NODE    332.00 TO NODE    333.00 IS CODE =    31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3040.79    DOWNSTREAM(FEET) = 3040.43
FLOW LENGTH(FEET) =    75.80    MANNING'S N =    0.012
DEPTH OF FLOW IN 36.0 INCH PIPE IS 24.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    7.24
ESTIMATED PIPE DIAMETER(INCH) = 36.00    NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =    37.37
PIPE TRAVEL TIME(MIN.) =    0.17    Tc(MIN.) =    13.43
LONGEST FLOWPATH FROM NODE    305.20 TO NODE    333.00 =    2043.30 FEET.

*****
FLOW PROCESS FROM NODE    333.00 TO NODE    335.00 IS CODE =    31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3040.43    DOWNSTREAM(FEET) = 3039.83
FLOW LENGTH(FEET) =    120.00    MANNING'S N =    0.012
DEPTH OF FLOW IN 36.0 INCH PIPE IS 24.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    7.39
ESTIMATED PIPE DIAMETER(INCH) = 36.00    NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =    37.37
PIPE TRAVEL TIME(MIN.) =    0.27    Tc(MIN.) =    13.70
LONGEST FLOWPATH FROM NODE    305.20 TO NODE    335.00 =    2163.30 FEET.

*****
FLOW PROCESS FROM NODE    335.00 TO NODE    335.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    4.950
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5400
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4888
SUBAREA AREA(ACRES) =    0.55    SUBAREA RUNOFF(CFS) =    1.47
TOTAL AREA(ACRES) =    15.7    TOTAL RUNOFF(CFS) =    38.06
TC(MIN.) =    13.70

*****
FLOW PROCESS FROM NODE    335.00 TO NODE    336.00 IS CODE =    31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3039.83    DOWNSTREAM(FEET) = 3038.68
FLOW LENGTH(FEET) =    174.10    MANNING'S N =    0.012
DEPTH OF FLOW IN 33.0 INCH PIPE IS 24.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    8.16
ESTIMATED PIPE DIAMETER(INCH) = 33.00    NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =    38.06
PIPE TRAVEL TIME(MIN.) =    0.36    Tc(MIN.) =    14.06
LONGEST FLOWPATH FROM NODE    305.20 TO NODE    336.00 =    2337.40 FEET.

```

```

*****
FLOW PROCESS FROM NODE      336.00 TO NODE      336.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
  100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  4.869
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5500
S.C.S. CURVE NUMBER (AMC II) =  88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4905
SUBAREA AREA(ACRES) =    0.46  SUBAREA RUNOFF(CFS) =    1.23
TOTAL AREA(ACRES) =    16.2  TOTAL RUNOFF(CFS) =    38.66
TC(MIN.) =    14.06
*****
FLOW PROCESS FROM NODE      336.00 TO NODE      337.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3038.68  DOWNSTREAM(FEET) = 3037.72
FLOW LENGTH(FEET) = 136.00  MANNING'S N = 0.012
DEPTH OF FLOW IN 33.0 INCH PIPE IS 23.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =  8.42
ESTIMATED PIPE DIAMETER(INCH) = 33.00  NUMBER OF PIPES =  1
PIPE-FLOW(CFS) =    38.66
PIPE TRAVEL TIME(MIN.) =  0.27  Tc(MIN.) =  14.33
LONGEST FLOWPATH FROM NODE    305.20 TO NODE    337.00 =    2473.40 FEET.
*****
FLOW PROCESS FROM NODE      337.00 TO NODE      337.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
  100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  4.810
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5400
S.C.S. CURVE NUMBER (AMC II) =  88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4915
SUBAREA AREA(ACRES) =    0.33  SUBAREA RUNOFF(CFS) =    0.86
TOTAL AREA(ACRES) =    16.5  TOTAL RUNOFF(CFS) =    39.05
TC(MIN.) =    14.33
*****
FLOW PROCESS FROM NODE      337.00 TO NODE      337.00 IS CODE =  10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<
=====
*****
FLOW PROCESS FROM NODE      337.00 TO NODE      337.00 IS CODE =  13
-----
>>>>CLEAR THE MAIN-STREAM MEMORY<<<<
=====
*****
FLOW PROCESS FROM NODE      345.00 TO NODE      346.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
=====
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) =  88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3057.70
DOWNSTREAM ELEVATION(FEET) = 3056.70
ELEVATION DIFFERENCE(FEET) =  1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =  9.063
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
         THE MAXIMUM OVERLAND FLOW LENGTH =  60.00
         (Reference: Table 3-1B of Hydrology Manual)
         THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  6.463

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                                PR300B.RES
SUBAREA RUNOFF(CFS) =          0.32
TOTAL AREA(ACRES) =          0.11  TOTAL RUNOFF(CFS) =          0.32

*****
FLOW PROCESS FROM NODE      346.00 TO NODE      347.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   3056.70  DOWNSTREAM(FEET) =   3056.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    68.60  CHANNEL SLOPE =   0.0102
CHANNEL BASE(FEET) =    3.00  "Z" FACTOR =   20.000
MANNING'S FACTOR = 0.030  MAXIMUM DEPTH(FEET) =    0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =   5.958
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) =   88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =          0.60
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    0.94
AVERAGE FLOW DEPTH(FEET) =    0.12  TRAVEL TIME(MIN.) =    1.22
Tc(MIN.) =   10.28
SUBAREA AREA(ACRES) =    0.21  SUBAREA RUNOFF(CFS) =    0.56
AREA-AVERAGE RUNOFF COEFFICIENT =   0.450
TOTAL AREA(ACRES) =    0.3  PEAK FLOW RATE(CFS) =          0.86

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.14  FLOW VELOCITY(FEET/SEC.) =    1.03
LONGEST FLOWPATH FROM NODE      345.00 TO NODE      347.00 =    168.60 FEET.

*****
FLOW PROCESS FROM NODE      347.00 TO NODE      348.00 IS CODE =  51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   3056.00  DOWNSTREAM(FEET) =   3055.88
CHANNEL LENGTH THRU SUBAREA(FEET) =    13.70  CHANNEL SLOPE =   0.0088
CHANNEL BASE(FEET) =    3.00  "Z" FACTOR =   20.000
MANNING'S FACTOR = 0.030  MAXIMUM DEPTH(FEET) =    0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =   5.894
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) =   88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =          2.44
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    1.31
AVERAGE FLOW DEPTH(FEET) =    0.24  TRAVEL TIME(MIN.) =    0.17
Tc(MIN.) =   10.46
SUBAREA AREA(ACRES) =    1.19  SUBAREA RUNOFF(CFS) =    3.16
AREA-AVERAGE RUNOFF COEFFICIENT =   0.450
TOTAL AREA(ACRES) =    1.5  PEAK FLOW RATE(CFS) =          4.00

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.30  FLOW VELOCITY(FEET/SEC.) =    1.48
LONGEST FLOWPATH FROM NODE      345.00 TO NODE      348.00 =    182.30 FEET.

*****
FLOW PROCESS FROM NODE      348.00 TO NODE      349.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   3049.38  DOWNSTREAM(FEET) =   3048.00
FLOW LENGTH(FEET) =   277.70  MANNING'S N =   0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS   9.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) =    4.27
ESTIMATED PIPE DIAMETER(INCH) = 18.00  NUMBER OF PIPES =    1
PIPE-FLOW(CFS) =    4.00
PIPE TRAVEL TIME(MIN.) =    1.08  Tc(MIN.) =   11.54
LONGEST FLOWPATH FROM NODE      345.00 TO NODE      349.00 =    460.00 FEET.

*****
FLOW PROCESS FROM NODE      349.00 TO NODE      349.00 IS CODE =  81
-----

```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

```
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.531
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4500
SUBAREA AREA(ACRES) = 1.56 SUBAREA RUNOFF(CFS) = 3.88
TOTAL AREA(ACRES) = 3.1 TOTAL RUNOFF(CFS) = 7.64
TC(MIN.) = 11.54
```

```
*****
FLOW PROCESS FROM NODE 349.00 TO NODE 350.00 IS CODE = 31
-----
```

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3048.00 DOWNSTREAM(FEET) = 3046.12
FLOW LENGTH(FEET) = 195.40 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.38
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.64
PIPE TRAVEL TIME(MIN.) = 0.51 Tc(MIN.) = 12.05
LONGEST FLOWPATH FROM NODE 345.00 TO NODE 350.00 = 655.40 FEET.
```

```
*****
FLOW PROCESS FROM NODE 351.00 TO NODE 350.00 IS CODE = 81
-----
```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

```
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.378
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4500
SUBAREA AREA(ACRES) = 0.70 SUBAREA RUNOFF(CFS) = 1.69
TOTAL AREA(ACRES) = 3.8 TOTAL RUNOFF(CFS) = 9.12
TC(MIN.) = 12.05
```

```
*****
FLOW PROCESS FROM NODE 350.00 TO NODE 354.00 IS CODE = 31
-----
```

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3046.12 DOWNSTREAM(FEET) = 3044.66
FLOW LENGTH(FEET) = 236.10 MANNING'S N = 0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.65
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.12
PIPE TRAVEL TIME(MIN.) = 0.70 Tc(MIN.) = 12.75
LONGEST FLOWPATH FROM NODE 345.00 TO NODE 354.00 = 891.50 FEET.
```

```
*****
FLOW PROCESS FROM NODE 355.00 TO NODE 354.00 IS CODE = 81
-----
```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

```
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.187
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5400
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4646
SUBAREA AREA(ACRES) = 0.73 SUBAREA RUNOFF(CFS) = 2.04
TOTAL AREA(ACRES) = 4.5 TOTAL RUNOFF(CFS) = 10.84
TC(MIN.) = 12.75
```

```
*****
FLOW PROCESS FROM NODE 354.00 TO NODE 354.00 IS CODE = 10
-----
```

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<


```

*****
FLOW PROCESS FROM NODE      358.00 TO NODE      359.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3057.70
DOWNSTREAM ELEVATION(FEET) = 3056.70
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.063
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
          THE MAXIMUM OVERLAND FLOW LENGTH = 60.00
          (Reference: Table 3-1B of Hydrology Manual)
          THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.463
SUBAREA RUNOFF(CFS) = 0.29
TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.29
*****
FLOW PROCESS FROM NODE      359.00 TO NODE      360.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.70 DOWNSTREAM(FEET) = 3056.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 65.70 CHANNEL SLOPE = 0.0107
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.991
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5100
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.57
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.97
AVERAGE FLOW DEPTH(FEET) = 0.11 TRAVEL TIME(MIN.) = 1.13
Tc(MIN.) = 10.19
SUBAREA AREA(ACRES) = 0.18 SUBAREA RUNOFF(CFS) = 0.55
AREA-AVERAGE RUNOFF COEFFICIENT = 0.489
TOTAL AREA(ACRES) = 0.3 PEAK FLOW RATE(CFS) = 0.82

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.13 FLOW VELOCITY(FEET/SEC.) = 1.07
LONGEST FLOWPATH FROM NODE 358.00 TO NODE 360.00 = 165.70 FEET.
*****
FLOW PROCESS FROM NODE      360.00 TO NODE      361.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.00 DOWNSTREAM(FEET) = 3055.99
CHANNEL LENGTH THRU SUBAREA(FEET) = 5.00 CHANNEL SLOPE = 0.0020
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.948
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5100
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.05
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.72
AVERAGE FLOW DEPTH(FEET) = 0.31 TRAVEL TIME(MIN.) = 0.12
Tc(MIN.) = 10.31
SUBAREA AREA(ACRES) = 0.81 SUBAREA RUNOFF(CFS) = 2.46
AREA-AVERAGE RUNOFF COEFFICIENT = 0.504
TOTAL AREA(ACRES) = 1.1 PEAK FLOW RATE(CFS) = 3.27

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.38 FLOW VELOCITY(FEET/SEC.) = 0.82
LONGEST FLOWPATH FROM NODE 358.00 TO NODE 361.00 = 170.70 FEET.

```

```

PR300B.RES
*****
FLOW PROCESS FROM NODE      361.00 TO NODE      362.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3049.49  DOWNSTREAM(FEET) = 3048.33
FLOW LENGTH(FEET) = 174.10  MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.52
ESTIMATED PIPE DIAMETER(INCH) = 18.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.27
PIPE TRAVEL TIME(MIN.) = 0.64  Tc(MIN.) = 10.95
LONGEST FLOWPATH FROM NODE 358.00 TO NODE 362.00 = 344.80 FEET.

*****
FLOW PROCESS FROM NODE      362.00 TO NODE      362.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.720
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5000
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.5024
SUBAREA AREA(ACRES) = 0.98  SUBAREA RUNOFF(CFS) = 2.80
TOTAL AREA(ACRES) = 2.1  TOTAL RUNOFF(CFS) = 5.95
TC(MIN.) = 10.95

*****
FLOW PROCESS FROM NODE      362.00 TO NODE      354.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3048.33  DOWNSTREAM(FEET) = 3044.66
FLOW LENGTH(FEET) = 136.10  MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.85
ESTIMATED PIPE DIAMETER(INCH) = 18.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.95
PIPE TRAVEL TIME(MIN.) = 0.26  Tc(MIN.) = 11.21
LONGEST FLOWPATH FROM NODE 358.00 TO NODE 354.00 = 480.90 FEET.

*****
FLOW PROCESS FROM NODE      354.00 TO NODE      354.00 IS CODE =  11
-----
>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1           5.95      11.21      5.636      2.07
LONGEST FLOWPATH FROM NODE 358.00 TO NODE 354.00 = 480.90 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)  (ACRE)
1           10.84     12.75      5.187      4.50
LONGEST FLOWPATH FROM NODE 345.00 TO NODE 354.00 = 891.50 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/HOUR)
1           15.48     11.21      5.636
2           16.32     12.75      5.187

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 16.32  Tc(MIN.) = 12.75
TOTAL AREA(ACRES) = 6.6

```

```

PR300B.RES
*****
FLOW PROCESS FROM NODE    354.00 TO NODE    354.00 IS CODE = 12
-----
>>>>>CLEAR MEMORY BANK # 2 <<<<<
=====
*****
FLOW PROCESS FROM NODE    354.00 TO NODE    363.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3044.66 DOWNSTREAM(FEET) = 3043.08
FLOW LENGTH(FEET) = 260.00 MANNING'S N = 0.012
DEPTH OF FLOW IN 24.0 INCH PIPE IS 18.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.37
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 16.32
PIPE TRAVEL TIME(MIN.) = 0.68 Tc(MIN.) = 13.43
LONGEST FLOWPATH FROM NODE    345.00 TO NODE    363.00 = 1151.50 FEET.
*****
FLOW PROCESS FROM NODE    364.00 TO NODE    363.00 IS CODE = 81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.016
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5800
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4868
SUBAREA AREA(ACRES) = 0.73 SUBAREA RUNOFF(CFS) = 2.12
TOTAL AREA(ACRES) = 7.3 TOTAL RUNOFF(CFS) = 17.83
TC(MIN.) = 13.43
*****
FLOW PROCESS FROM NODE    363.00 TO NODE    363.00 IS CODE = 10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<
=====
*****
FLOW PROCESS FROM NODE    363.00 TO NODE    363.00 IS CODE = 13
-----
>>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<
=====
*****
FLOW PROCESS FROM NODE    365.00 TO NODE    367.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .4500
S.C.S. CURVE NUMBER (AMC II) = 88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3057.70
DOWNSTREAM ELEVATION(FEET) = 3056.70
ELEVATION DIFFERENCE(FEET) = 1.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 9.063
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN
THE MAXIMUM OVERLAND FLOW LENGTH = 60.00
(Reference: Table 3-1B of Hydrology Manual)
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.463
SUBAREA RUNOFF(CFS) = 0.29
TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.29
*****
FLOW PROCESS FROM NODE    367.00 TO NODE    368.00 IS CODE = 51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====

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```

                                PR300B.RES
ELEVATION DATA: UPSTREAM(FEET) = 3056.70 DOWNSTREAM(FEET) = 3056.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 65.70 CHANNEL SLOPE = 0.0107
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.991
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5100
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.57
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.97
AVERAGE FLOW DEPTH(FEET) = 0.11 TRAVEL TIME(MIN.) = 1.13
Tc(MIN.) = 10.19
SUBAREA AREA(ACRES) = 0.18 SUBAREA RUNOFF(CFS) = 0.55
AREA-AVERAGE RUNOFF COEFFICIENT = 0.489
TOTAL AREA(ACRES) = 0.3 PEAK FLOW RATE(CFS) = 0.82

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.13 FLOW VELOCITY(FEET/SEC.) = 1.07
LONGEST FLOWPATH FROM NODE 365.00 TO NODE 368.00 = 165.70 FEET.

*****
FLOW PROCESS FROM NODE 368.00 TO NODE 369.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3056.00 DOWNSTREAM(FEET) = 3055.99
CHANNEL LENGTH THRU SUBAREA(FEET) = 5.10 CHANNEL SLOPE = 0.0020
CHANNEL BASE(FEET) = 3.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 0.50
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.946
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5000
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.02
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.71
AVERAGE FLOW DEPTH(FEET) = 0.31 TRAVEL TIME(MIN.) = 0.12
Tc(MIN.) = 10.31
SUBAREA AREA(ACRES) = 0.81 SUBAREA RUNOFF(CFS) = 2.41
AREA-AVERAGE RUNOFF COEFFICIENT = 0.497
TOTAL AREA(ACRES) = 1.1 PEAK FLOW RATE(CFS) = 3.22

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.38 FLOW VELOCITY(FEET/SEC.) = 0.80
LONGEST FLOWPATH FROM NODE 365.00 TO NODE 369.00 = 170.80 FEET.

*****
FLOW PROCESS FROM NODE 369.00 TO NODE 370.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3049.49 DOWNSTREAM(FEET) = 3048.34
FLOW LENGTH(FEET) = 174.10 MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.49
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.22
PIPE TRAVEL TIME(MIN.) = 0.65 Tc(MIN.) = 10.96
LONGEST FLOWPATH FROM NODE 365.00 TO NODE 370.00 = 344.90 FEET.

*****
FLOW PROCESS FROM NODE 370.00 TO NODE 370.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.718
*USER SPECIFIED(SUBAREA):
GENERAL INDUSTRIAL RUNOFF COEFFICIENT = .5000
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4985
SUBAREA AREA(ACRES) = 0.98 SUBAREA RUNOFF(CFS) = 2.80
TOTAL AREA(ACRES) = 2.1 TOTAL RUNOFF(CFS) = 5.90

```

TC(MIN.) = 10.96

```

*****
FLOW PROCESS FROM NODE      370.00 TO NODE      363.00 IS CODE =  31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3048.34  DOWNSTREAM(FEET) = 3043.08
FLOW LENGTH(FEET) = 136.00  MANNING'S N = 0.012
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.08
ESTIMATED PIPE DIAMETER(INCH) = 18.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.90
PIPE TRAVEL TIME(MIN.) = 0.22  Tc(MIN.) = 11.18
LONGEST FLOWPATH FROM NODE      365.00 TO NODE      363.00 = 480.90 FEET.

*****
FLOW PROCESS FROM NODE      363.00 TO NODE      363.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/ HOUR)  (ACRE)
1           5.90      11.18      5.643      2.07
LONGEST FLOWPATH FROM NODE      365.00 TO NODE      363.00 = 480.90 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **
STREAM      RUNOFF      Tc      INTENSITY      AREA
NUMBER      (CFS)      (MIN.)  (INCH/ HOUR)  (ACRE)
1           17.83     13.43      5.016      7.30
LONGEST FLOWPATH FROM NODE      345.00 TO NODE      363.00 = 1151.50 FEET.

** PEAK FLOW RATE TABLE **
STREAM      RUNOFF      Tc      INTENSITY
NUMBER      (CFS)      (MIN.)  (INCH/ HOUR)
1           20.75     11.18      5.643
2           23.07     13.43      5.016

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 23.07  Tc(MIN.) = 13.43
TOTAL AREA(ACRES) = 9.4

*****
FLOW PROCESS FROM NODE      363.00 TO NODE      363.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 2 <<<<
=====

*****
FLOW PROCESS FROM NODE      363.00 TO NODE      337.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3043.08  DOWNSTREAM(FEET) = 3041.20
FLOW LENGTH(FEET) = 239.50  MANNING'S N = 0.012
DEPTH OF FLOW IN 27.0 INCH PIPE IS 19.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.73
ESTIMATED PIPE DIAMETER(INCH) = 27.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 23.07
PIPE TRAVEL TIME(MIN.) = 0.52  Tc(MIN.) = 13.94
LONGEST FLOWPATH FROM NODE      345.00 TO NODE      337.00 = 1391.00 FEET.

*****
FLOW PROCESS FROM NODE      337.00 TO NODE      337.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **

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PR300B.RES

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/ HOUR)	AREA (ACRE)
1	23.07	13.94	4.895	9.37

LONGEST FLOWPATH FROM NODE 345.00 TO NODE 337.00 = 1391.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/ HOUR)	AREA (ACRE)
1	39.05	14.33	4.810	16.52

LONGEST FLOWPATH FROM NODE 305.20 TO NODE 337.00 = 2473.40 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/ HOUR)
1	61.07	13.94	4.895
2	61.72	14.33	4.810

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 61.72 Tc(MIN.) = 14.33
 TOTAL AREA(ACRES) = 25.9

 FLOW PROCESS FROM NODE 337.00 TO NODE 337.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<

 FLOW PROCESS FROM NODE 337.00 TO NODE 371.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 3037.72 DOWNSTREAM(FEET) = 3037.64
 FLOW LENGTH(FEET) = 22.80 MANNING'S N = 0.012
 DEPTH OF FLOW IN 45.0 INCH PIPE IS 32.2 INCHES
 PIPE-FLOW VELOCITY(FEET/ SEC.) = 7.29
 ESTIMATED PIPE DIAMETER(INCH) = 45.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 61.72
 PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 14.38
 LONGEST FLOWPATH FROM NODE 305.20 TO NODE 371.00 = 2496.20 FEET.

 FLOW PROCESS FROM NODE 371.00 TO NODE 372.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 3037.64 DOWNSTREAM(FEET) = 3037.25
 FLOW LENGTH(FEET) = 25.80 MANNING'S N = 0.012
 DEPTH OF FLOW IN 33.0 INCH PIPE IS 25.7 INCHES
 PIPE-FLOW VELOCITY(FEET/ SEC.) = 12.44
 ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 61.72
 PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 14.41
 LONGEST FLOWPATH FROM NODE 305.20 TO NODE 372.00 = 2522.00 FEET.

 FLOW PROCESS FROM NODE 372.00 TO NODE 373.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 3037.25 DOWNSTREAM(FEET) = 3007.99
 FLOW LENGTH(FEET) = 77.80 MANNING'S N = 0.012
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 14.1 INCHES
 PIPE-FLOW VELOCITY(FEET/ SEC.) = 41.46
 ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 61.72
 PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 14.45
 LONGEST FLOWPATH FROM NODE 305.20 TO NODE 373.00 = 2599.80 FEET.

 FLOW PROCESS FROM NODE 373.00 TO NODE 373.00 IS CODE = 81

```
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.785
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4881
SUBAREA AREA(ACRES) = 0.78 SUBAREA RUNOFF(CFS) = 1.49
TOTAL AREA(ACRES) = 26.7 TOTAL RUNOFF(CFS) = 62.28
TC(MIN.) = 14.45
*****
FLOW PROCESS FROM NODE 373.00 TO NODE 374.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3007.99 DOWNSTREAM(FEET) = 2984.19
FLOW LENGTH(FEET) = 67.30 MANNING'S N = 0.012
DEPTH OF FLOW IN 18.0 INCH PIPE IS 14.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 40.25
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 62.28
PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 14.47
LONGEST FLOWPATH FROM NODE 305.20 TO NODE 374.00 = 2667.10 FEET.
*****
FLOW PROCESS FROM NODE 374.00 TO NODE 374.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.779
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4843
SUBAREA AREA(ACRES) = 1.19 SUBAREA RUNOFF(CFS) = 2.27
TOTAL AREA(ACRES) = 27.9 TOTAL RUNOFF(CFS) = 64.48
TC(MIN.) = 14.47
*****
FLOW PROCESS FROM NODE 374.00 TO NODE 376.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 2984.19 DOWNSTREAM(FEET) = 2962.02
FLOW LENGTH(FEET) = 66.40 MANNING'S N = 0.012
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 41.20
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 64.48
PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 14.50
LONGEST FLOWPATH FROM NODE 305.20 TO NODE 376.00 = 2733.50 FEET.
*****
FLOW PROCESS FROM NODE 376.00 TO NODE 378.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 2962.02 DOWNSTREAM(FEET) = 2960.90
FLOW LENGTH(FEET) = 27.90 MANNING'S N = 0.012
DEPTH OF FLOW IN 30.0 INCH PIPE IS 20.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 18.52
ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 64.48
PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 14.52
LONGEST FLOWPATH FROM NODE 305.20 TO NODE 378.00 = 2761.40 FEET.
*****
FLOW PROCESS FROM NODE 378.00 TO NODE 378.00 IS CODE = 81
-----
```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

```
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.768
*USER SPECIFIED(SUBAREA):
URBAN NEWLY GRADED AREAS RUNOFF COEFFICIENT = .4000
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.4783
SUBAREA AREA(ACRES) = 2.14 SUBAREA RUNOFF(CFS) = 4.08
TOTAL AREA(ACRES) = 30.0 TOTAL RUNOFF(CFS) = 68.41
TC(MIN.) = 14.52
=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 30.0 TC(MIN.) = 14.52
PEAK FLOW RATE(CFS) = 68.41
=====
END OF RATIONAL METHOD ANALYSIS
=====
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□

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
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 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

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 15070 Avenue of Science
 Suite 100
 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 11-9-2009 *
 * PROPOSED HYDROLOGY *

FILE NAME: PR400.DAT
 TIME/DATE OF STUDY: 08:48 11/09/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
 1. Relative Flow-Depth = 0.00 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 400.00 TO NODE 401.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 2979.00
 DOWNSTREAM ELEVATION(FEET) = 2962.00
 ELEVATION DIFFERENCE(FEET) = 17.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.26
 TOTAL AREA(ACRES) = 0.09 TOTAL RUNOFF(CFS) = 0.26

 FLOW PROCESS FROM NODE 401.00 TO NODE 402.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 2962.00 DOWNSTREAM(FEET) = 2924.00

```

                                PR400.RES
CHANNEL LENGTH THRU SUBAREA(FEET) = 147.00 CHANNEL SLOPE = 0.2585
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1828 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.26
FLOW VELOCITY(FEET/SEC) = 2.39 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.02 Tc(MIN.) = 7.29
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 402.00 = 247.00 FEET.

*****
FLOW PROCESS FROM NODE 401.00 TO NODE 402.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.437
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.27 SUBAREA RUNOFF(CFS) = 0.70
TOTAL AREA(ACRES) = 0.4 TOTAL RUNOFF(CFS) = 0.94
TC(MIN.) = 7.29

*****
FLOW PROCESS FROM NODE 402.00 TO NODE 403.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 2924.00 DOWNSTREAM(FEET) = 2784.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 243.00 CHANNEL SLOPE = 0.5761
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2288 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.94
FLOW VELOCITY(FEET/SEC) = 2.68 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.51 Tc(MIN.) = 8.80
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 403.00 = 490.00 FEET.

*****
FLOW PROCESS FROM NODE 402.00 TO NODE 403.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.586
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.94 SUBAREA RUNOFF(CFS) = 2.17
TOTAL AREA(ACRES) = 1.3 TOTAL RUNOFF(CFS) = 3.00
TC(MIN.) = 8.80

*****
FLOW PROCESS FROM NODE 403.00 TO NODE 404.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 2784.00 DOWNSTREAM(FEET) = 2671.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 176.00 CHANNEL SLOPE = 0.6420
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2300 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 3.00
FLOW VELOCITY(FEET/SEC) = 3.87 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 0.76 Tc(MIN.) = 9.56
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 404.00 = 666.00 FEET.

*****
FLOW PROCESS FROM NODE 403.00 TO NODE 404.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.244

```

PR400.RES

*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 1.83 SUBAREA RUNOFF(CFS) = 4.00
TOTAL AREA(ACRES) = 3.1 TOTAL RUNOFF(CFS) = 6.84
TC(MIN.) = 9.56

FLOW PROCESS FROM NODE 404.00 TO NODE 405.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	2671.00	DOWNSTREAM(FEET) =	2510.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	520.00	CHANNEL SLOPE =	0.3096
SLOPE ADJUSTMENT CURVE USED:			
EFFECTIVE SLOPE = .1974 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)			
CHANNEL FLOW THRU SUBAREA(CFS) =	6.84		
FLOW VELOCITY(FEET/SEC) =	4.72 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)		
TRAVEL TIME(MIN.) =	1.84	Tc(MIN.) =	11.40
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 405.00 =	1186.00 FEET.		

FLOW PROCESS FROM NODE 404.00 TO NODE 405.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	5.575
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500
SOIL CLASSIFICATION IS "D"	
S.C.S. CURVE NUMBER (AMC II) =	88
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3500
SUBAREA AREA(ACRES) =	2.67 SUBAREA RUNOFF(CFS) = 5.21
TOTAL AREA(ACRES) =	5.8 TOTAL RUNOFF(CFS) = 11.32
TC(MIN.) =	11.40

=====

END OF STUDY SUMMARY:			
TOTAL AREA(ACRES)	=	5.8	TC(MIN.) = 11.40
PEAK FLOW RATE(CFS)	=	11.32	

=====

END OF RATIONAL METHOD ANALYSIS

□

 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
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Analysis prepared by:

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***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 05-21-2009 *
 * BASIN 500 *

FILE NAME: PR500.DAT
 TIME/DATE OF STUDY: 08:50 05/21/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
 1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 500.00 TO NODE 505.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 3256.00
DOWNSTREAM ELEVATION(FEET) = 3248.20
ELEVATION DIFFERENCE(FEET) = 7.80
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.808
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.773
SUBAREA RUNOFF(CFS) = 0.52
TOTAL AREA(ACRES) = 0.19 TOTAL RUNOFF(CFS) = 0.52

 FLOW PROCESS FROM NODE 505.00 TO NODE 510.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3248.20	DOWNSTREAM(FEET) = 3244.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 87.00	CHANNEL SLOPE = 0.0483

PR500.RES

SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .0483 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.52
FLOW VELOCITY(FEET/SEC) = 1.23 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.18 Tc(MIN.) = 7.99
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 510.00 = 187.00 FEET.

FLOW PROCESS FROM NODE 505.00 TO NODE 510.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.012
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.70 SUBAREA RUNOFF(CFS) = 1.72
TOTAL AREA(ACRES) = 0.9 TOTAL RUNOFF(CFS) = 2.18
TC(MIN.) = 7.99

FLOW PROCESS FROM NODE 510.00 TO NODE 515.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3244.00 DOWNSTREAM(FEET) = 3236.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 154.00 CHANNEL SLOPE = 0.0519
CHANNEL BASE(FEET) = 40.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.050
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.13
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.25
AVERAGE FLOW DEPTH(FEET) = 0.08 TRAVEL TIME(MIN.) = 2.05
Tc(MIN.) = 10.04
SUBAREA AREA(ACRES) = 1.84 SUBAREA RUNOFF(CFS) = 3.90
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 2.7 PEAK FLOW RATE(CFS) = 5.78

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.10 FLOW VELOCITY(FEET/SEC.) = 1.47
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 515.00 = 341.00 FEET.

FLOW PROCESS FROM NODE 515.00 TO NODE 520.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3236.00 DOWNSTREAM(FEET) = 3218.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 398.00 CHANNEL SLOPE = 0.0452
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.985
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 10.70
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.89
AVERAGE FLOW DEPTH(FEET) = 0.18 TRAVEL TIME(MIN.) = 3.51
Tc(MIN.) = 13.55
SUBAREA AREA(ACRES) = 5.60 SUBAREA RUNOFF(CFS) = 9.77
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 8.3 PEAK FLOW RATE(CFS) = 14.54

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.21 FLOW VELOCITY(FEET/SEC.) = 2.15
LONGEST FLOWPATH FROM NODE 500.00 TO NODE 520.00 = 739.00 FEET.

```

PR500.RES
*****
FLOW PROCESS FROM NODE    520.00 TO NODE    525.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   3218.00 DOWNSTREAM(FEET) =   3208.00
CHANNEL LENGTH THRU SUBAREA(FEET) =   233.00 CHANNEL SLOPE =   0.0429
CHANNEL BASE(FEET) =    30.00 "Z" FACTOR =   10.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) =    3.00
  100 YEAR RAINFALL INTENSITY(INCH/HOUR) =   4.640
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =   88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =    21.45
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    2.43
AVERAGE FLOW DEPTH(FEET) =    0.27 TRAVEL TIME(MIN.) =    1.60
Tc(MIN.) =    15.15
SUBAREA AREA(ACRES) =    8.51 SUBAREA RUNOFF(CFS) =   13.82
AREA-AVERAGE RUNOFF COEFFICIENT =   0.350
TOTAL AREA(ACRES) =   16.8 PEAK FLOW RATE(CFS) =   27.35

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =   0.31 FLOW VELOCITY(FEET/SEC.) =    2.67
LONGEST FLOWPATH FROM NODE    500.00 TO NODE    525.00 =   972.00 FEET.

*****
FLOW PROCESS FROM NODE    525.00 TO NODE    530.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   3208.00 DOWNSTREAM(FEET) =   3136.00
CHANNEL LENGTH THRU SUBAREA(FEET) =   1267.00 CHANNEL SLOPE =   0.0568
CHANNEL BASE(FEET) =    30.00 "Z" FACTOR =    7.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) =    3.00
  100 YEAR RAINFALL INTENSITY(INCH/HOUR) =   3.746
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =   88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =   43.83
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    3.55
AVERAGE FLOW DEPTH(FEET) =    0.38 TRAVEL TIME(MIN.) =    5.96
Tc(MIN.) =   21.11
SUBAREA AREA(ACRES) =   25.02 SUBAREA RUNOFF(CFS) =   32.81
AREA-AVERAGE RUNOFF COEFFICIENT =   0.350
TOTAL AREA(ACRES) =   41.9 PEAK FLOW RATE(CFS) =   54.89

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =   0.43 FLOW VELOCITY(FEET/SEC.) =    3.85
LONGEST FLOWPATH FROM NODE    500.00 TO NODE    530.00 =  2239.00 FEET.

*****
FLOW PROCESS FROM NODE    530.00 TO NODE    535.00 IS CODE =  51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =   3136.00 DOWNSTREAM(FEET) =   3062.00
CHANNEL LENGTH THRU SUBAREA(FEET) =   1438.00 CHANNEL SLOPE =   0.0515
CHANNEL BASE(FEET) =    30.00 "Z" FACTOR =    5.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) =    3.00
  100 YEAR RAINFALL INTENSITY(INCH/HOUR) =   3.259
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =   88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =   100.66
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    4.71
AVERAGE FLOW DEPTH(FEET) =    0.64 TRAVEL TIME(MIN.) =    5.09
Tc(MIN.) =   26.20
SUBAREA AREA(ACRES) =   80.02 SUBAREA RUNOFF(CFS) =   91.27
AREA-AVERAGE RUNOFF COEFFICIENT =   0.350
TOTAL AREA(ACRES) =  121.9 PEAK FLOW RATE(CFS) =  139.02

```

PR500.RES

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(Feet) = 0.78 FLOW VELOCITY(Feet/Sec.) = 5.29

LONGEST FLOWPATH FROM NODE 500.00 TO NODE 535.00 = 3677.00 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 121.9 TC(MIN.) = 26.20

PEAK FLOW RATE(CFS) = 139.02

=====

END OF RATIONAL METHOD ANALYSIS

□

 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
 (c) Copyright 1982-2008 Advanced Engineering Software (aes)
 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

Nolte Associates, Inc.
 15070 Avenue of Science
 Suite 100
 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 08-10-2009 *
 * BASIN 600 *

FILE NAME: PR600.DAT
 TIME/DATE OF STUDY: 11:30 08/10/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 600.00 TO NODE 601.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3362.00
 DOWNSTREAM ELEVATION(FEET) = 3341.00
 ELEVATION DIFFERENCE(FEET) = 21.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.20
 TOTAL AREA(ACRES) = 0.07 TOTAL RUNOFF(CFS) = 0.20

 FLOW PROCESS FROM NODE 601.00 TO NODE 602.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3341.00 DOWNSTREAM(FEET) = 3324.00


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                                PR600.RES
CHANNEL LENGTH THRU SUBAREA(FEET) =    62.00    CHANNEL SLOPE =    0.2742
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1881 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) =          0.20
FLOW VELOCITY(FEET/SEC) =    2.43 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    0.43    Tc(MIN.) =    6.69
LONGEST FLOWPATH FROM NODE    600.00 TO NODE    602.00 =    162.00 FEET.

*****
FLOW PROCESS FROM NODE    601.00 TO NODE    602.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    7.859
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) =    0.21    SUBAREA RUNOFF(CFS) =    0.58
TOTAL AREA(ACRES) =    0.3    TOTAL RUNOFF(CFS) =    0.77
TC(MIN.) =    6.69

*****
FLOW PROCESS FROM NODE    602.00 TO NODE    604.00 IS CODE =    53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    3324.00    DOWNSTREAM(FEET) =    3288.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    253.00    CHANNEL SLOPE =    0.1423
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1299 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) =    0.77
FLOW VELOCITY(FEET/SEC) =    2.02 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    2.09    Tc(MIN.) =    8.78
LONGEST FLOWPATH FROM NODE    600.00 TO NODE    604.00 =    415.00 FEET.

*****
FLOW PROCESS FROM NODE    602.00 TO NODE    604.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    6.596
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) =    0.39    SUBAREA RUNOFF(CFS) =    0.90
TOTAL AREA(ACRES) =    0.7    TOTAL RUNOFF(CFS) =    1.55
TC(MIN.) =    8.78

*****
FLOW PROCESS FROM NODE    604.00 TO NODE    606.00 IS CODE =    53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    3288.00    DOWNSTREAM(FEET) =    3286.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    65.00    CHANNEL SLOPE =    0.0308
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .0308 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) =    1.55
FLOW VELOCITY(FEET/SEC) =    1.14 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    0.95    Tc(MIN.) =    9.74
LONGEST FLOWPATH FROM NODE    600.00 TO NODE    606.00 =    480.00 FEET.

*****
FLOW PROCESS FROM NODE    604.00 TO NODE    606.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    6.171

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                                PR600.RES
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 2.84 SUBAREA RUNOFF(CFS) = 6.13
TOTAL AREA(ACRES) = 3.5 TOTAL RUNOFF(CFS) = 7.58
TC(MIN.) = 9.74

*****
FLOW PROCESS FROM NODE 606.00 TO NODE 608.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3286.00 DOWNSTREAM(FEET) = 3260.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 348.00 CHANNEL SLOPE = 0.0747
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .0747 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 7.58
FLOW VELOCITY(FEET/SEC) = 3.01 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.93 Tc(MIN.) = 11.67
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 608.00 = 828.00 FEET.

*****
FLOW PROCESS FROM NODE 606.00 TO NODE 608.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.492
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 5.84 SUBAREA RUNOFF(CFS) = 11.23
TOTAL AREA(ACRES) = 9.4 TOTAL RUNOFF(CFS) = 17.97
TC(MIN.) = 11.67

*****
FLOW PROCESS FROM NODE 608.00 TO NODE 610.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3260.00 DOWNSTREAM(FEET) = 3228.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 419.00 CHANNEL SLOPE = 0.0764
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .0764 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 17.97
FLOW VELOCITY(FEET/SEC) = 4.05 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.72 Tc(MIN.) = 13.39
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 610.00 = 1247.00 FEET.

*****
FLOW PROCESS FROM NODE 608.00 TO NODE 610.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.025
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 19.32 SUBAREA RUNOFF(CFS) = 33.98
TOTAL AREA(ACRES) = 28.7 TOTAL RUNOFF(CFS) = 50.42
TC(MIN.) = 13.39

*****
FLOW PROCESS FROM NODE 610.00 TO NODE 612.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3228.00 DOWNSTREAM(FEET) = 3190.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 708.00 CHANNEL SLOPE = 0.0537

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```

                                PR600.RES
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 4.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.600
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 92.17
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 6.01
AVERAGE FLOW DEPTH(FEET) = 1.07 TRAVEL TIME(MIN.) = 1.96
Tc(MIN.) = 15.35
SUBAREA AREA(ACRES) = 51.81 SUBAREA RUNOFF(CFS) = 83.42
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 80.5 PEAK FLOW RATE(CFS) = 129.58

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 1.29 FLOW VELOCITY(FEET/SEC.) = 6.65
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 612.00 = 1955.00 FEET.

*****
FLOW PROCESS FROM NODE 612.00 TO NODE 614.00 IS CODE = 51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3190.00 DOWNSTREAM(FEET) = 3082.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 2185.00 CHANNEL SLOPE = 0.0494
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 3.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.859
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 191.49
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 7.57
AVERAGE FLOW DEPTH(FEET) = 1.68 TRAVEL TIME(MIN.) = 4.81
Tc(MIN.) = 20.16
SUBAREA AREA(ACRES) = 91.43 SUBAREA RUNOFF(CFS) = 123.48
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 171.9 PEAK FLOW RATE(CFS) = 232.17

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 1.86 FLOW VELOCITY(FEET/SEC.) = 7.99
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 614.00 = 4140.00 FEET.

*****
FLOW PROCESS FROM NODE 614.00 TO NODE 616.00 IS CODE = 51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3082.00 DOWNSTREAM(FEET) = 3042.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1069.00 CHANNEL SLOPE = 0.0374
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 4.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.568
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 295.57
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 6.84
AVERAGE FLOW DEPTH(FEET) = 1.63 TRAVEL TIME(MIN.) = 2.61
Tc(MIN.) = 22.77
SUBAREA AREA(ACRES) = 101.50 SUBAREA RUNOFF(CFS) = 126.74
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 273.4 PEAK FLOW RATE(CFS) = 341.41

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 1.77 FLOW VELOCITY(FEET/SEC.) = 7.14
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 616.00 = 5209.00 FEET.

*****
FLOW PROCESS FROM NODE 616.00 TO NODE 616.00 IS CODE = 10
-----
>>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

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=====
*****
FLOW PROCESS FROM NODE      616.00 TO NODE      616.00 IS CODE =  13
-----
>>>>>CLEAR THE MAIN-STREAM MEMORY<<<<<
=====
*****
FLOW PROCESS FROM NODE      618.00 TO NODE      620.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =  88
INITIAL SUBAREA FLOW-LENGTH(FEET) =    88.00
UPSTREAM ELEVATION(FEET) =    3433.90
DOWNSTREAM ELEVATION(FEET) =    3404.90
ELEVATION DIFFERENCE(FEET) =    29.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =    5.879
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  8.545
SUBAREA RUNOFF(CFS) =    0.36
TOTAL AREA(ACRES) =    0.12  TOTAL RUNOFF(CFS) =    0.36
=====
*****
FLOW PROCESS FROM NODE      620.00 TO NODE      622.00 IS CODE =  53
-----
>>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    3404.90  DOWNSTREAM(FEET) =    3360.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    136.00  CHANNEL SLOPE =    0.3301
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2018 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) =    0.36
FLOW VELOCITY(FEET/SEC) =    2.52 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    0.90  Tc(MIN.) =    6.78
LONGEST FLOWPATH FROM NODE      618.00 TO NODE      622.00 =    224.00 FEET.
=====
*****
FLOW PROCESS FROM NODE      620.00 TO NODE      622.00 IS CODE =  81
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =  7.794
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =  88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) =    0.43  SUBAREA RUNOFF(CFS) =    1.17
TOTAL AREA(ACRES) =    0.6  TOTAL RUNOFF(CFS) =    1.50
TC(MIN.) =    6.78
=====
*****
FLOW PROCESS FROM NODE      622.00 TO NODE      626.00 IS CODE =  53
-----
>>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    3265.00  DOWNSTREAM(FEET) =    3205.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    638.00  CHANNEL SLOPE =    0.0940
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .0940 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) =    1.50
FLOW VELOCITY(FEET/SEC) =    1.97 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    5.41  Tc(MIN.) =    12.19
LONGEST FLOWPATH FROM NODE      618.00 TO NODE      626.00 =    862.00 FEET.
=====
*****
FLOW PROCESS FROM NODE      622.00 TO NODE      626.00 IS CODE =  81
-----

```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

```
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.339
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 1.69 SUBAREA RUNOFF(CFS) = 3.16
TOTAL AREA(ACRES) = 2.2 TOTAL RUNOFF(CFS) = 4.19
TC(MIN.) = 12.19
```

```
*****
FLOW PROCESS FROM NODE 626.00 TO NODE 628.00 IS CODE = 51
-----
```

```
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
```

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3205.00 DOWNSTREAM(FEET) = 3150.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 509.00 CHANNEL SLOPE = 0.1081
CHANNEL BASE(FEET) = 50.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.429
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 10.62
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.07
AVERAGE FLOW DEPTH(FEET) = 0.10 TRAVEL TIME(MIN.) = 4.09
Tc(MIN.) = 16.28
SUBAREA AREA(ACRES) = 8.25 SUBAREA RUNOFF(CFS) = 12.79
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 10.5 PEAK FLOW RATE(CFS) = 16.26
```

```
END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.13 FLOW VELOCITY(FEET/SEC.) = 2.44
LONGEST FLOWPATH FROM NODE 618.00 TO NODE 628.00 = 1371.00 FEET.
```

```
*****
FLOW PROCESS FROM NODE 628.00 TO NODE 630.00 IS CODE = 51
-----
```

```
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
```

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3150.00 DOWNSTREAM(FEET) = 3095.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 741.00 CHANNEL SLOPE = 0.0742
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 3.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.027
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 28.37
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.76
AVERAGE FLOW DEPTH(FEET) = 0.52 TRAVEL TIME(MIN.) = 2.59
Tc(MIN.) = 18.87
SUBAREA AREA(ACRES) = 17.17 SUBAREA RUNOFF(CFS) = 24.20
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 27.7 PEAK FLOW RATE(CFS) = 38.98
```

```
END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.62 FLOW VELOCITY(FEET/SEC.) = 5.29
LONGEST FLOWPATH FROM NODE 618.00 TO NODE 630.00 = 2112.00 FEET.
```

```
*****
FLOW PROCESS FROM NODE 630.00 TO NODE 632.00 IS CODE = 51
-----
```

```
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
```

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3095.00 DOWNSTREAM(FEET) = 3060.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 569.00 CHANNEL SLOPE = 0.0615
CHANNEL BASE(FEET) = 15.00 "Z" FACTOR = 3.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.794
```

```

                                PR600.RES
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 59.53
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.19
AVERAGE FLOW DEPTH(FEET) = 0.67 TRAVEL TIME(MIN.) = 1.83
Tc(MIN.) = 20.70
SUBAREA AREA(ACRES) = 30.93 SUBAREA RUNOFF(CFS) = 41.07
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 58.6 PEAK FLOW RATE(CFS) = 77.79

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.78 FLOW VELOCITY(FEET/SEC.) = 5.73
LONGEST FLOWPATH FROM NODE 618.00 TO NODE 632.00 = 2681.00 FEET.

*****
FLOW PROCESS FROM NODE 633.00 TO NODE 633.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.794
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3800
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3550
SUBAREA AREA(ACRES) = 11.71 SUBAREA RUNOFF(CFS) = 16.88
TOTAL AREA(ACRES) = 70.3 TOTAL RUNOFF(CFS) = 94.67
TC(MIN.) = 20.70

*****
FLOW PROCESS FROM NODE 632.00 TO NODE 616.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3060.00 DOWNSTREAM(FEET) = 3042.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 342.00 CHANNEL SLOPE = 0.0526
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 9.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.669
*USER SPECIFIED(SUBAREA):
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .5900
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 96.99
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.20
AVERAGE FLOW DEPTH(FEET) = 0.99 TRAVEL TIME(MIN.) = 1.10
Tc(MIN.) = 21.80
SUBAREA AREA(ACRES) = 2.14 SUBAREA RUNOFF(CFS) = 4.63
AREA-AVERAGE RUNOFF COEFFICIENT = 0.362
TOTAL AREA(ACRES) = 72.4 PEAK FLOW RATE(CFS) = 96.21

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.98 FLOW VELOCITY(FEET/SEC.) = 5.21
LONGEST FLOWPATH FROM NODE 618.00 TO NODE 616.00 = 3023.00 FEET.

*****
FLOW PROCESS FROM NODE 616.00 TO NODE 616.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
=====

** MAIN STREAM CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 96.21 21.80 3.669 72.44
LONGEST FLOWPATH FROM NODE 618.00 TO NODE 616.00 = 3023.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)
1 341.41 22.77 3.568 273.41
LONGEST FLOWPATH FROM NODE 600.00 TO NODE 616.00 = 5209.00 FEET.

** PEAK FLOW RATE TABLE **

```

PR600.RES

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	423.06	21.80	3.669
2	434.95	22.77	3.568

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 434.95 Tc(MIN.) = 22.77
TOTAL AREA(ACRES) = 345.9

FLOW PROCESS FROM NODE 616.00 TO NODE 634.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 3042.00 DOWNSTREAM(FEET) = 3032.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 259.00 CHANNEL SLOPE = 0.0386

CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 4.000

MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 3.00

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.516

*USER SPECIFIED(SUBAREA):

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3900

S.C.S. CURVE NUMBER (AMC II) = 88

TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 442.60

TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 8.29

AVERAGE FLOW DEPTH(FEET) = 2.61 TRAVEL TIME(MIN.) = 0.52

Tc(MIN.) = 23.29

SUBAREA AREA(ACRES) = 11.15 SUBAREA RUNOFF(CFS) = 15.29

AREA-AVERAGE RUNOFF COEFFICIENT = 0.354

TOTAL AREA(ACRES) = 357.0 PEAK FLOW RATE(CFS) = 443.94

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 2.61 FLOW VELOCITY(FEET/SEC.) = 8.30

LONGEST FLOWPATH FROM NODE 600.00 TO NODE 634.00 = 5468.00 FEET.

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 357.0 TC(MIN.) = 23.29

PEAK FLOW RATE(CFS) = 443.94

END OF RATIONAL METHOD ANALYSIS

□

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
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 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

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 15070 Avenue of Science
 Suite 100
 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 05-21-2009 *
 * BASIN 700 *

FILE NAME: PR700.DAT
 TIME/DATE OF STUDY: 09:17 05/21/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 700.00 TO NODE 702.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3450.00
 DOWNSTREAM ELEVATION(FEET) = 3434.00
 ELEVATION DIFFERENCE(FEET) = 16.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.11
 TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.11

 FLOW PROCESS FROM NODE 702.00 TO NODE 704.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3434.00 DOWNSTREAM(FEET) = 3416.00


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                                PR700.RES
CHANNEL LENGTH THRU SUBAREA(FEET) =    92.00    CHANNEL SLOPE =    0.1957
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1578 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) =          0.11
FLOW VELOCITY(FEET/SEC) =    2.22 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    0.69    Tc(MIN.) =    6.96
LONGEST FLOWPATH FROM NODE    700.00 TO NODE    704.00 =    192.00 FEET.

*****
FLOW PROCESS FROM NODE    702.00 TO NODE    704.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    7.666
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) =    0.10    SUBAREA RUNOFF(CFS) =    0.27
TOTAL AREA(ACRES) =    0.1    TOTAL RUNOFF(CFS) =    0.38
TC(MIN.) =    6.96

*****
FLOW PROCESS FROM NODE    704.00 TO NODE    706.00 IS CODE =    53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    3416.00    DOWNSTREAM(FEET) =    3370.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    132.00    CHANNEL SLOPE =    0.3485
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2050 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) =    0.38
FLOW VELOCITY(FEET/SEC) =    2.54 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    0.87    Tc(MIN.) =    7.82
LONGEST FLOWPATH FROM NODE    700.00 TO NODE    706.00 =    324.00 FEET.

*****
FLOW PROCESS FROM NODE    704.00 TO NODE    706.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    7.106
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) =    0.20    SUBAREA RUNOFF(CFS) =    0.50
TOTAL AREA(ACRES) =    0.3    TOTAL RUNOFF(CFS) =    0.85
TC(MIN.) =    7.82

*****
FLOW PROCESS FROM NODE    706.00 TO NODE    708.00 IS CODE =    53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    3370.00    DOWNSTREAM(FEET) =    3272.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    294.00    CHANNEL SLOPE =    0.3333
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .2023 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) =    0.85
FLOW VELOCITY(FEET/SEC) =    2.52 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =    1.95    Tc(MIN.) =    9.77
LONGEST FLOWPATH FROM NODE    700.00 TO NODE    708.00 =    618.00 FEET.

*****
FLOW PROCESS FROM NODE    706.00 TO NODE    708.00 IS CODE =    81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====

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                                PR700.RES
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.158
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 0.64 SUBAREA RUNOFF(CFS) = 1.38
TOTAL AREA(ACRES) = 1.0 TOTAL RUNOFF(CFS) = 2.11
TC(MIN.) = 9.77

*****
FLOW PROCESS FROM NODE 708.00 TO NODE 710.00 IS CODE = 53
-----
>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3272.00 DOWNSTREAM(FEET) = 3214.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 192.00 CHANNEL SLOPE = 0.3021
SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .1955 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 2.11
FLOW VELOCITY(FEET/SEC) = 3.18 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 1.01 Tc(MIN.) = 10.78
LONGEST FLOWPATH FROM NODE 700.00 TO NODE 710.00 = 810.00 FEET.

*****
FLOW PROCESS FROM NODE 708.00 TO NODE 710.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.780
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3500
SUBAREA AREA(ACRES) = 2.66 SUBAREA RUNOFF(CFS) = 5.38
TOTAL AREA(ACRES) = 3.6 TOTAL RUNOFF(CFS) = 7.36
TC(MIN.) = 10.78

*****
FLOW PROCESS FROM NODE 710.00 TO NODE 712.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3214.00 DOWNSTREAM(FEET) = 3100.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 418.00 CHANNEL SLOPE = 0.2727
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 3.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 3.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.362
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 11.62
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.24
AVERAGE FLOW DEPTH(FEET) = 0.21 TRAVEL TIME(MIN.) = 1.33
Tc(MIN.) = 12.11
SUBAREA AREA(ACRES) = 4.53 SUBAREA RUNOFF(CFS) = 8.50
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 8.2 PEAK FLOW RATE(CFS) = 15.33

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.25 FLOW VELOCITY(FEET/SEC.) = 5.78
LONGEST FLOWPATH FROM NODE 700.00 TO NODE 712.00 = 1228.00 FEET.

*****
FLOW PROCESS FROM NODE 712.00 TO NODE 714.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3100.00 DOWNSTREAM(FEET) = 3068.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 172.15 CHANNEL SLOPE = 0.1859
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 4.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 3.00

```

```

                                PR700.RES
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.219
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 19.66
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.53
AVERAGE FLOW DEPTH(FEET) = 0.32 TRAVEL TIME(MIN.) = 0.52
Tc(MIN.) = 12.63
SUBAREA AREA(ACRES) = 4.74 SUBAREA RUNOFF(CFS) = 8.66
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 12.9 PEAK FLOW RATE(CFS) = 23.58

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.35 FLOW VELOCITY(FEET/SEC.) = 5.84
LONGEST FLOWPATH FROM NODE 700.00 TO NODE 714.00 = 1400.15 FEET.
=====
END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 12.9 TC(MIN.) = 12.63
PEAK FLOW RATE(CFS) = 23.58
=====
END OF RATIONAL METHOD ANALYSIS

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RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
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 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

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 15070 Avenue of Science
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 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 11-9-2009 *
 * BASIN 800: CFS/AC RATIO TO BE USED FOR CULVERTS W/BASINS UP TO 6 AC *

FILE NAME: PR800.DAT
 TIME/DATE OF STUDY: 16:44 11/11/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
 1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 800.00 TO NODE 805.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3243.00
 DOWNSTREAM ELEVATION(FEET) = 3231.80
 ELEVATION DIFFERENCE(FEET) = 11.20
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.11
 TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.11

 FLOW PROCESS FROM NODE 805.00 TO NODE 810.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3231.80 DOWNSTREAM(FEET) = 3210.00

```

PR800.RES
CHANNEL LENGTH THRU SUBAREA(FEET) = 88.80 CHANNEL SLOPE = 0.2455
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 7.146
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.20
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.99
AVERAGE FLOW DEPTH(FEET) = 0.02 TRAVEL TIME(MIN.) = 1.49
Tc(MIN.) = 7.76
SUBAREA AREA(ACRES) = 0.07 SUBAREA RUNOFF(CFS) = 0.18
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 0.1 PEAK FLOW RATE(CFS) = 0.28

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.03 FLOW VELOCITY(FEET/SEC.) = 1.04
LONGEST FLOWPATH FROM NODE 800.00 TO NODE 810.00 = 188.80 FEET.

*****
FLOW PROCESS FROM NODE 810.00 TO NODE 815.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3210.00 DOWNSTREAM(FEET) = 3178.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 145.10 CHANNEL SLOPE = 0.2205
CHANNEL BASE(FEET) = 8.00 "Z" FACTOR = 8.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.394
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.57
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.66
AVERAGE FLOW DEPTH(FEET) = 0.04 TRAVEL TIME(MIN.) = 1.46
Tc(MIN.) = 9.21
SUBAREA AREA(ACRES) = 0.26 SUBAREA RUNOFF(CFS) = 0.58
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 0.4 PEAK FLOW RATE(CFS) = 0.83

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.05 FLOW VELOCITY(FEET/SEC.) = 1.95
LONGEST FLOWPATH FROM NODE 800.00 TO NODE 815.00 = 333.90 FEET.

*****
FLOW PROCESS FROM NODE 815.00 TO NODE 820.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3178.00 DOWNSTREAM(FEET) = 3154.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 140.70 CHANNEL SLOPE = 0.1706
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.850
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.02
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.72
AVERAGE FLOW DEPTH(FEET) = 0.06 TRAVEL TIME(MIN.) = 1.36
Tc(MIN.) = 10.58
SUBAREA AREA(ACRES) = 0.19 SUBAREA RUNOFF(CFS) = 0.39
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 0.6 PEAK FLOW RATE(CFS) = 1.15

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.06 FLOW VELOCITY(FEET/SEC.) = 1.86
LONGEST FLOWPATH FROM NODE 800.00 TO NODE 820.00 = 474.60 FEET.

*****
FLOW PROCESS FROM NODE 820.00 TO NODE 825.00 IS CODE = 51
-----

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PR800.RES

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3154.00 DOWNSTREAM(FEET) = 3148.79
CHANNEL LENGTH THRU SUBAREA(FEET) = 37.30 CHANNEL SLOPE = 0.1397
CHANNEL BASE(FEET) = 15.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.734
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.09
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.86
AVERAGE FLOW DEPTH(FEET) = 0.07 TRAVEL TIME(MIN.) = 0.33
Tc(MIN.) = 10.91
SUBAREA AREA(ACRES) = 0.94 SUBAREA RUNOFF(CFS) = 1.89
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 1.5 PEAK FLOW RATE(CFS) = 3.01
```

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

```
DEPTH(FEET) = 0.09 FLOW VELOCITY(FEET/SEC.) = 2.14
LONGEST FLOWPATH FROM NODE 800.00 TO NODE 825.00 = 511.90 FEET.
=====
```

END OF STUDY SUMMARY:

```
TOTAL AREA(ACRES) = 1.5 TC(MIN.) = 10.91
PEAK FLOW RATE(CFS) = 3.01
=====
```

END OF RATIONAL METHOD ANALYSIS

□

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
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 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

Nolte Associates, Inc.
 15070 Avenue of Science
 Suite 100
 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 11-9-2009 *
 * BASIN 900: CFS/AC RATIO USED FOR CULVERTS W/BASINS BTWN 7-9AC *

FILE NAME: PR900.DAT
 TIME/DATE OF STUDY: 16:35 11/11/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP HIKE (FT) (FT) (FT)	MANNING FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313 0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 900.00 TO NODE 905.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
 SOIL CLASSIFICATION IS "D"
 S.C.S. CURVE NUMBER (AMC II) = 88
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
 UPSTREAM ELEVATION(FEET) = 3250.00
 DOWNSTREAM ELEVATION(FEET) = 3232.00
 ELEVATION DIFFERENCE(FEET) = 18.00
 SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 8.200
 SUBAREA RUNOFF(CFS) = 0.11
 TOTAL AREA(ACRES) = 0.04 TOTAL RUNOFF(CFS) = 0.11

 FLOW PROCESS FROM NODE 905.00 TO NODE 910.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 3232.00 DOWNSTREAM(FEET) = 3200.00

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                                PR900.RES
CHANNEL LENGTH THRU SUBAREA(FEET) = 110.60 CHANNEL SLOPE = 0.2893
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.883
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.24
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.94
AVERAGE FLOW DEPTH(FEET) = 0.01 TRAVEL TIME(MIN.) = 1.95
Tc(MIN.) = 8.22
SUBAREA AREA(ACRES) = 0.10 SUBAREA RUNOFF(CFS) = 0.24
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 0.1 PEAK FLOW RATE(CFS) = 0.34

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.02 FLOW VELOCITY(FEET/SEC.) = 0.91
LONGEST FLOWPATH FROM NODE 900.00 TO NODE 910.00 = 210.60 FEET.

*****
FLOW PROCESS FROM NODE 910.00 TO NODE 915.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3200.00 DOWNSTREAM(FEET) = 3166.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 115.60 CHANNEL SLOPE = 0.2941
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 20.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.117
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.62
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.17
AVERAGE FLOW DEPTH(FEET) = 0.03 TRAVEL TIME(MIN.) = 1.65
Tc(MIN.) = 9.87
SUBAREA AREA(ACRES) = 0.26 SUBAREA RUNOFF(CFS) = 0.56
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 0.4 PEAK FLOW RATE(CFS) = 0.86

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.03 FLOW VELOCITY(FEET/SEC.) = 1.51
LONGEST FLOWPATH FROM NODE 900.00 TO NODE 915.00 = 326.20 FEET.

*****
FLOW PROCESS FROM NODE 915.00 TO NODE 920.00 IS CODE = 51
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 3166.00 DOWNSTREAM(FEET) = 3150.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 56.80 CHANNEL SLOPE = 0.2817
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.968
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.51
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.46
AVERAGE FLOW DEPTH(FEET) = 0.06 TRAVEL TIME(MIN.) = 0.39
Tc(MIN.) = 10.26
SUBAREA AREA(ACRES) = 0.63 SUBAREA RUNOFF(CFS) = 1.32
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 1.0 PEAK FLOW RATE(CFS) = 2.15

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.07 FLOW VELOCITY(FEET/SEC.) = 2.73
LONGEST FLOWPATH FROM NODE 900.00 TO NODE 920.00 = 383.00 FEET.

*****
FLOW PROCESS FROM NODE 920.00 TO NODE 925.00 IS CODE = 51
-----

```


>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3150.00 DOWNSTREAM(FEET) = 3110.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 146.70 CHANNEL SLOPE = 0.2727
CHANNEL BASE(FEET) = 6.00 "Z" FACTOR = 6.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.759
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.59
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.20
AVERAGE FLOW DEPTH(FEET) = 0.16 TRAVEL TIME(MIN.) = 0.58
Tc(MIN.) = 10.84
SUBAREA AREA(ACRES) = 2.42 SUBAREA RUNOFF(CFS) = 4.88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 3.5 PEAK FLOW RATE(CFS) = 6.95
```

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.20 FLOW VELOCITY(FEET/SEC.) = 4.79
 LONGEST FLOWPATH FROM NODE 900.00 TO NODE 925.00 = 529.70 FEET.

 FLOW PROCESS FROM NODE 925.00 TO NODE 930.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3110.00 DOWNSTREAM(FEET) = 3094.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 126.00 CHANNEL SLOPE = 0.1270
CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 3.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.601
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 8.23
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.38
AVERAGE FLOW DEPTH(FEET) = 0.32 TRAVEL TIME(MIN.) = 0.48
Tc(MIN.) = 11.32
SUBAREA AREA(ACRES) = 1.30 SUBAREA RUNOFF(CFS) = 2.55
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 4.8 PEAK FLOW RATE(CFS) = 9.31
```

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.34 FLOW VELOCITY(FEET/SEC.) = 4.57
 LONGEST FLOWPATH FROM NODE 900.00 TO NODE 930.00 = 655.70 FEET.

 FLOW PROCESS FROM NODE 930.00 TO NODE 935.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

```
=====
ELEVATION DATA: UPSTREAM(FEET) = 3094.00 DOWNSTREAM(FEET) = 3067.81
CHANNEL LENGTH THRU SUBAREA(FEET) = 361.50 CHANNEL SLOPE = 0.0724
CHANNEL BASE(FEET) = 2.00 "Z" FACTOR = 1.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.450
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 12.13
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 12.31
AVERAGE FLOW DEPTH(FEET) = 0.41 TRAVEL TIME(MIN.) = 0.49
Tc(MIN.) = 11.81
SUBAREA AREA(ACRES) = 2.96 SUBAREA RUNOFF(CFS) = 5.65
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 7.7 PEAK FLOW RATE(CFS) = 14.71
```

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.46 FLOW VELOCITY(FEET/SEC.) = 12.97
 LONGEST FLOWPATH FROM NODE 900.00 TO NODE 935.00 = 1017.20 FEET.

PR900.RES

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 7.7 TC(MIN.) = 11.81

PEAK FLOW RATE(CFS) = 14.71

=====

END OF RATIONAL METHOD ANALYSIS

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RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
 2003,1985,1981 HYDROLOGY MANUAL
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 Ver. 15.0 Release Date: 04/01/2008 License ID 1504

Analysis prepared by:

Nolte Associates, Inc.
 15070 Avenue of Science
 Suite 100
 San Diego, CA 92128

***** DESCRIPTION OF STUDY *****
 * SDB039600 *
 * 11-9-2009 *
 * BASIN 1000: CFS/AC RATIO USED FOR CULVERTS W/BASINS BTWN 17-23AC *

FILE NAME: PR1000.DAT
 TIME/DATE OF STUDY: 11:49 11/12/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 6-HOUR DURATION PRECIPITATION (INCHES) = 3.600
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
 NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS
 USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT) (FT)	HIKE (FT)	FACTOR (n)
1	16.0	11.0	0.018/0.018/0.020	0.50	1.50 0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
 1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
 *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

 FLOW PROCESS FROM NODE 1000.00 TO NODE 1005.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT =	.3500
SOIL CLASSIFICATION IS "D"	
S.C.S. CURVE NUMBER (AMC II) =	88
INITIAL SUBAREA FLOW-LENGTH(FEET) =	95.90
UPSTREAM ELEVATION(FEET) =	3362.00
DOWNSTREAM ELEVATION(FEET) =	3328.00
ELEVATION DIFFERENCE(FEET) =	34.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) =	6.137
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc CALCULATION!	
100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	8.311
SUBAREA RUNOFF(CFS) =	0.15
TOTAL AREA(ACRES) =	0.05
TOTAL RUNOFF(CFS) =	0.15

 FLOW PROCESS FROM NODE 1005.00 TO NODE 1010.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	3328.00	DOWNSTREAM(FEET) =	3314.00
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                                PRI000.RES
CHANNEL LENGTH THRU SUBAREA(FEET) =    76.70  CHANNEL SLOPE =    0.1825
CHANNEL BASE(FEET) =    20.00  "Z" FACTOR =    20.000
MANNING'S FACTOR =    0.050  MAXIMUM DEPTH(FEET) =    2.00
  100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    7.166
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =          0.30
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    0.81
AVERAGE FLOW DEPTH(FEET) =    0.02  TRAVEL TIME(MIN.) =    1.59
Tc(MIN.) =    7.72
SUBAREA AREA(ACRES) =    0.12  SUBAREA RUNOFF(CFS) =    0.30
AREA-AVERAGE RUNOFF COEFFICIENT =    0.350
TOTAL AREA(ACRES) =    0.2  PEAK FLOW RATE(CFS) =          0.43

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =    0.02  FLOW VELOCITY(FEET/SEC.) =    1.04
LONGEST FLOWPATH FROM NODE    1000.00 TO NODE    1010.00 =    172.60 FEET.

*****
FLOW PROCESS FROM NODE    1010.00 TO NODE    1015.00 IS CODE =    51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    3314.00  DOWNSTREAM(FEET) =    3310.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    32.80  CHANNEL SLOPE =    0.1220
CHANNEL BASE(FEET) =    20.00  "Z" FACTOR =    6.000
MANNING'S FACTOR =    0.050  MAXIMUM DEPTH(FEET) =    2.00
  100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    6.970
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =          1.92
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    1.62
AVERAGE FLOW DEPTH(FEET) =    0.06  TRAVEL TIME(MIN.) =    0.34
Tc(MIN.) =    8.06
SUBAREA AREA(ACRES) =    1.22  SUBAREA RUNOFF(CFS) =    2.98
AREA-AVERAGE RUNOFF COEFFICIENT =    0.350
TOTAL AREA(ACRES) =    1.4  PEAK FLOW RATE(CFS) =          3.39

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =    0.09  FLOW VELOCITY(FEET/SEC.) =    1.90
LONGEST FLOWPATH FROM NODE    1000.00 TO NODE    1015.00 =    205.40 FEET.

*****
FLOW PROCESS FROM NODE    1015.00 TO NODE    1020.00 IS CODE =    51
-----
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =    3310.00  DOWNSTREAM(FEET) =    3290.00
CHANNEL LENGTH THRU SUBAREA(FEET) =    195.60  CHANNEL SLOPE =    0.1022
CHANNEL BASE(FEET) =    20.00  "Z" FACTOR =    6.000
MANNING'S FACTOR =    0.050  MAXIMUM DEPTH(FEET) =    2.00
  100 YEAR RAINFALL INTENSITY(INCH/HOUR) =    6.251
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) =    88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =          5.07
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) =    2.20
AVERAGE FLOW DEPTH(FEET) =    0.11  TRAVEL TIME(MIN.) =    1.48
Tc(MIN.) =    9.54
SUBAREA AREA(ACRES) =    1.54  SUBAREA RUNOFF(CFS) =    3.37
AREA-AVERAGE RUNOFF COEFFICIENT =    0.350
TOTAL AREA(ACRES) =    2.9  PEAK FLOW RATE(CFS) =          6.41

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) =    0.13  FLOW VELOCITY(FEET/SEC.) =    2.33
LONGEST FLOWPATH FROM NODE    1000.00 TO NODE    1020.00 =    401.00 FEET.

*****
FLOW PROCESS FROM NODE    1020.00 TO NODE    1025.00 IS CODE =    51
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

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=====
ELEVATION DATA: UPSTREAM(FEET) = 3290.00 DOWNSTREAM(FEET) = 3188.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 502.90 CHANNEL SLOPE = 0.2028
CHANNEL BASE(FEET) = 8.00 "Z" FACTOR = 8.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.540
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 8.61
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.26
AVERAGE FLOW DEPTH(FEET) = 0.21 TRAVEL TIME(MIN.) = 1.97
Tc(MIN.) = 11.51
SUBAREA AREA(ACRES) = 2.26 SUBAREA RUNOFF(CFS) = 4.38
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 5.2 PEAK FLOW RATE(CFS) = 10.06
```

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.23 FLOW VELOCITY(FEET/SEC.) = 4.41
LONGEST FLOWPATH FROM NODE 1000.00 TO NODE 1025.00 = 903.90 FEET.

FLOW PROCESS FROM NODE 1025.00 TO NODE 1030.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

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=====
ELEVATION DATA: UPSTREAM(FEET) = 3188.00 DOWNSTREAM(FEET) = 3174.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 53.70 CHANNEL SLOPE = 0.2607
CHANNEL BASE(FEET) = 8.00 "Z" FACTOR = 8.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.494
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 18.90
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 6.00
AVERAGE FLOW DEPTH(FEET) = 0.30 TRAVEL TIME(MIN.) = 0.15
Tc(MIN.) = 11.66
SUBAREA AREA(ACRES) = 9.19 SUBAREA RUNOFF(CFS) = 17.67
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 14.4 PEAK FLOW RATE(CFS) = 27.65
```

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.38 FLOW VELOCITY(FEET/SEC.) = 6.66
LONGEST FLOWPATH FROM NODE 1000.00 TO NODE 1030.00 = 957.60 FEET.

FLOW PROCESS FROM NODE 1030.00 TO NODE 1035.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

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=====
ELEVATION DATA: UPSTREAM(FEET) = 3174.00 DOWNSTREAM(FEET) = 3121.75
CHANNEL LENGTH THRU SUBAREA(FEET) = 508.70 CHANNEL SLOPE = 0.1027
CHANNEL BASE(FEET) = 6.00 "Z" FACTOR = 10.000
MANNING'S FACTOR = 0.050 MAXIMUM DEPTH(FEET) = 2.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 5.030
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 31.74
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.96
AVERAGE FLOW DEPTH(FEET) = 0.55 TRAVEL TIME(MIN.) = 1.71
Tc(MIN.) = 13.37
SUBAREA AREA(ACRES) = 4.65 SUBAREA RUNOFF(CFS) = 8.19
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350
TOTAL AREA(ACRES) = 19.0 PEAK FLOW RATE(CFS) = 33.50
```

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.57 FLOW VELOCITY(FEET/SEC.) = 5.03
LONGEST FLOWPATH FROM NODE 1000.00 TO NODE 1035.00 = 1466.30 FEET.

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END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 19.0 TC(MIN.) = 13.37

PEAK FLOW RATE(CFS) = 33.50

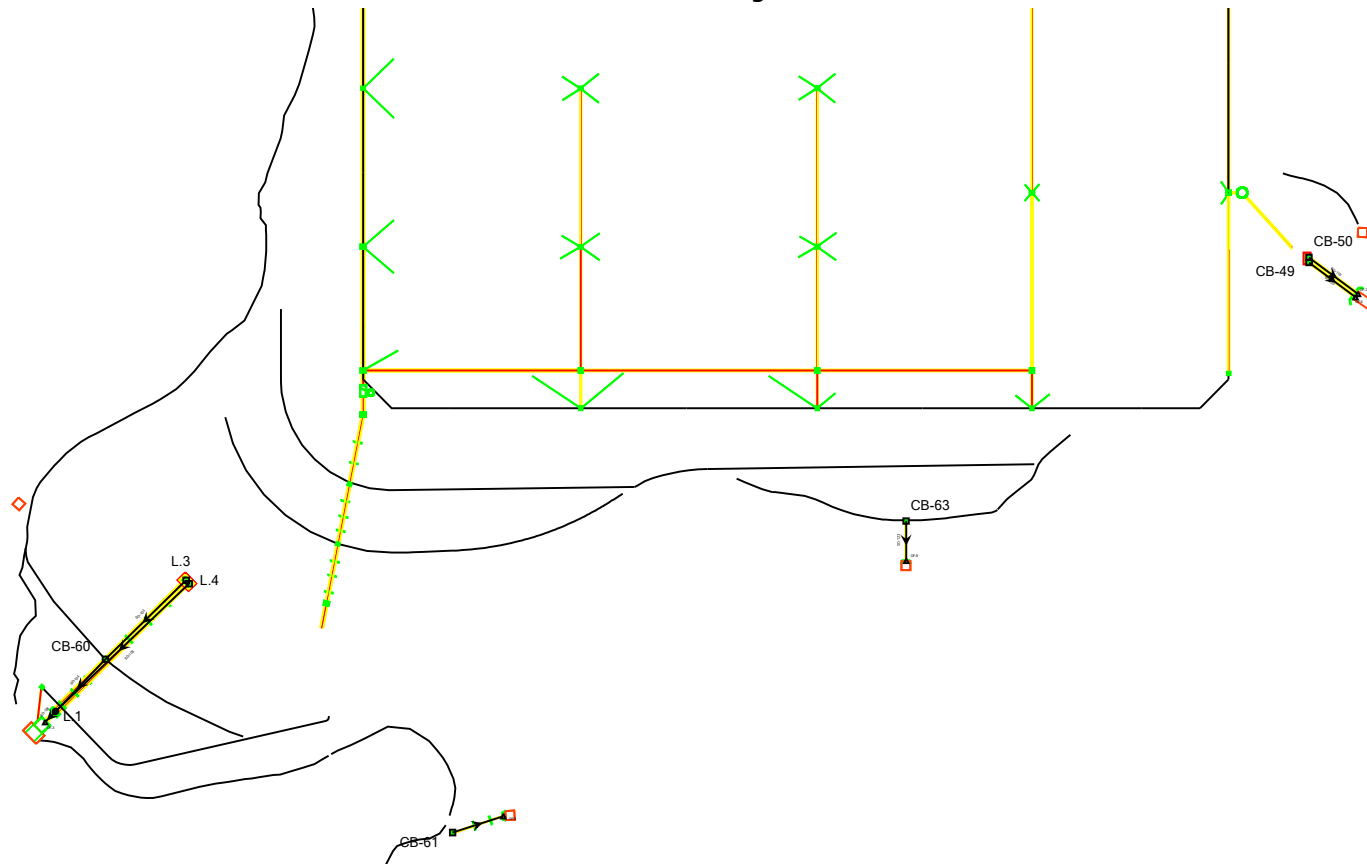
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END OF RATIONAL METHOD ANALYSIS

□

HYDRAULIC CALCULATIONS

Scenario: 100yr Basin 100



FlexTable: Conduit Table (Rev Basin 100.stc)

Label	Start Node	Invert (Upstream) (ft)	Stop Node	Invert (Downstream) (ft)	Manning's n	Section Size	Diameter (in)	Flow (ft³/s)	Length (Unified) (ft)	Slope (Calculate d) (ft/ft)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Cover (Start) (ft)	Cover (Stop) (ft)	Velocity (Average) (ft/s)
SD-64	CDS-199.2	3,034.55	MH-199.3	3,011.11	0.012	42 inch	42.0	58.05	104.8	0.224	3,056.30	3,017.80	3,036.94	3,016.48	18.25	3.19	35.46
SD-68	MH-199.3	3,011.11	OF-199.4	3,011.00	0.013	42 inch	42.0	58.05	22.8	0.005	3,017.80	3,011.00	3,016.20	3,016.12	3.19	-3.50	6.03
SD-57	CB-190	3,047.22	CB-191	3,046.43	0.012	18 inch	18.0	9.34	159.3	0.005	3,055.75	3,055.56	3,048.78	3,047.61	7.03	7.63	5.29
SD-56	CB-190.5	3,048.87	CB-190	3,048.08	0.012	18 inch	18.0	1.91	159.6	0.005	3,055.62	3,055.75	3,049.43	3,049.43	5.25	6.17	3.72
SD-55	CB-188	3,049.72	CB-189	3,048.90	0.012	18 inch	18.0	2.61	158.5	0.005	3,056.22	3,056.22	3,050.33	3,050.03	5.00	5.82	4.12
SD-41	CB-189	3,048.60	CB-190	3,047.50	0.012	18 inch	18.0	4.90	219.0	0.005	3,056.22	3,055.75	3,049.78	3,049.43	6.12	6.75	4.78
SD-71	CB-183.5	3,051.81	MH-183	3,051.60	0.012	18 inch	18.0	2.01	38.0	0.006	3,056.31	3,056.91	3,052.34	3,052.10	3.00	3.81	3.93
SD-94	CB-197	3,051.64	MH-198	3,051.40	0.012	18 inch	18.0	2.60	38.0	0.006	3,056.14	3,056.82	3,052.25	3,051.95	3.00	3.92	4.42
SD-43	CB-178	3,048.57	MH-179	3,047.82	0.012	36 inch	36.0	24.37	150.2	0.005	3,056.45	3,056.00	3,050.26	3,050.31	4.88	5.18	7.14
SD-47	CB-180.5	3,049.72	CB-180	3,048.70	0.012	18 inch	18.0	3.58	193.5	0.005	3,056.22	3,056.28	3,050.44	3,049.39	5.00	6.08	4.51
SD-46	MH-179	3,047.52	CB-180	3,046.28	0.012	36 inch	36.0	28.51	246.8	0.005	3,056.00	3,056.28	3,049.25	3,049.01	5.48	7.00	7.44
SD-58	CB-191	3,045.93	CB-192	3,044.87	0.012	24 inch	24.0	12.99	212.2	0.005	3,055.56	3,055.56	3,047.23	3,046.45	7.63	8.69	6.05
SD-59	CB-192	3,044.57	CB-193	3,043.53	0.012	24 inch	24.0	16.62	207.3	0.005	3,055.56	3,055.58	3,046.14	3,045.00	8.99	10.05	6.29
SD-60	CB-193	3,042.53	CB-184	3,041.76	0.012	36 inch	36.0	19.82	154.2	0.005	3,055.58	3,056.62	3,044.38	3,044.38	10.05	11.86	6.77
SD-61	MH-183	3,042.32	CB-184	3,041.22	0.012	42 inch	42.0	39.80	219.0	0.005	3,056.91	3,056.62	3,044.28	3,044.38	11.09	11.90	8.09
SD-67	CB-184	3,040.92	CDS-199.2	3,040.87	0.012	42 inch	42.0	58.05	9.0	0.006	3,056.62	3,056.30	3,043.31	3,043.16	12.20	11.93	9.18
SD-129	CB-179.5	3,052.28	MH-179	3,049.54	0.012	18 inch	18.0	2.54	27.1	0.101	3,060.58	3,056.00	3,052.88	3,050.31	6.80	4.96	11.80
SD-54	CB-182	3,043.58	MH-183	3,042.62	0.012	42 inch	42.0	37.79	193.5	0.005	3,056.60	3,056.91	3,045.49	3,044.84	9.52	10.79	7.95
SD-48	CB-180	3,045.98	CB-181	3,044.96	0.012	36 inch	36.0	33.31	205.0	0.005	3,056.28	3,056.29	3,048.10	3,047.92	7.30	8.33	7.68
SD-51	CB-181.5	3,049.73	CB-181	3,048.70	0.012	18 inch	18.0	2.96	193.5	0.005	3,056.23	3,056.29	3,050.38	3,049.32	5.00	6.09	4.30
SD-52	CB-181	3,044.66	CB-182	3,043.88	0.012	36 inch	36.0	37.39	156.5	0.005	3,056.29	3,056.60	3,046.80	3,046.64	8.63	9.72	7.88
SD-89	CB-199.1	3,051.60	MH-199	3,051.40	0.012	18 inch	18.0	3.26	38.0	0.005	3,055.82	3,056.62	3,052.29	3,052.06	2.72	3.72	4.40
SD-63	MH-199	3,048.58	CB-184	3,048.39	0.012	18 inch	18.0	5.86	38.2	0.005	3,056.62	3,056.62	3,049.53	3,049.32	6.54	6.73	4.96
SD-62	MH-198	3,050.19	MH-199	3,048.88	0.012	18 inch	18.0	2.60	260.8	0.005	3,056.82	3,056.62	3,050.80	3,049.76	5.13	6.24	4.07
SD-69	CB-51	3,003.01	MH-2	3,002.81	0.012	36 inch	36.0	24.64	41.0	0.005	3,011.00	3,017.82	3,004.61	3,004.30	4.99	12.01	7.09
SD-70	MH-2	3,000.54	MH-6	2,962.10	0.012	36 inch	36.0	24.64	151.5	0.254	3,017.82	2,970.00	3,002.14	2,964.02	14.28	4.90	29.37
SD-73	MH-6	2,962.06	OF-2	2,961.94	0.013	48	48.0	42.84	15.8	0.008	2,970.00	2,960.12	2,964.02	2,963.71	3.94	-5.82	9.02
SD-65	CB-52	3,004.00	MH-3	3,002.81	0.012	36 inch	36.0	18.20	237.8	0.005	3,010.00	3,017.75	3,005.37	3,004.19	3.00	11.94	6.62
SD-66	MH-3	3,002.51	MH-6	2,962.10	0.012	36 inch	36.0	18.20	159.2	0.254	3,017.75	2,970.00	3,003.88	2,964.02	12.24	4.90	26.83
SD-103	L.3	2,953.23	CB-60	2,917.96	0.012	36 inch	36.0	53.80	119.3	0.296	2,960.20	2,944.37	2,955.61	2,920.98	3.97	23.41	38.94
SD-104	CB-60	2,917.96	L.1	2,890.09	0.012	36 inch	36.0	54.44	80.3	0.347	2,944.37	2,899.53	2,920.36	2,893.79	23.41	6.44	41.36
SD-105	L.4	2,953.23	L.1	2,890.09	0.012	36 inch	36.0	57.47	199.6	0.316	2,960.20	2,899.53	2,955.69	2,893.79	3.97	6.44	40.65
SD-106	L.1	2,889.79	OF-3	2,889.66	0.013	48	48.0	111.91	12.8	0.010	2,899.53	2,887.90	2,892.99	2,892.63	5.74	-5.76	12.71
SD-109	CB-174	3,052.90	CB-176	3,052.25	0.012	12 inch	12.0	2.87	132.1	0.005	3,056.70	3,056.70	3,054.46	3,053.73	2.17	3.45	3.65
SD-112	CB-176	3,051.75	MH-169	3,051.40	0.012	18 inch	18.0	3.46	70.5	0.005	3,056.70	3,057.70	3,053.23	3,053.17	3.45	4.80	4.37
SD-113	CB-165	3,051.56	CB-167	3,051.16	0.012	24 inch	24.0	10.31	81.0	0.005	3,058.64	3,056.10	3,055.13	3,054.99	5.08	2.94	3.28
SD-114	CB-167	3,050.86	MH-169	3,050.53	0.012	24 inch	24.0	17.18	66.4	0.005	3,056.10	3,057.70	3,053.49	3,053.17	3.24	5.17	5.47
SD-115	CB-166	3,051.39	CB-167	3,051.16	0.012	24 inch	24.0	5.00	46.8	0.005	3,056.95	3,056.10	3,055.01	3,054.99	3.56	2.94	1.59
SD-116	CB-173	3,053.31	CB-174	3,053.20	0.012	12 inch	12.0	1.39	21.4	0.005	3,054.31	3,056.07	3,054.80	3,054.77	0.00	1.87	1.77
SD-117	MH-169	3,050.23	CB-178	3,049.57	0.012	24 inch	24.0	20.64	132.0	0.005	3,057.70	3,056.45	3,052.27	3,051.20	5.47	4.88	6.57
SD-119	CB-50	3,038.00	OF-7	3,037.59	0.012	18 inch	18.0	0.94	62.8	0.007	3,042.90	3,037.59	3,038.36	3,037.91	3.40	-1.50	3.35
SD-120	CB-49	3,037.98	OF-5	3,037.59	0.012	18 inch	18.0	3.48	59.8	0.007	3,042.90	3,037.59	3,038.69	3,038.23	3.42	-1.50	4.84
SD-110	CB-61	2,935.76	OF-8	2,928.23	0.012	12 inch	12.0	0.62	57.1	0.132	2,939.62	2,931.00	2,936.09	2,928.37	2.86	1.77	8.95
SD-123	CB-63	2,982.76	OF-9	2,982.53	0.012	12 inch	12.0	0.73	42.4	0.005	3,014.35	2,984.00	2,983.12	2,982.88	30.59	0.47	3.03
SD-30	CB-65	3,037.82	MH-38	3,012.68	0.012	18 inch	18.0	0.82	65.1	0.386	3,043.00	3,017.80	3,038.16	3,016.62	3.68	3.62	13.44
SD-30A	MH-38	3,010.14	OF-10	3,010.00	0.012	18 inch	18.0	0.82	25.5	0.005	3,017.80	3,012.00	3,016.12	3,016.12	6.16	0.50	0.46

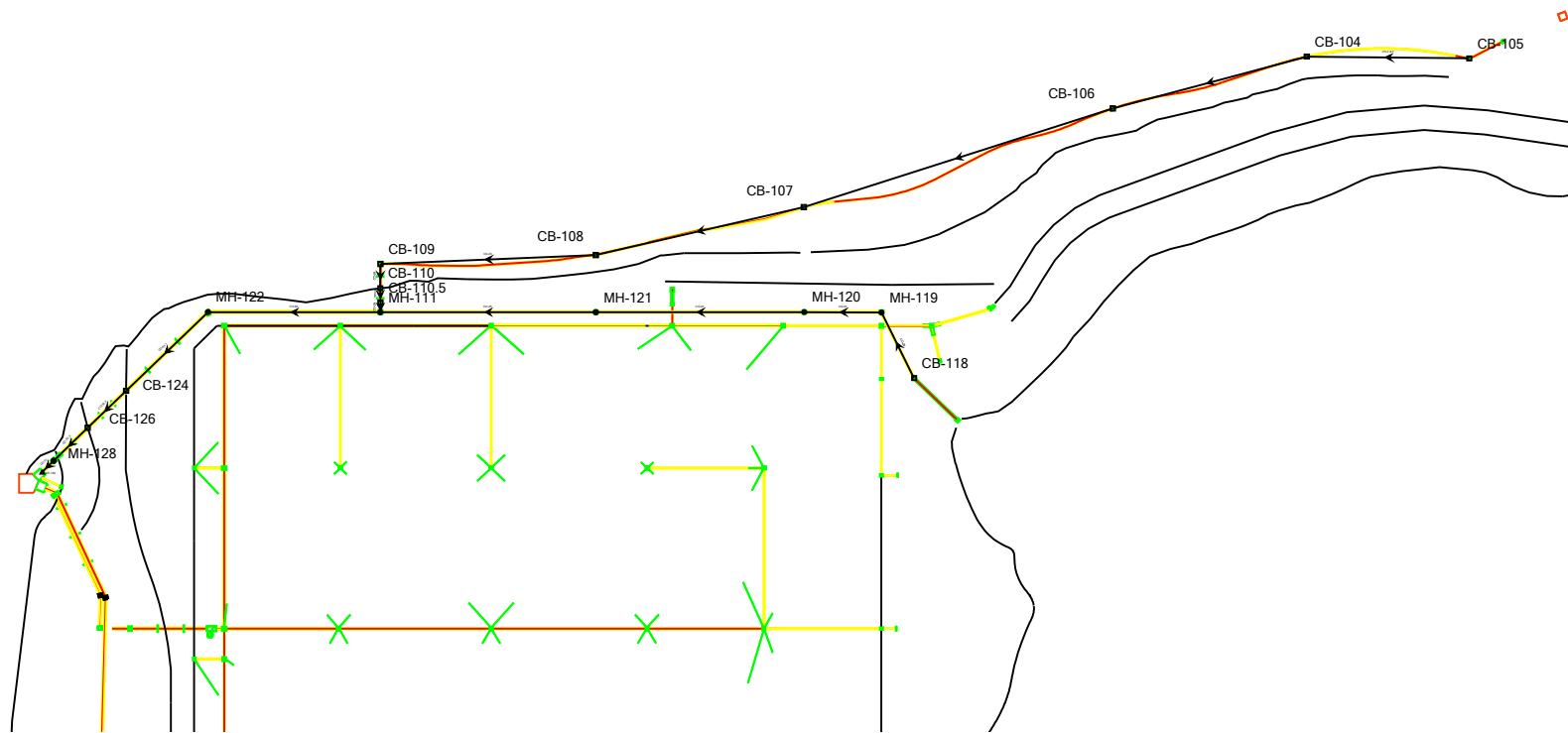
FlexTable: Conduit Table (Basin200.stc)

Label	Start Node	Invert (Upstream) (ft)	Stop Node	Invert (Downstream) (ft)	Manning's n	Section Size	Diameter (in)	Flow (ft³/s)	Length (Unified) (ft)	Slope (Calculate d) (ft/ft)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Cover (Start) (ft)	Cover (Stop) (ft)	Velocity (Average) (ft/s)
CO-5	CB-218	3,049.54	CB-220	3,048.17	0.012	18 inch	18.0	2.69	274.5	0.005	3,056.04	3,055.28	3,050.16	3,049.47	5.00	5.61	4.10
CO-6	CB-230	3,049.85	CB-220	3,048.88	0.012	18 inch	18.0	1.50	195.5	0.005	3,056.60	3,055.28	3,050.31	3,049.47	5.25	4.90	3.48
CO-7	CB-220	3,043.48	MH-232	3,043.44	0.013	24 inch	24.0	5.59	7.5	0.005	3,055.28	3,056.36	3,049.40	3,049.40	9.80	10.92	1.78
CO-8	MH-232	3,043.44	OF-234	3,043.00	0.013	24 inch	24.0	5.59	76.4	0.006	3,056.36	3,048.00	3,048.44	3,048.39	10.92	3.00	1.78

FlexTable: Conduit Table (Basin300.stc)

Label	Start Node	Invert (Upstream) (ft)	Stop Node	Invert (Downstream) (ft)	Manning's n	Section Size	Diameter (in)	Flow (ft³/s)	Length (Unified) (ft)	Slope (Calculate d) (ft/ft)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Cover (Start) (ft)	Cover (Stop) (ft)	Velocity (Average) (ft/s)
CO-1	CB-303	3,053.38	CB-304	3,051.31	0.012	12 inch	12.0	1.38	66.0	0.031	3,057.40	3,056.14	3,053.88	3,052.30	3.02	3.83	6.81
CO-2	CB-304	3,050.81	CB-305	3,050.40	0.012	18 inch	18.0	3.56	82.7	0.005	3,056.14	3,057.19	3,052.23	3,052.15	3.83	5.29	4.40
CO-3	CB-306	3,051.50	CB-305	3,050.40	0.012	18 inch	18.0	2.58	24.7	0.045	3,056.50	3,057.19	3,052.11	3,052.15	3.50	5.29	8.88
CO-4	CB-305	3,050.40	CB-307	3,049.79	0.012	18 inch	18.0	5.95	120.9	0.005	3,057.19	3,056.22	3,051.89	3,051.56	5.29	4.93	5.00
CO-7	CB-307	3,049.51	CB-308	3,048.71	0.012	18 inch	18.0	7.86	159.5	0.005	3,056.22	3,055.87	3,051.10	3,050.34	5.21	5.66	4.45
CO-8	CB-308.5	3,049.77	CB-308	3,048.70	0.012	18 inch	18.0	2.14	206.5	0.005	3,056.27	3,055.87	3,050.35	3,050.34	5.00	5.67	3.90
CO-9	CB-308	3,048.21	CB-309	3,046.78	0.012	24 inch	24.0	12.83	286.5	0.005	3,055.87	3,055.86	3,049.50	3,048.06	5.66	7.08	6.03
CO-10	CB-313	3,049.43	CB-309	3,048.30	0.012	18 inch	18.0	3.73	212.7	0.005	3,055.93	3,055.86	3,050.17	3,049.01	5.00	6.06	4.57
CO-11	CB-309	3,045.78	CB-314	3,044.50	0.012	36 inch	36.0	19.42	256.0	0.005	3,055.86	3,055.87	3,047.20	3,047.30	7.08	8.37	6.73
CO-12	CB325	3,049.42	CB-314	3,048.33	0.012	18 inch	18.0	3.73	212.2	0.005	3,055.92	3,055.87	3,050.16	3,049.04	5.00	6.04	4.51
CO-13	CB-319	3,048.10	CB-314	3,046.70	0.012	18 inch	18.0	9.53	265.4	0.005	3,056.07	3,055.87	3,049.84	3,047.89	6.47	7.67	5.39
CO-14	CB-318	3,049.50	CB-319	3,048.40	0.012	18 inch	18.0	3.39	211.8	0.005	3,056.06	3,056.07	3,050.68	3,050.51	5.06	6.17	4.42
CO-15	CB-320	3,049.59	CB-319	3,048.40	0.012	18 inch	18.0	3.09	207.6	0.006	3,056.09	3,056.07	3,050.63	3,050.51	5.00	6.17	4.47
CO-16	CB-314	3,044.19	CB-326	3,043.00	0.012	36 inch	36.0	34.42	235.5	0.005	3,055.87	3,056.20	3,046.46	3,046.13	8.68	10.20	7.79
CO-17	MH-331	3,050.52	CB-326	3,050.44	0.012	12 inch	12.0	1.17	17.1	0.005	3,056.65	3,056.20	3,050.99	3,050.90	5.13	4.76	3.26
CO-19	CB-330	3,085.02	MH-331	3,050.82	0.012	12 inch	12.0	1.17	56.1	0.610	3,088.94	3,056.65	3,085.48	3,050.95	2.92	4.83	18.55
CO-20	CB-326	3,042.17	CB-332	3,041.10	0.012	36 inch	36.0	36.26	214.0	0.005	3,056.20	3,056.00	3,045.52	3,044.98	11.03	11.90	5.13
CO-21	CB-332	3,041.10	MH-333	3,040.73	0.012	36 inch	36.0	37.37	73.9	0.005	3,056.00	3,057.27	3,044.77	3,044.57	11.90	13.54	5.29
CO-22	MH-333	3,040.43	CB-335	3,039.84	0.012	36 inch	36.0	37.37	117.8	0.005	3,057.27	3,056.37	3,044.35	3,044.04	13.84	13.53	5.29
CO-23	CB-335	3,039.84	CB-336	3,038.98	0.012	36 inch	36.0	38.06	172.4	0.005	3,056.37	3,056.44	3,043.81	3,043.33	13.53	14.46	5.38
CO-24	CB-336	3,038.68	CB-337	3,038.02	0.012	42 inch	42.0	38.66	132.0	0.005	3,056.44	3,056.60	3,043.21	3,043.04	14.26	15.08	4.02
CO-25	CB-348	3,049.38	CB-349	3,048.30	0.012	18 inch	18.0	4.00	274.7	0.004	3,055.88	3,055.84	3,050.21	3,049.92	5.00	6.04	4.15
CO-26	CB-349	3,048.00	MH-350	3,046.74	0.012	18 inch	18.0	7.64	191.9	0.007	3,055.84	3,056.16	3,049.78	3,048.91	6.34	7.92	4.32
CO-29	CB-351	3,052.20	MH-350	3,052.00	0.012	18 inch	18.0	1.69	37.5	0.005	3,056.18	3,056.16	3,052.69	3,052.46	2.48	2.66	3.69
CO-30	MH-350	3,046.12	MH-354	3,044.96	0.012	18 inch	18.0	9.33	232.1	0.005	3,056.16	3,056.72	3,048.52	3,046.96	8.54	10.26	5.28
CO-31	CB-361	3,049.49	CB-362	3,048.63	0.012	18 inch	18.0	3.27	171.1	0.005	3,055.99	3,056.04	3,050.18	3,049.48	5.00	5.91	4.33
CO-32	CB-362	3,048.33	MH-354	3,047.66	0.012	18 inch	18.0	5.95	132.6	0.005	3,056.04	3,056.72	3,049.29	3,048.60	6.21	7.56	5.00
CO-33	CB-355	3,051.47	MH-354	3,051.28	0.012	18 inch	18.0	2.04	37.5	0.005	3,055.97	3,056.72	3,052.01	3,051.79	3.00	3.94	3.82
CO-34	MH-354	3,044.66	MH-363	3,043.38	0.012	24 inch	24.0	17.32	256.0	0.005	3,056.72	3,056.72	3,046.42	3,045.36	10.06	11.34	6.29
CO-35	CB-369	3,049.49	CB-370	3,048.64	0.012	18 inch	18.0	3.22	171.1	0.005	3,055.99	3,056.05	3,050.17	3,049.49	5.00	5.91	4.29
CO-37	CB-370	3,048.34	MH-363	3,047.68	0.012	18 inch	18.0	5.90	132.5	0.005	3,056.05	3,056.72	3,049.30	3,048.62	6.21	7.54	4.97
CO-38	CB-364	3,051.45	MH-363	3,051.20	0.012	18 inch	18.0	2.12	37.5	0.007	3,055.95	3,056.72	3,052.00	3,051.69	3.00	4.02	4.26
CO-39	MH-363	3,043.08	CB-337	3,041.20	0.012	36 inch	36.0	25.34	235.0	0.008	3,056.72	3,056.60	3,044.70	3,043.04	10.64	12.40	8.59
CO-40	CB-337	3,037.72	MH-371	3,037.64	0.012	42 inch	42.0	61.72	14.5	0.006	3,056.60	3,056.70	3,042.08	3,042.03	15.38	15.56	6.42
CO-41	MH-371	3,037.64	MH-372	3,037.55	0.012	42 inch	42.0	61.72	18.0	0.005	3,056.70	3,056.80	3,040.41	3,040.39	15.56	15.75	8.89
CO-42	MH-372	3,037.25	CB-373	3,009.48	0.012	42 inch	42.0	61.72	78.5	0.354	3,056.80	3,024.56	3,039.71	3,012.52	16.05	11.58	42.46
CO-43	CB-373	3,009.48	CB-374	2,985.80	0.012	42 inch	42.0	62.28	67.0	0.354	3,024.56	2,997.00	3,011.95	2,988.91	11.58	7.70	42.57
CO-44	CB-374	2,985.80	MH-376	2,962.04	0.012	42 inch	42.0	64.48	64.4	0.369	2,997.00	2,967.92	2,988.32	2,966.04	7.70	2.38	43.65
CO-45	MH-376	2,961.03	OF-378	2,959.80	0.013	48 inch	48.0	64.48	29.7	0.041	2,967.92	2,965.90	2,965.84	2,965.78	2.89	2.10	5.13

Scenario: 100-Year



FlexTable: Conduit Table (Rev Offsite Storm Drain.stc)

Label	Start Node	Invert (Upstream) (ft)	Stop Node	Invert (Downstream) (ft)	Manning's n	Section Size	Diameter (in)	Flow (ft³/s)	Length (Unified) (ft)	Slope (Calculate d) (ft/ft)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Cover (Start) (ft)	Cover (Stop) (ft)	Velocity (Average) (ft/s)
CO-32	MH-119	3,047.02	MH-120	3,046.50	0.012	24 inch	24.0	17.52	103.2	0.005	3,057.64	3,057.34	3,048.68	3,048.17	8.62	8.84	6.31
CO-42	MH-120	3,046.20	MH-121	3,044.78	0.012	24 inch	24.0	17.52	284.6	0.005	3,057.34	3,057.64	3,047.86	3,046.44	9.14	10.86	6.28
CO-45	MH-121	3,044.48	MH-111	3,043.00	0.012	24 inch	24.0	17.52	294.5	0.005	3,057.64	3,057.50	3,046.14	3,044.65	11.16	12.50	6.30
CO-50	MH-111	3,041.50	MH-122	3,040.33	0.012	42 inch	42.0	58.31	235.0	0.005	3,057.50	3,057.41	3,043.89	3,043.07	12.50	13.58	8.79
CO-53.1	MH-122	3,040.03	CB-124	2,998.01	0.012	42 inch	42.0	58.31	155.7	0.270	3,057.41	3,024.68	3,042.42	3,000.94	13.88	23.17	37.96
CO-53.2	CB-124	2,998.01	CB-126	2,978.32	0.012	42 inch	42.0	58.31	73.0	0.270	3,024.68	2,990.78	3,000.40	2,981.25	23.17	8.96	37.96
CO-53.3	CB-126	2,978.32	MH-128	2,962.19	0.012	42 inch	42.0	58.31	63.8	0.253	2,990.78	2,970.00	2,980.71	2,965.05	8.96	4.31	37.09
CO-72	MH-128	2,962.12	OF-130	2,961.93	0.013	42 inch	42.0	58.31	19.0	0.010	2,970.00	2,960.19	2,964.51	2,964.06	4.38	-5.24	10.83
CO-2 & 3	CB-105	3,142.71	CB-104	3,140.48	0.012	18 inch	18.0	0.79	223.5	0.010	3,151.00	3,146.24	3,143.04	3,141.66	6.79	4.26	3.70
CO-4, 5, 6 & 7	CB-104	3,140.33	CB-106	3,127.96	0.012	18 inch	18.0	7.73	276.0	0.045	3,146.24	3,135.38	3,141.41	3,129.63	4.41	5.92	12.14
CO-8-15	CB-106	3,127.66	CB-107	3,108.92	0.012	18 inch	18.0	14.75	449.4	0.042	3,135.38	3,114.38	3,129.06	3,109.79	6.22	3.96	13.92
CO-22	CB-107	3,107.98	CB-108	3,091.29	0.012	24 inch	24.0	24.48	292.0	0.057	3,114.38	3,098.87	3,109.73	3,093.79	4.40	5.58	17.81
CO-23	CB-108	3,090.99	CB-109	3,075.41	0.012	24 inch	24.0	33.07	295.5	0.053	3,098.87	3,082.68	3,092.89	3,080.66	5.88	5.27	18.63
CO-25	CB-118	3,048.38	MH-119	3,047.32	0.012	24 inch	24.0	17.52	95.4	0.011	3,058.23	3,057.64	3,049.89	3,049.05	7.85	8.32	8.84
CO-26	CB-109	3,075.11	CB-110	3,059.45	0.012	24 inch	24.0	38.76	30.3	0.516	3,082.68	3,073.27	3,077.06	3,061.90	5.57	11.82	44.96
CO-28	CB-110	3,059.45	CB-110.5	3,050.09	0.012	24 inch	24.0	40.45	21.0	0.445	3,073.27	3,060.00	3,061.40	3,052.55	11.82	7.91	43.13
CO-29	CB-110.5	3,050.09	MH-111	3,045.00	0.012	24 inch	24.0	40.79	12.1	0.422	3,060.00	3,057.50	3,052.05	3,046.11	7.91	10.50	42.43

Culvert Calculator Report

SD01-Public (12+59.60)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,228.24 ft	Headwater Depth/Height	1.24
Computed Headwater Elev.	3,228.23 ft	Discharge	16.32 cfs
Inlet Control HW Elev.	3,228.09 ft	Tailwater Elevation	3,222.44 ft
Outlet Control HW Elev.	3,228.23 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,225.74 ft	Downstream Invert	3,221.94 ft
Length	56.43 ft	Constructed Slope	0.067340 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.76 ft
Slope Type	Steep	Normal Depth	0.69 ft
Flow Regime	Supercritical	Critical Depth	1.46 ft
Velocity Downstream	14.95 ft/s	Critical Slope	0.005721 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	2.00 ft
Section Size	24 inch	Rise	2.00 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,228.23 ft	Upstream Velocity Head	0.69 ft
Ke	0.50	Entrance Loss	0.34 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,228.09 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	3.1 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD01 (31+32.45)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,146.53 ft	Headwater Depth/Height	1.21
Computed Headwater Elev.	3,146.14 ft	Discharge	15.58 cfs
Inlet Control HW Elev.	3,145.99 ft	Tailwater Elevation	3,141.28 ft
Outlet Control HW Elev.	3,146.14 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,143.73 ft	Downstream Invert	3,140.78 ft
Length	44.92 ft	Constructed Slope	0.065672 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.77 ft
Slope Type	Steep	Normal Depth	0.68 ft
Flow Regime	Supercritical	Critical Depth	1.42 ft
Velocity Downstream	14.06 ft/s	Critical Slope	0.005527 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	2.00 ft
Section Size	24 inch	Rise	2.00 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,146.14 ft	Upstream Velocity Head	0.66 ft
Ke	0.50	Entrance Loss	0.33 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,145.99 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	3.1 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD02-Public (17+60)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,189.36 ft	Headwater Depth/Height	1.12
Computed Headwater Elev.	3,189.36 ft	Discharge	24.40 cfs
Inlet Control HW Elev.	3,189.21 ft	Tailwater Elevation	3,186.59 ft
Outlet Control HW Elev.	3,189.36 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,186.55 ft	Downstream Invert	3,186.09 ft
Length	35.73 ft	Constructed Slope	0.012874 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	1.35 ft
Slope Type	Steep	Normal Depth	1.23 ft
Flow Regime	Supercritical	Critical Depth	1.68 ft
Velocity Downstream	9.02 ft/s	Critical Slope	0.004777 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	2.50 ft
Section Size	30 inch	Rise	2.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,189.36 ft	Upstream Velocity Head	0.75 ft
Ke	0.50	Entrance Loss	0.37 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,189.21 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	4.9 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD02 (33+37.23)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,148.91 ft	Headwater Depth/Height	0.69
Computed Headwater Elev.	3,146.97 ft	Discharge	3.01 cfs
Inlet Control HW Elev.	3,146.81 ft	Tailwater Elevation	3,143.25 ft
Outlet Control HW Elev.	3,146.97 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,145.93 ft	Downstream Invert	3,142.75 ft
Length	43.34 ft	Constructed Slope	0.073373 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.32 ft
Slope Type	Steep	Normal Depth	0.32 ft
Flow Regime	Supercritical	Critical Depth	0.66 ft
Velocity Downstream	10.91 ft/s	Critical Slope	0.004369 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	1.50 ft
Section Size	18 inch	Rise	1.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,146.97 ft	Upstream Velocity Head	0.25 ft
Ke	0.50	Entrance Loss	0.13 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,146.81 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	1.8 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD03 (38+01.54)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,125.58 ft	Headwater Depth/Height	1.48
Computed Headwater Elev.	3,125.46 ft	Discharge	36.14 cfs
Inlet Control HW Elev.	3,125.46 ft	Tailwater Elevation	3,122.25 ft
Outlet Control HW Elev.	3,125.45 ft	Control Type	Inlet Control
Grades			
Upstream Invert	3,121.75 ft	Downstream Invert	3,117.30 ft
Length	45.50 ft	Constructed Slope	0.097802 ft/ft
Hydraulic Profile			
Profile CompositePressureProfileS1S2		Depth, Downstream	1.06 ft
Slope Type	N/A	Normal Depth	0.87 ft
Flow Regime	N/A	Critical Depth	2.04 ft
Velocity Downstream	18.30 ft/s	Critical Slope	0.006676 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	2.50 ft
Section Size	30 inch	Rise	2.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,125.45 ft	Upstream Velocity Head	1.10 ft
Ke	0.50	Entrance Loss	0.55 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,125.46 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	4.9 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD04 (40+34.69)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,125.10 ft	Headwater Depth/Height	1.58
Computed Headwater Elev.	3,124.19 ft	Discharge	42.77 cfs
Inlet Control HW Elev.	3,124.19 ft	Tailwater Elevation	3,120.63 ft
Outlet Control HW Elev.	3,124.06 ft	Control Type	Inlet Control
Grades			
Upstream Invert	3,121.03 ft	Downstream Invert	3,120.13 ft
Length	44.94 ft	Constructed Slope	0.020027 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	1.25 ft
Slope Type	Steep	Normal Depth	1.14 ft
Flow Regime	Supercritical	Critical Depth	1.66 ft
Velocity Downstream	10.39 ft/s	Critical Slope	0.007488 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	2.00 ft
Section Size	24 inch	Rise	2.00 ft
Number Sections	2		
Outlet Control Properties			
Outlet Control HW Elev.	3,124.06 ft	Upstream Velocity Head	0.92 ft
Ke	0.50	Entrance Loss	0.46 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,124.19 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	6.3 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD05

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,076.90 ft	Headwater Depth/Height	1.63
Computed Headwater Elev.	3,076.90 ft	Discharge	10.82 cfs
Inlet Control HW Elev.	3,076.90 ft	Tailwater Elevation	3,073.92 ft
Outlet Control HW Elev.	3,076.80 ft	Control Type	Inlet Control
Grades			
Upstream Invert	3,074.45 ft	Downstream Invert	3,071.92 ft
Length	38.05 ft	Constructed Slope	0.066491 ft/ft
Hydraulic Profile			
Profile Composite	Pressure Profile	Depth, Downstream	0.71 ft
Slope Type	N/A	Normal Depth	0.63 ft
Flow Regime	N/A	Critical Depth	1.26 ft
Velocity Downstream	13.16 ft/s	Critical Slope	0.008656 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	1.50 ft
Section Size	18 inch	Rise	1.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,076.80 ft	Upstream Velocity Head	0.72 ft
Ke	0.50	Entrance Loss	0.36 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,076.90 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	1.8 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD06 (56+21.82)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,055.82 ft	Headwater Depth/Height	1.40
Computed Headwater Elev.	3,055.54 ft	Discharge	33.67 cfs
Inlet Control HW Elev.	3,055.49 ft	Tailwater Elevation	3,048.80 ft
Outlet Control HW Elev.	3,055.54 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,052.04 ft	Downstream Invert	3,048.30 ft
Length	47.80 ft	Constructed Slope	0.078243 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	1.06 ft
Slope Type	Steep	Normal Depth	0.89 ft
Flow Regime	Supercritical	Critical Depth	1.97 ft
Velocity Downstream	16.93 ft/s	Critical Slope	0.006171 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	2.50 ft
Section Size	30 inch	Rise	2.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,055.54 ft	Upstream Velocity Head	1.02 ft
Ke	0.50	Entrance Loss	0.51 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,055.49 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	4.9 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD07 (59+66.37)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,078.66 ft	Headwater Depth/Height	1.46
Computed Headwater Elev.	3,078.17 ft	Discharge	9.94 cfs
Inlet Control HW Elev.	3,078.16 ft	Tailwater Elevation	3,072.10 ft
Outlet Control HW Elev.	3,078.17 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,075.98 ft	Downstream Invert	3,071.60 ft
Length	40.30 ft	Constructed Slope	0.108685 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.58 ft
Slope Type	Steep	Normal Depth	0.53 ft
Flow Regime	Supercritical	Critical Depth	1.22 ft
Velocity Downstream	15.65 ft/s	Critical Slope	0.007789 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	1.50 ft
Section Size	18 inch	Rise	1.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,078.17 ft	Upstream Velocity Head	0.65 ft
Ke	0.50	Entrance Loss	0.33 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,078.16 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	1.8 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD08 (70+82.70)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,071.37 ft	Headwater Depth/Height	1.16
Computed Headwater Elev.	3,070.13 ft	Discharge	14.71 cfs
Inlet Control HW Elev.	3,070.00 ft	Tailwater Elevation	3,067.06 ft
Outlet Control HW Elev.	3,070.13 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,067.81 ft	Downstream Invert	3,066.56 ft
Length	38.54 ft	Constructed Slope	0.032434 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.90 ft
Slope Type	Steep	Normal Depth	0.80 ft
Flow Regime	Supercritical	Critical Depth	1.38 ft
Velocity Downstream	10.74 ft/s	Critical Slope	0.005315 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	2.00 ft
Section Size	24 inch	Rise	2.00 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,070.13 ft	Upstream Velocity Head	0.63 ft
Ke	0.50	Entrance Loss	0.31 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,070.00 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	3.1 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD09 (73+01.11)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,067.18 ft	Headwater Depth/Height	1.29
Computed Headwater Elev.	3,066.86 ft	Discharge	139.02 cfs
Inlet Control HW Elev.	3,066.69 ft	Tailwater Elevation	3,061.57 ft
Outlet Control HW Elev.	3,066.86 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,062.35 ft	Downstream Invert	3,060.21 ft
Length	42.00 ft	Constructed Slope	0.050952 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	1.65 ft
Slope Type	Steep	Normal Depth	1.27 ft
Flow Regime	Supercritical	Critical Depth	2.61 ft
Velocity Downstream	15.63 ft/s	Critical Slope	0.004944 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	3.50 ft
Section Size	42 inch	Rise	3.50 ft
Number Sections	2		
Outlet Control Properties			
Outlet Control HW Elev.	3,066.86 ft	Upstream Velocity Head	1.27 ft
Ke	0.50	Entrance Loss	0.63 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,066.69 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	19.2 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD10 (79+71.02)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,094.50 ft	Headwater Depth/Height	0.45
Computed Headwater Elev.	3,091.98 ft	Discharge	1.38 cfs
Inlet Control HW Elev.	3,091.87 ft	Tailwater Elevation	309.45 ft
Outlet Control HW Elev.	3,091.98 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,091.30 ft	Downstream Invert	3,088.95 ft
Length	44.90 ft	Constructed Slope	0.052339 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.23 ft
Slope Type	Steep	Normal Depth	0.23 ft
Flow Regime	Supercritical	Critical Depth	0.44 ft
Velocity Downstream	7.81 ft/s	Critical Slope	0.004180 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	1.50 ft
Section Size	18 inch	Rise	1.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,091.98 ft	Upstream Velocity Head	0.16 ft
Ke	0.50	Entrance Loss	0.08 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,091.87 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	1.8 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD11 (83+06.52)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,084.33 ft	Headwater Depth/Height	1.23
Computed Headwater Elev.	3,083.23 ft	Discharge	16.02 cfs
Inlet Control HW Elev.	3,083.11 ft	Tailwater Elevation	3,079.68 ft
Outlet Control HW Elev.	3,083.23 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,080.77 ft	Downstream Invert	3,079.18 ft
Length	41.00 ft	Constructed Slope	0.038780 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.90 ft
Slope Type	Steep	Normal Depth	0.79 ft
Flow Regime	Supercritical	Critical Depth	1.44 ft
Velocity Downstream	11.66 ft/s	Critical Slope	0.005641 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	2.00 ft
Section Size	24 inch	Rise	2.00 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,083.23 ft	Upstream Velocity Head	0.68 ft
Ke	0.50	Entrance Loss	0.34 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,083.11 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	3.1 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD12 (86+18.61)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,067.78 ft	Headwater Depth/Height	1.77
Computed Headwater Elev.	3,067.78 ft	Discharge	23.58 cfs
Inlet Control HW Elev.	3,067.78 ft	Tailwater Elevation	3,061.67 ft
Outlet Control HW Elev.	3,067.54 ft	Control Type	Inlet Control
Grades			
Upstream Invert	3,064.25 ft	Downstream Invert	3,061.17 ft
Length	60.40 ft	Constructed Slope	0.050993 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	1.02 ft
Slope Type	Steep	Normal Depth	0.91 ft
Flow Regime	Supercritical	Critical Depth	1.72 ft
Velocity Downstream	14.71 ft/s	Critical Slope	0.008551 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	2.00 ft
Section Size	24 inch	Rise	2.00 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,067.54 ft	Upstream Velocity Head	1.04 ft
Ke	0.50	Entrance Loss	0.52 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,067.78 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	3.1 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD13 (89+63.56)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,055.89 ft	Headwater Depth/Height	1.66
Computed Headwater Elev.	3,055.43 ft	Discharge	11.04 cfs
Inlet Control HW Elev.	3,055.43 ft	Tailwater Elevation	3,051.33 ft
Outlet Control HW Elev.	3,055.32 ft	Control Type	Inlet Control
Grades			
Upstream Invert	3,052.94 ft	Downstream Invert	3,049.13 ft
Length	45.00 ft	Constructed Slope	0.084667 ft/ft
Hydraulic Profile			
Profile Composite	Pressure Profile S1 S2	Depth, Downstream	0.66 ft
Slope Type	N/A	Normal Depth	0.60 ft
Flow Regime	N/A	Critical Depth	1.27 ft
Velocity Downstream	14.85 ft/s	Critical Slope	0.008895 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	1.50 ft
Section Size	18 inch	Rise	1.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,055.32 ft	Upstream Velocity Head	0.74 ft
Ke	0.50	Entrance Loss	0.37 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,055.43 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	1.8 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD14 (95+06.72)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,073.91 ft	Headwater Depth/Height	1.57
Computed Headwater Elev.	3,073.67 ft	Discharge	21.39 cfs
Inlet Control HW Elev.	3,073.67 ft	Tailwater Elevation	3,068.73 ft
Outlet Control HW Elev.	3,073.58 ft	Control Type	Inlet Control
Grades			
Upstream Invert	3,070.54 ft	Downstream Invert	3,068.23 ft
Length	43.70 ft	Constructed Slope	0.052860 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.99 ft
Slope Type	Steep	Normal Depth	0.85 ft
Flow Regime	Supercritical	Critical Depth	1.66 ft
Velocity Downstream	13.79 ft/s	Critical Slope	0.007490 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	2.00 ft
Section Size	24 inch	Rise	2.00 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,073.58 ft	Upstream Velocity Head	0.92 ft
Ke	0.50	Entrance Loss	0.46 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,073.67 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	3.1 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD15 (101+17.00)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,083.96 ft	Headwater Depth/Height	0.88
Computed Headwater Elev.	3,082.39 ft	Discharge	4.54 cfs
Inlet Control HW Elev.	3,082.26 ft	Tailwater Elevation	3,079.60 ft
Outlet Control HW Elev.	3,082.39 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,081.08 ft	Downstream Invert	3,079.10 ft
Length	40.10 ft	Constructed Slope	0.049377 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.45 ft
Slope Type	Steep	Normal Depth	0.43 ft
Flow Regime	Supercritical	Critical Depth	0.82 ft
Velocity Downstream	10.13 ft/s	Critical Slope	0.004771 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	1.50 ft
Section Size	18 inch	Rise	1.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,082.39 ft	Upstream Velocity Head	0.33 ft
Ke	0.50	Entrance Loss	0.16 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,082.26 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	1.8 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD16 (103+20.95)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,082.06 ft	Headwater Depth/Height	0.68
Computed Headwater Elev.	3,080.22 ft	Discharge	2.92 cfs
Inlet Control HW Elev.	3,080.09 ft	Tailwater Elevation	3,076.98 ft
Outlet Control HW Elev.	3,080.22 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,079.20 ft	Downstream Invert	3,077.00 ft
Length	43.97 ft	Constructed Slope	0.050034 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.35 ft
Slope Type	Steep	Normal Depth	0.34 ft
Flow Regime	Supercritical	Critical Depth	0.65 ft
Velocity Downstream	9.41 ft/s	Critical Slope	0.004361 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	1.50 ft
Section Size	18 inch	Rise	1.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,080.22 ft	Upstream Velocity Head	0.25 ft
Ke	0.50	Entrance Loss	0.12 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,080.09 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	1.8 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD17 (107+36.02)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,069.12 ft	Headwater Depth/Height	1.15
Computed Headwater Elev.	3,068.20 ft	Discharge	25.27 cfs
Inlet Control HW Elev.	3,068.01 ft	Tailwater Elevation	3,063.75 ft
Outlet Control HW Elev.	3,068.20 ft	Control Type	Entrance Control
Grades			
Upstream Invert	3,065.33 ft	Downstream Invert	3,062.81 ft
Length	46.70 ft	Constructed Slope	0.053961 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.98 ft
Slope Type	Steep	Normal Depth	0.84 ft
Flow Regime	Supercritical	Critical Depth	1.71 ft
Velocity Downstream	14.13 ft/s	Critical Slope	0.004882 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	2.50 ft
Section Size	30 inch	Rise	2.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,068.20 ft	Upstream Velocity Head	0.77 ft
Ke	0.50	Entrance Loss	0.39 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,068.01 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	4.9 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Culvert Calculator Report

SD20 (123+23.61)

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,083.14 ft	Headwater Depth/Height	3.02
Computed Headwater Elev.	3,083.13 ft	Discharge	16.88 cfs
Inlet Control HW Elev.	3,083.13 ft	Tailwater Elevation	3,071.80 ft
Outlet Control HW Elev.	3,082.23 ft	Control Type	Inlet Control
Grades			
Upstream Invert	3,078.60 ft	Downstream Invert	3,071.30 ft
Length	52.64 ft	Constructed Slope	0.138678 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	0.73 ft
Slope Type	Steep	Normal Depth	0.66 ft
Flow Regime	Supercritical	Critical Depth	1.44 ft
Velocity Downstream	19.90 ft/s	Critical Slope	0.019150 ft/ft
Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	HDPE (Smooth Interior)	Span	1.50 ft
Section Size	18 inch	Rise	1.50 ft
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	3,082.23 ft	Upstream Velocity Head	1.46 ft
Ke	0.50	Entrance Loss	0.73 ft
Inlet Control Properties			
Inlet Control HW Elev.	3,083.13 ft	Flow Control	N/A
Inlet Type	Square edge w/headwall	Area Full	1.8 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

BASIN 100

RUN DATE 11/9/2009
HYDROGRAPH FILE NAME Text1
TIME OF CONCENTRATION 10 MIN.
6 HOUR RAINFALL 3.6 INCHES
BASIN AREA 22.5 ACRES
RUNOFF COEFFICIENT 0.52
PEAK DISCHARGE 71.27 CFS

TIME (MIN) = 0	DISCHARGE (CFS) = 0
TIME (MIN) = 10	DISCHARGE (CFS) = 2.5
TIME (MIN) = 20	DISCHARGE (CFS) = 2.6
TIME (MIN) = 30	DISCHARGE (CFS) = 2.7
TIME (MIN) = 40	DISCHARGE (CFS) = 2.7
TIME (MIN) = 50	DISCHARGE (CFS) = 2.8
TIME (MIN) = 60	DISCHARGE (CFS) = 2.9
TIME (MIN) = 70	DISCHARGE (CFS) = 3
TIME (MIN) = 80	DISCHARGE (CFS) = 3.1
TIME (MIN) = 90	DISCHARGE (CFS) = 3.3
TIME (MIN) = 100	DISCHARGE (CFS) = 3.4
TIME (MIN) = 110	DISCHARGE (CFS) = 3.6
TIME (MIN) = 120	DISCHARGE (CFS) = 3.7
TIME (MIN) = 130	DISCHARGE (CFS) = 4
TIME (MIN) = 140	DISCHARGE (CFS) = 4.1
TIME (MIN) = 150	DISCHARGE (CFS) = 4.5
TIME (MIN) = 160	DISCHARGE (CFS) = 4.7
TIME (MIN) = 170	DISCHARGE (CFS) = 5.2
TIME (MIN) = 180	DISCHARGE (CFS) = 5.5
TIME (MIN) = 190	DISCHARGE (CFS) = 6.3
TIME (MIN) = 200	DISCHARGE (CFS) = 6.9
TIME (MIN) = 210	DISCHARGE (CFS) = 8.4
TIME (MIN) = 220	DISCHARGE (CFS) = 9.6
TIME (MIN) = 230	DISCHARGE (CFS) = 14.1
TIME (MIN) = 240	DISCHARGE (CFS) = 19.5
TIME (MIN) = 250	DISCHARGE (CFS) = 71.27
TIME (MIN) = 260	DISCHARGE (CFS) = 11.3
TIME (MIN) = 270	DISCHARGE (CFS) = 7.5
TIME (MIN) = 280	DISCHARGE (CFS) = 5.9
TIME (MIN) = 290	DISCHARGE (CFS) = 4.9
TIME (MIN) = 300	DISCHARGE (CFS) = 4.3
TIME (MIN) = 310	DISCHARGE (CFS) = 3.8
TIME (MIN) = 320	DISCHARGE (CFS) = 3.5
TIME (MIN) = 330	DISCHARGE (CFS) = 3.2
TIME (MIN) = 340	DISCHARGE (CFS) = 3
TIME (MIN) = 350	DISCHARGE (CFS) = 2.8
TIME (MIN) = 360	DISCHARGE (CFS) = 2.6
TIME (MIN) = 370	DISCHARGE (CFS) = 0

DETENTION BASIN 100

5x4 (Top of Pipe Elev.: 3015.5')								
Elevation (ft)	Perforations		Top of Riser					Q (cfs) 50% Debris Blockage
	Delta Elevation (ft)	Q (cfs)	Delta Elevation (ft)	H/R	Csw	Q _{weir} (cfs)	Q _{orifice} (cfs)	
3010	0.0	0.00	-5.5	N/A	N/A	0.00	0.00	0.00
3010.5	0.5	0.67	-5.0	N/A	N/A	0.00	0.00	0.33
3011	1.0	0.95	-4.5	N/A	N/A	0.00	0.00	0.47
3011.5	1.5	1.16	-4.0	N/A	N/A	0.00	0.00	0.58
3012	2.0	2.01	-3.5	N/A	N/A	0.00	0.00	1.00
3012.5	2.5	2.44	-3.0	N/A	N/A	0.00	0.00	1.22
3013	3.0	3.46	-2.5	N/A	N/A	0.00	0.00	1.73
3013.5	3.5	4.05	-2.0	N/A	N/A	0.00	0.00	2.03
3014	4.0	4.54	-1.5	N/A	N/A	0.00	0.00	2.27
3014.5	4.5	4.98	-1.0	N/A	N/A	0.00	0.00	2.49
3015	5.0	5.38	-0.5	N/A	N/A	0.00	0.00	2.69
3015.5	5.5	5.75	0.0	N/A	N/A	0.00	0.00	2.87
3016	6.0	6.09	0.5	N/A	3.31	21.04	68.09	13.57
3016.5	6.5	6.42	1.0	N/A	3.34	60.17	96.30	33.29
3017	7.0	6.72	1.5	N/A	3.38	111.74	117.94	59.23
3017.5	7.5	7.02	2.0	N/A	3.42	173.89	136.19	71.60
3018	8.0	7.30	2.5	N/A	3.45	245.60	152.26	79.78

MASTER DESIGN STORM SUMMARY

Hydrograph Queue Only Network

MASTER NETWORK SUMMARY

SCS Unit Hydrograph Method

Hydrograph File Import Option Used For 1 node(s)

(*Node=Outfall; +Node=Diversion;)

(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
100-YEAR	HYG	100	3.487		4.1667	71.27		
DET.BASIN 100IN	POND	100	3.487		4.1700	70.07		
DET.BASIN 100OUT	POND	100	3.221	R	4.3100	18.20	3016.12	2.340
*OUT 20	JCT	100	3.221	R	4.3100	18.20		

Culvert Calculator Report

Basin 100

Solve For: Headwater Elevation

3006.15 < 3016.12'
OK✓

Culvert Summary			
Allowable HW Elevation	3,016.12 ft	Headwater Depth/Height	0.72
Computed Headwater Elev.	3,006.15 ft	Discharge	18.20 cfs
Inlet Control HW Elev.	3,005.95 ft	Tailwater Elevation	3,004.31 ft
Outlet Control HW Elev.	3,006.15 ft	Control Type	Entrance Control

Grades			
Upstream Invert	3,004.00 ft	Downstream Invert	3,002.81 ft
Length	237.84 ft	Constructed Slope	0.005003 ft/ft

Hydraulic Profile			
Profile	CompositeS1S2	Depth, Downstream	1.50 ft
Slope Type	Steep	Normal Depth	1.24 ft
Flow Regime	N/A	Critical Depth	1.37 ft
Velocity Downstream	5.15 ft/s	Critical Slope	0.003505 ft/ft

Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	Concrete	Span	3.00 ft
Section Size	36 inch	Rise	3.00 ft
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	3,006.15 ft	Upstream Velocity Head	0.52 ft
Ke	0.50	Entrance Loss	0.26 ft

Inlet Control Properties			
Inlet Control HW Elev.	3,005.95 ft	Flow Control	Unsubmerged
Inlet Type	Square edge w/headwall	Area Full	7.1 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

RUN DATE 11/10/2009
HYDROGRAPH FILE NAME Text1
TIME OF CONCENTRATION 12 MIN.
6 HOUR RAINFALL 3.6 INCHES
BASIN AREA 2.7 ACRES
RUNOFF COEFFICIENT 0.44
PEAK DISCHARGE 6.37 CFS

BASIN 200

TIME (MIN) = 0	DISCHARGE (CFS) = 0
TIME (MIN) = 12	DISCHARGE (CFS) = 0.3
TIME (MIN) = 24	DISCHARGE (CFS) = 0.3
TIME (MIN) = 36	DISCHARGE (CFS) = 0.3
TIME (MIN) = 48	DISCHARGE (CFS) = 0.3
TIME (MIN) = 60	DISCHARGE (CFS) = 0.3
TIME (MIN) = 72	DISCHARGE (CFS) = 0.3
TIME (MIN) = 84	DISCHARGE (CFS) = 0.3
TIME (MIN) = 96	DISCHARGE (CFS) = 0.3
TIME (MIN) = 108	DISCHARGE (CFS) = 0.4
TIME (MIN) = 120	DISCHARGE (CFS) = 0.4
TIME (MIN) = 132	DISCHARGE (CFS) = 0.4
TIME (MIN) = 144	DISCHARGE (CFS) = 0.4
TIME (MIN) = 156	DISCHARGE (CFS) = 0.5
TIME (MIN) = 168	DISCHARGE (CFS) = 0.5
TIME (MIN) = 180	DISCHARGE (CFS) = 0.6
TIME (MIN) = 192	DISCHARGE (CFS) = 0.6
TIME (MIN) = 204	DISCHARGE (CFS) = 0.8
TIME (MIN) = 216	DISCHARGE (CFS) = 0.9
TIME (MIN) = 228	DISCHARGE (CFS) = 1.3
TIME (MIN) = 240	DISCHARGE (CFS) = 1.8
TIME (MIN) = 252	DISCHARGE (CFS) = 6.37
TIME (MIN) = 264	DISCHARGE (CFS) = 1
TIME (MIN) = 276	DISCHARGE (CFS) = 0.7
TIME (MIN) = 288	DISCHARGE (CFS) = 0.5
TIME (MIN) = 300	DISCHARGE (CFS) = 0.4
TIME (MIN) = 312	DISCHARGE (CFS) = 0.4
TIME (MIN) = 324	DISCHARGE (CFS) = 0.3
TIME (MIN) = 336	DISCHARGE (CFS) = 0.3
TIME (MIN) = 348	DISCHARGE (CFS) = 0.3
TIME (MIN) = 360	DISCHARGE (CFS) = 0.3
TIME (MIN) = 372	DISCHARGE (CFS) = 0

DETENTION BASIN 200

24" Riser (Top of Pipe Elev.: 3048.40')

Elevation (ft)	Perforations		Top of Riser				Q (cfs) 50% Debris Blockage
	Delta Elevation (ft)	Q (cfs)	Delta Elevation (ft)	H/R	Q _{weir} (cfs)	Q _{orifice} (cfs)	
3042.9	0.0	0.00	-5.5	0.00	0.00	0.00	0.00
3043.4	0.5	0.07	-5.0	0.00	0.00	0.00	0.04
3043.9	1.0	0.11	-4.5	0.00	0.00	0.00	0.05
3044.4	1.5	0.13	-4.0	0.00	0.00	0.00	0.06
3044.9	2.0	0.15	-3.5	0.00	0.00	0.00	0.07
3045.4	2.5	0.17	-3.0	0.00	0.00	0.00	0.08
3045.9	3.0	0.85	-2.5	0.00	0.00	0.00	0.43
3046.4	3.5	1.14	-2.0	0.00	0.00	0.00	0.57
3046.9	4.0	1.37	-1.5	0.00	0.00	0.00	0.68
3047.4	4.5	1.56	-1.0	0.00	0.00	0.00	0.78
3047.9	5.0	1.73	-0.5	0.00	0.00	0.00	0.86
3048.4	5.5	1.88	0.0	0.00	0.00	0.00	0.94
3048.9	6.0	2.03	0.5	0.50	7.44	10.70	4.73
3049.4	6.5	2.16	1.0	1.00	12.75	15.13	7.46
3049.9	7.0	2.28	1.5	1.50	15.81	18.53	9.05
3050.4	7.5	2.40	2.0	2.00	17.77	21.39	10.09

MASTER DESIGN STORM SUMMARY

Hydrograph Queue Only Network

MASTER NETWORK SUMMARY

SCS Unit Hydrograph Method

Hydrograph File Import Option Used For 1 node(s)

(*Node=Outfall; +Node=Diversion;)

(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
100-YEAR	HYG	100	.357		4.2000	6.37		
DET.BASIN 200IN	POND	100	.357		4.2000	6.37		
DET.BASIN 200OUT	POND	100	.356		4.3800	.94	3048.39	.204
*OUT 20	JCT	100	.356		4.3800	.94		

Culvert Calculator Report Basin 200

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	3,048.39 ft	Headwater Depth/Height	0.37
Computed Headwater Elev.	3,038.53 ft	Discharge	0.94 cfs
Inlet Control HW Elev.	3,038.47 ft	Tailwater Elevation	3,038.34 ft
Outlet Control HW Elev.	3,038.53 ft	Control Type	Entrance Control

3038.53 < 3048.39'
OK ✓

Grades			
Upstream Invert	3,037.98 ft	Downstream Invert	3,037.59 ft
Length	59.82 ft	Constructed Slope	0.006520 ft/ft

Hydraulic Profile			
Profile	CompositeS1S2	Depth, Downstream	0.75 ft
Slope Type	Steep	Normal Depth	0.32 ft
Flow Regime	N/A	Critical Depth	0.36 ft
Velocity Downstream	1.06 ft/s	Critical Slope	0.004216 ft/ft

Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	Concrete	Span	1.50 ft
Section Size	18 inch	Rise	1.50 ft
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	3,038.53 ft	Upstream Velocity Head	0.13 ft
Ke	0.50	Entrance Loss	0.06 ft

Inlet Control Properties			
Inlet Control HW Elev.	3,038.47 ft	Flow Control	Unsubmerged
Inlet Type	Square edge w/headwall	Area Full	1.8 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

BASIN 300

RUN DATE 11/11/2009
HYDROGRAPH FILE NAME Text1
TIME OF CONCENTRATION 15 MIN.
6 HOUR RAINFALL 3.6 INCHES
BASIN AREA 30 ACRES
RUNOFF COEFFICIENT 0.48
PEAK DISCHARGE 68.41 CFS

TIME (MIN) = 0	DISCHARGE (CFS) = 0
TIME (MIN) = 15	DISCHARGE (CFS) = 3.1
TIME (MIN) = 30	DISCHARGE (CFS) = 3.2
TIME (MIN) = 45	DISCHARGE (CFS) = 3.4
TIME (MIN) = 60	DISCHARGE (CFS) = 3.5
TIME (MIN) = 75	DISCHARGE (CFS) = 3.8
TIME (MIN) = 90	DISCHARGE (CFS) = 3.9
TIME (MIN) = 105	DISCHARGE (CFS) = 4.3
TIME (MIN) = 120	DISCHARGE (CFS) = 4.5
TIME (MIN) = 135	DISCHARGE (CFS) = 4.9
TIME (MIN) = 150	DISCHARGE (CFS) = 5.2
TIME (MIN) = 165	DISCHARGE (CFS) = 6
TIME (MIN) = 180	DISCHARGE (CFS) = 6.5
TIME (MIN) = 195	DISCHARGE (CFS) = 8
TIME (MIN) = 210	DISCHARGE (CFS) = 9.1
TIME (MIN) = 225	DISCHARGE (CFS) = 13.3
TIME (MIN) = 240	DISCHARGE (CFS) = 17.6
TIME (MIN) = 255	DISCHARGE (CFS) = 68.41
TIME (MIN) = 270	DISCHARGE (CFS) = 10.7
TIME (MIN) = 285	DISCHARGE (CFS) = 7.1
TIME (MIN) = 300	DISCHARGE (CFS) = 5.6
TIME (MIN) = 315	DISCHARGE (CFS) = 4.7
TIME (MIN) = 330	DISCHARGE (CFS) = 4.1
TIME (MIN) = 345	DISCHARGE (CFS) = 3.6
TIME (MIN) = 360	DISCHARGE (CFS) = 3.3
TIME (MIN) = 375	DISCHARGE (CFS) = 0

DETENTION BASIN 300

5'x4' (Top of Pipe Elev.: 2964.5')								
Elevation (ft)	Perforations		Top of Riser				Q (cfs) 50% Debris Blockage	
	Delta Elevation (ft)	Q (cfs)	Delta Elevation (ft)	H/R	Cscw	Q _{weir} (cfs)	Q _{orifice} (cfs)	
2959	0.0	0.00	-5.5	N/A	N/A	0.00	0.00	0.00
2959.5	0.5	0.67	-5.0	N/A	N/A	0.00	0.00	0.33
2960	1.0	0.95	-4.5	N/A	N/A	0.00	0.00	0.47
2960.5	1.5	1.16	-4.0	N/A	N/A	0.00	0.00	0.58
2961	2.0	2.01	-3.5	N/A	N/A	0.00	0.00	1.00
2961.5	2.5	2.44	-3.0	N/A	N/A	0.00	0.00	1.22
2962	3.0	3.46	-2.5	N/A	N/A	0.00	0.00	1.73
2962.5	3.5	4.05	-2.0	N/A	N/A	0.00	0.00	2.03
2963	4.0	4.54	-1.5	N/A	N/A	0.00	0.00	2.27
2963.5	4.5	4.98	-1.0	N/A	N/A	0.00	0.00	2.49
2964	5.0	5.38	-0.5	N/A	N/A	0.00	0.00	2.69
2964.5	5.5	5.75	0.0	N/A	3.27	0.00	0.00	2.87
2965	6.0	6.09	0.5	N/A	3.31	21.04	68.09	13.57
2965.5	6.5	6.42	1.0	N/A	3.34	60.17	96.30	33.29
2966	7.0	6.72	1.5	N/A	3.38	111.74	117.94	59.23
2966.5	7.5	7.02	2.0	N/A	3.42	173.89	136.19	71.60
2967	8.0	7.30	2.5	N/A	3.45	245.60	152.26	79.78
2967.5	8.5	7.57	3.0	N/A	3.49	326.25	166.80	87.18
2968	9.0	7.84	3.5	N/A	3.52	415.41	180.16	94.00

MASTER DESIGN STORM SUMMARY

Hydrograph Queue Only Network

MASTER NETWORK SUMMARY

SCS Unit Hydrograph Method

Hydrograph File Import Option Used For 1 node(s)

(*Node=Outfall; +Node=Diversion;)

(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
100-YEAR	HYG	100	4.294		4.2500	68.41		
DET.BASIN 300IN	POND	100	4.294		4.2500	68.41		
DET.BASIN 300OUT	POND	100	4.293		4.3500	47.66	2965.78	2.226
*OUT 20	JCT	100	4.293		4.3500	47.66		

Culvert Calculator Report Basin 300

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	2,965.78 ft	Headwater Depth/Height	1.30
Computed Headwater Elev.	2,957.12 ft	Discharge	47.66 cfs
Inlet Control HW Elev.	2,956.58 ft	Tailwater Elevation	2,891.59 ft
Outlet Control HW Elev.	2,957.12 ft	Control Type	Entrance Control

2957.12 < 2965.78
OK ✓

Grades			
Upstream Invert	2,953.23 ft	Downstream Invert	2,890.09 ft
Length	199.60 ft	Constructed Slope	0.316333 ft/ft

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.69 ft
Slope Type	Steep	Normal Depth	0.69 ft
Flow Regime	Supercritical	Critical Depth	2.25 ft
Velocity Downstream	38.51 ft/s	Critical Slope	0.005239 ft/ft

Section			
Section Shape	Circular	Mannings Coefficient	0.012
Section Material	Concrete	Span	3.00 ft
Section Size	36 inch	Rise	3.00 ft
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	2,957.12 ft	Upstream Velocity Head	1.09 ft
Ke	0.50	Entrance Loss	0.55 ft

Inlet Control Properties			
Inlet Control HW Elev.	2,956.58 ft	Flow Control	Transition
Inlet Type	Square edge w/headwall	Area Full	7.1 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

EXISTING CONDITIONS BASIN 100

SDHM Final

File Edit View Help

Icons: [Globe] [3D View] [Bar Chart] [Document] [Tools] [LID]

Schematic

SCENARIOS

☒ Pre-Project

☐ Mitigated

Run Scenario

ELEMENTS

Icons: [Rain] [Sewer] [Storm] [Road] [Field] [Forest] [Shrub] [Urban] [Water] [Wetland] [Barriers] [Buildings] [Ponds] [Ditches] [Canals] [Pipes] [Structures]

Move Elements

Save x,y Load x,y

X: 0 Y: 12 #

Basin 100 Pre-Project

Subbasin Name: Basin 100

Flows To : Surface Interflow Groundwater

Area in Basin

☒ Show Only Selected

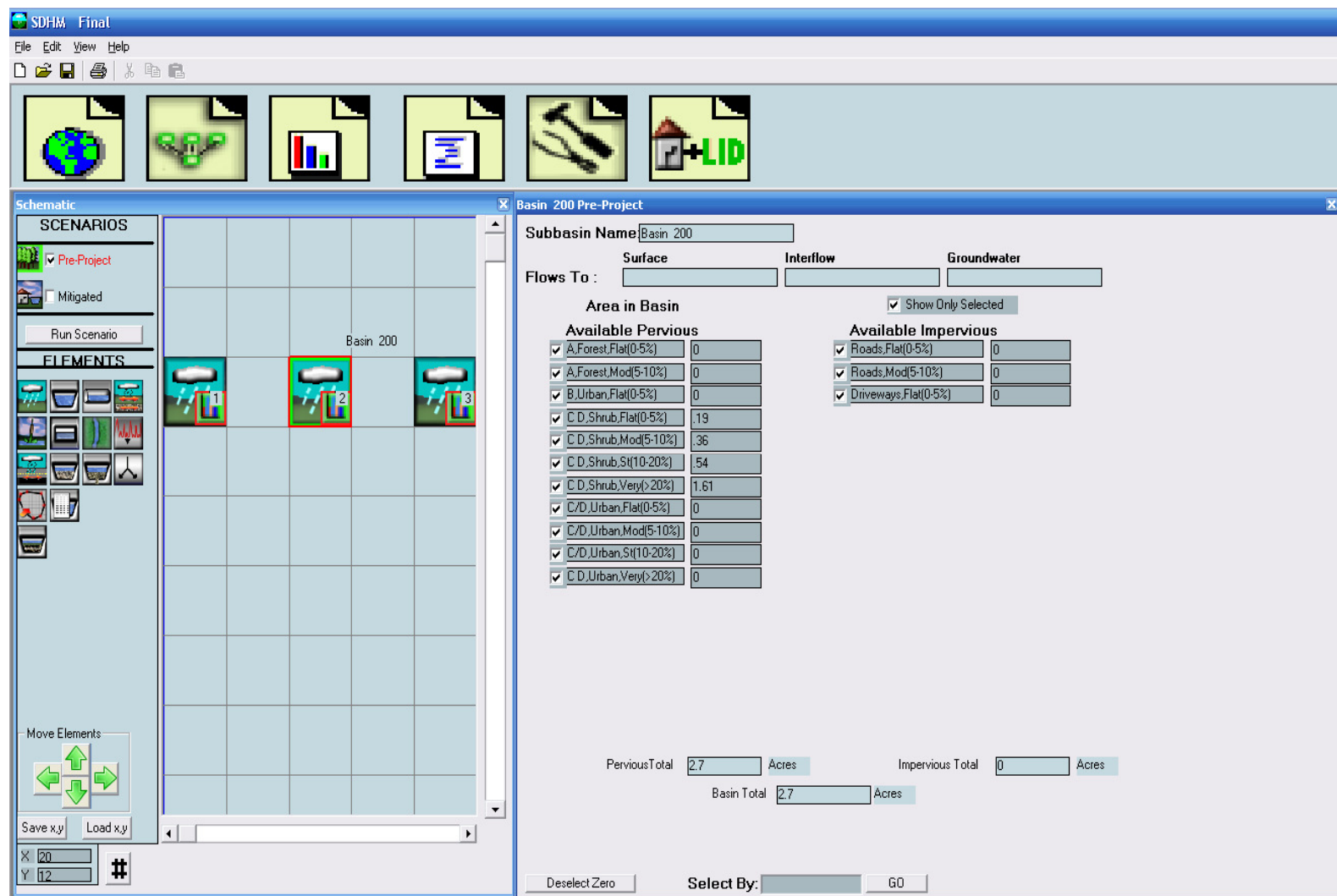
Available Pervious		Available Impervious	
☒ A.Forest.Flat(0-5%)	0	☒ Roads.Flat(0-5%)	.76
☒ A.Forest.Mod(5-10%)	0	☒ Roads.Mod(5-10%)	0
☒ B.Urban.Flat(0-5%)	0	☒ Driveways.Flat(0-5%)	0
☒ C.D.Shrub.Flat(0-5%)	.81		
☒ C.D.Shrub.Mod(5-10%)	.92		
☒ C.D.Shrub.St(10-20%)	7.95		
☒ C.D.Shrub.Very(>20%)	12.06		
☒ C/D.Urban.Flat(0-5%)	0		
☒ C/D.Urban.Mod(5-10%)	0		
☒ C/D.Urban.St(10-20%)	0		
☒ C.D.Urban.Very(>20%)	0		

Pervious Total 21.74 Acres Impervious Total 0.76 Acres

Basin Total 22.5 Acres

Deselect Zero Select By: GO

EXISTING CONDITIONS BASIN 200



EXISTING CONDITIONS BASIN 300

SDHM Final

File Edit View Zoom Help

Icons: [Globe] [Network] [Bar Chart] [Document] [Hand] [LID]

Schematic

SCENARIOS

☒ Pre-Project

☐ Mitigated

Run Scenario

ELEMENTS

Move Elements

Save x,y Load x,y

X: 40 Y: 12

Basin 300

Basin 300 Pre-Project

Subbasin Name: Basin 300

Flows To : Surface Interflow Groundwater

Area in Basin ☒ Show Only Selected

Available Pervious		Available Impervious	
☒ A.Forest.Flat(0-5%)	0	☒ Roads.Flat(0-5%)	.37
☒ A.Forest.Mod(5-10%)	0	☒ Roads.Mod(5-10%)	0
☒ B.Urban.Flat(0-5%)	0	☒ Driveways.Flat(0-5%)	0
☒ C.D.Shrub.Flat(0-5%)	1		
☒ C.D.Shrub.Mod(5-10%)	3.63		
☒ C.D.Shrub.St(10-20%)	9.3		
☒ C.D.Shrub.Very(>20%)	15.7		
☒ C/D.Urban.Flat(0-5%)	0		
☒ C/D.Urban.Mod(5-10%)	0		
☒ C/D.Urban.St(10-20%)	0		
☒ C.D.Urban.Very(>20%)	0		

Pervious Total 29.63 Acres Impervious Total 0.37 Acres

Basin Total 30 Acres

Deselect Zero Select By: GO

DETENTION BASIN 100

SDHM Final
File Edit View Help

Icons: [Globe] [Network] [Bar Chart] [Document] [Hammer] [House with LID]

Schematic

SCENARIOS

- ☐ Pre-Project
- ☒ Mitigated

Run Scenario

ELEMENTS

Icons: [Rain] [Pond] [Basin] [Structure] [Inlet] [Outlet] [Gate] [Infiltration] [LID] [Misc]

Move Elements: [Left] [Up] [Down] [Right]

Save x,y Load x,y

X: 10 Y: 12

Basin 100 Mitigated

Facility Name: Basin 100

Downstream Connections

Outlet 1	Outlet 2	Outlet 3
0	0	0

Facility Type: Irregular Pond

☒ Precipitation Applied to Facility

☒ Evaporation Applied to Facility

Facility Bottom Elevation (ft): 3010

Facility Dimensions

Effective Depth: 7.6

Open PondPad

Infiltration: NO

Outlet Structure

Riser Height (ft)	Riser Diameter(in)	Riser Type	Notch Type
5.5	68.75	Flat	

Orifice Number	Diameter (In)	Height (Ft)	QMax (cfs)
1	6	0	2.60656
2	6	1.5	2.33521
3	6	2.5	2.13524

Pond Volume at Riser Head (acre-ft): 2.076

Pond Increment: 0.10

Show Pond Table: Open Table

Use Tide Gate? NO

DETENTION BASIN 200

SDHM Final

File Edit View Help

Icons: [Globe] [Network] [Bar Chart] [Document] [Hammer] [House with LID]

Schematic

SCENARIOS

☐ Pre-Project

☒ Mitigated

Run Scenario

ELEMENTS

[Icons for various elements: rain, storage, infiltration, etc.]

Move Elements

Save x,y Load x,y

X: 40 Y: 0

Basin 200 Mitigated

Facility Name Basin 200

Downstream Connections

Facility Type Irregular Pond

☒ Precipitation Applied to Facility

☒ Evaporation Applied to Facility

Facility Bottom Elevation (ft) 3042.9

Facility Dimensions

Effective Depth 7.1

Open PondPad

Infiltration NO

Outlet Structure

Orifice Number	Diameter (In)	Height (Ft)	QMax (cfs)
1	2	0	0.27993
2	6	2.5	2.02787
3	0	0	0

Pond Volume at Riser Head (acre-ft) .197

Pond Increment 0.10

Show Pond Table Open Table

Use Tide Gate? NO

DETENTION BASIN 300

SDHM Final

File Edit View Zoom Help

Icons: [Globe] [Basin] [Bar Chart] [Document] [Hammer] [House+LID]

Schematic

SCENARIOS

☐ Pre-Project

☒ Mitigated

Run Scenario

ELEMENTS

[Icons for various elements: rain, pond, structure, etc.]

Move Elements: [Left Arrow] [Up Arrow] [Down Arrow] [Right Arrow]

Save x,y Load x,y

X: 40 Y: 12 #

Basin 300 Mitigated

Facility Name Basin 300

Downstream Connections

Facility Type Irregular Pond

☒ Precipitation Applied to Facility

☒ Evaporation Applied to Facility

Facility Bottom Elevation (ft) 2959

Facility Dimensions

Effective Depth 8.6

Open PondPad

Infiltration NO

Outlet Structure

Outlet	Riser Height (ft)	Riser Diameter (in)	Riser Type	Notch Type
Outlet 1	5.5	68.75	Flat	
Outlet 2	0			
Outlet 3	0			

Orifice Data Table

Orifice Number	Diameter (In)	Height (Ft)	QMax (cfs)
1	6	0	2.77275
2	6	1.5	2.51936
3	6	2.5	2.33521

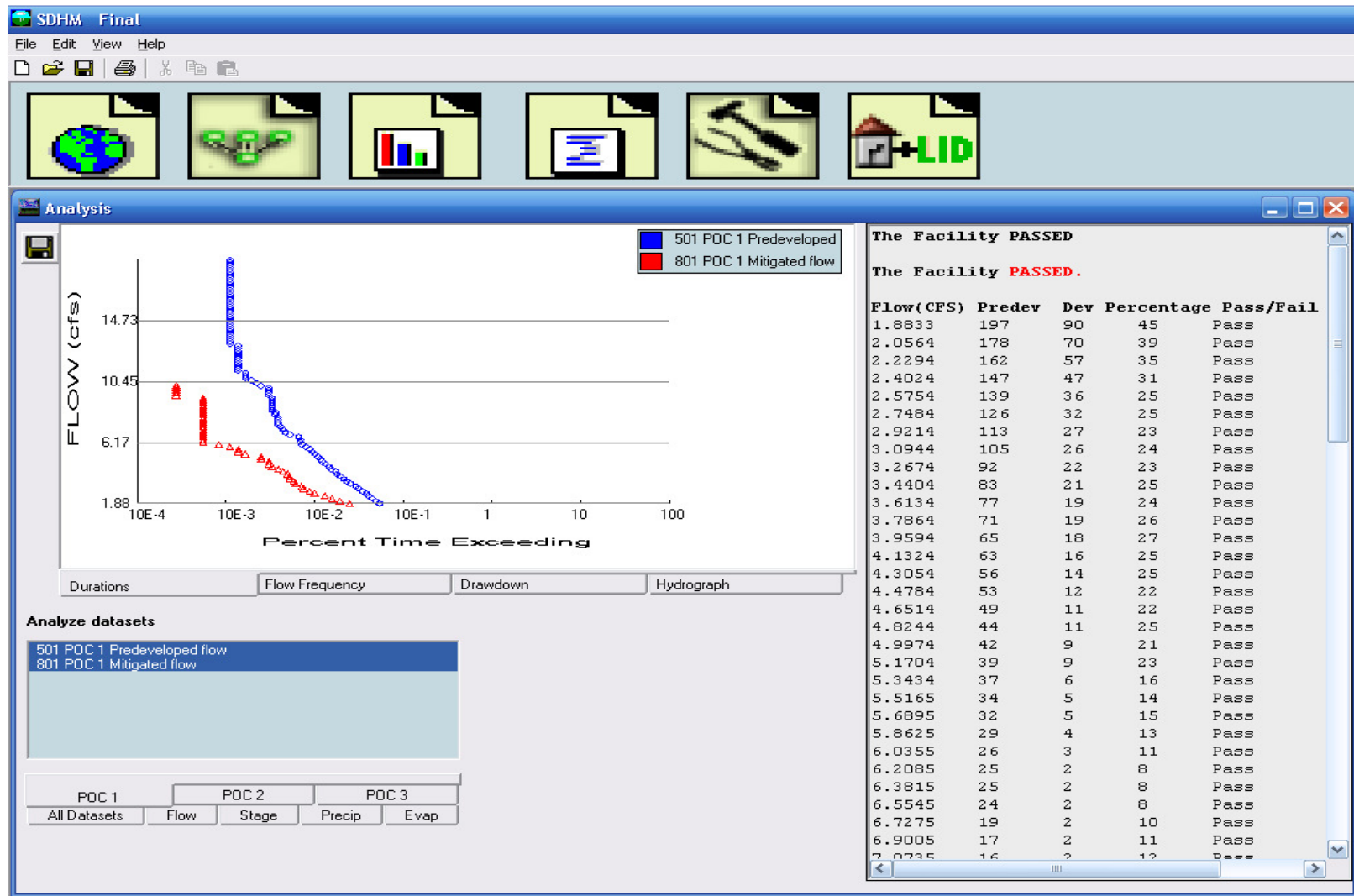
Pond Volume at Riser Head (acre-ft) 1.547

Pond Increment 0.10

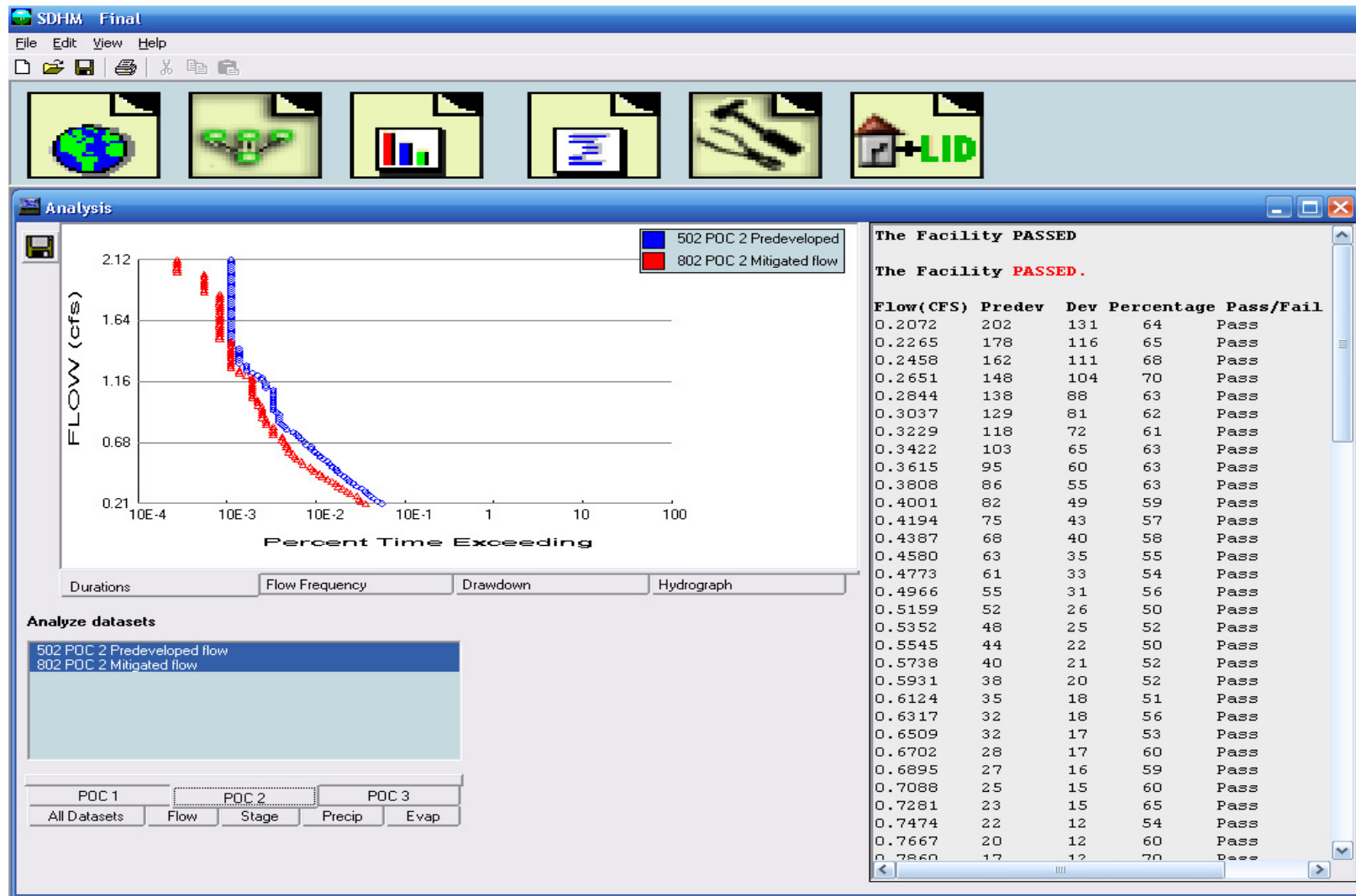
Show Pond Table Open Table

Use Tide Gate? NO

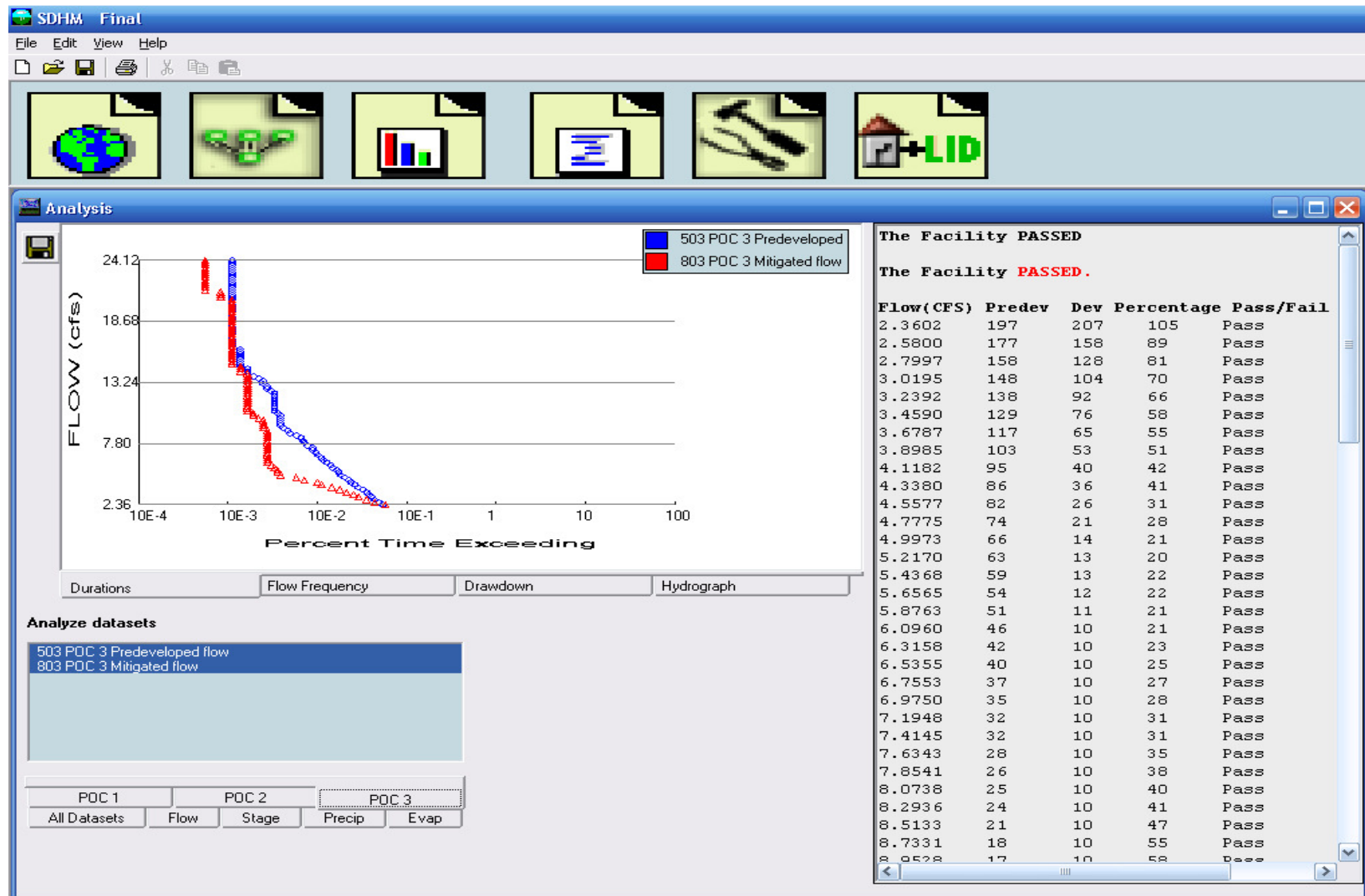
POC BASIN 100



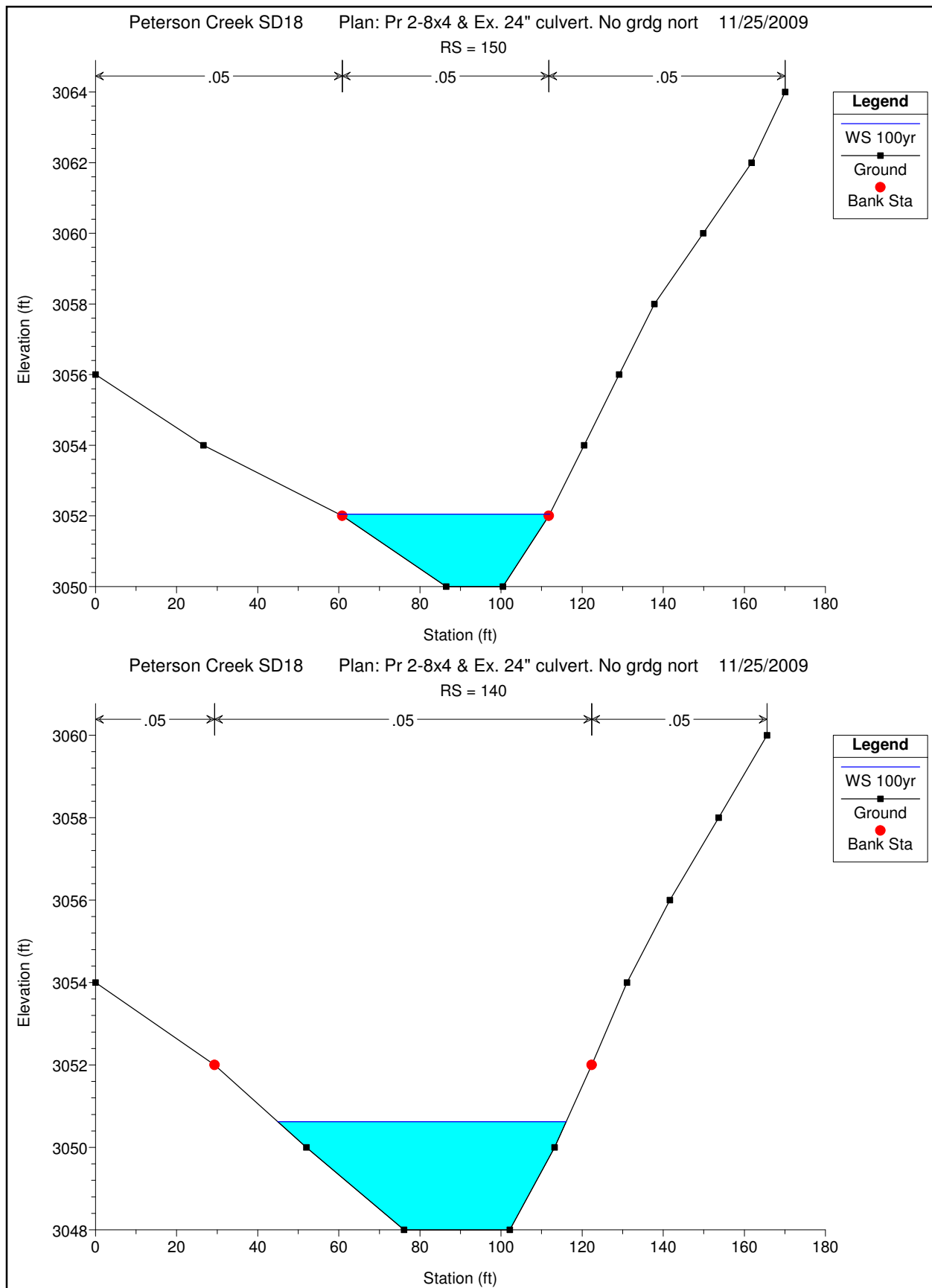
POC BASIN 200

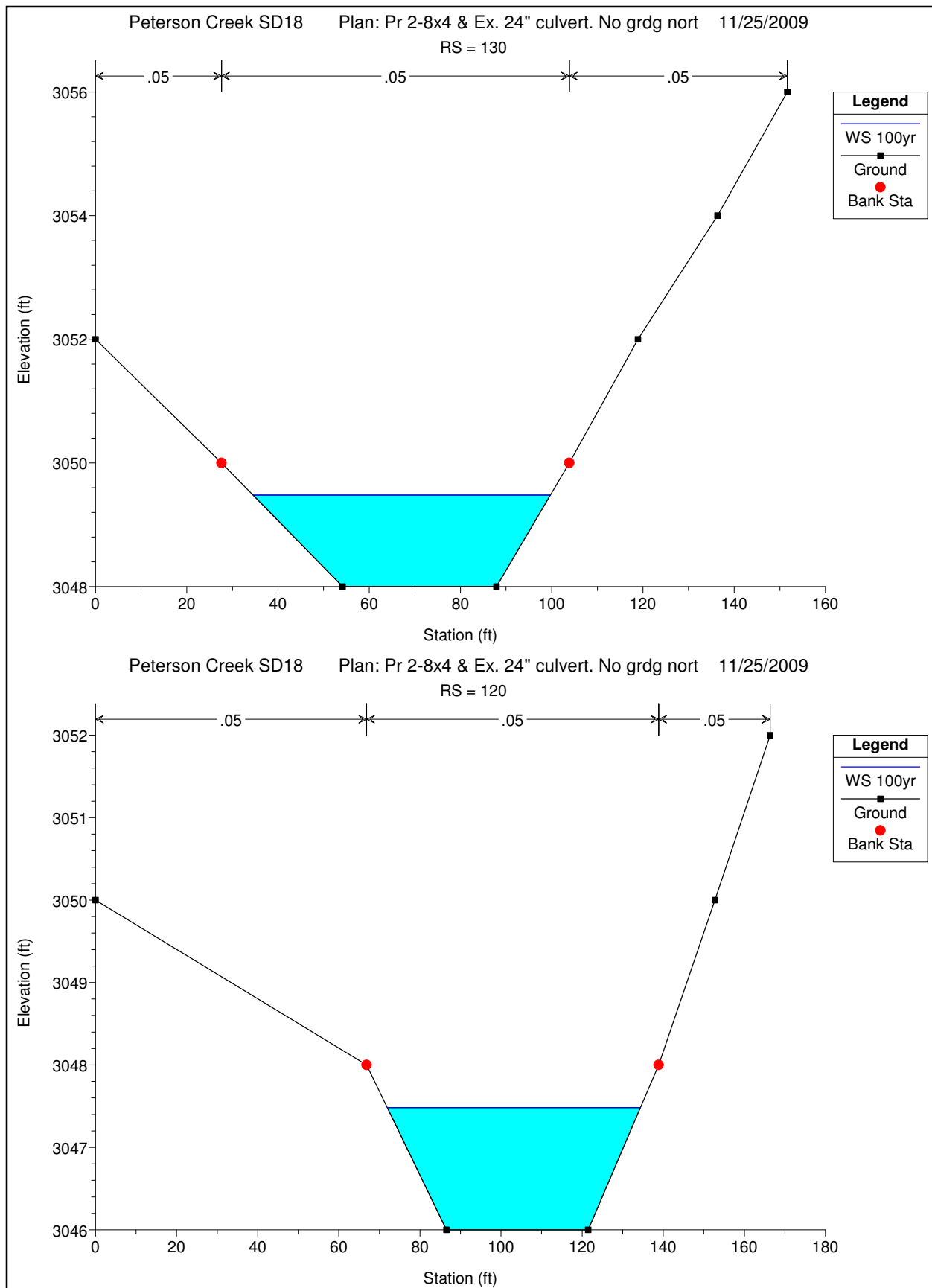


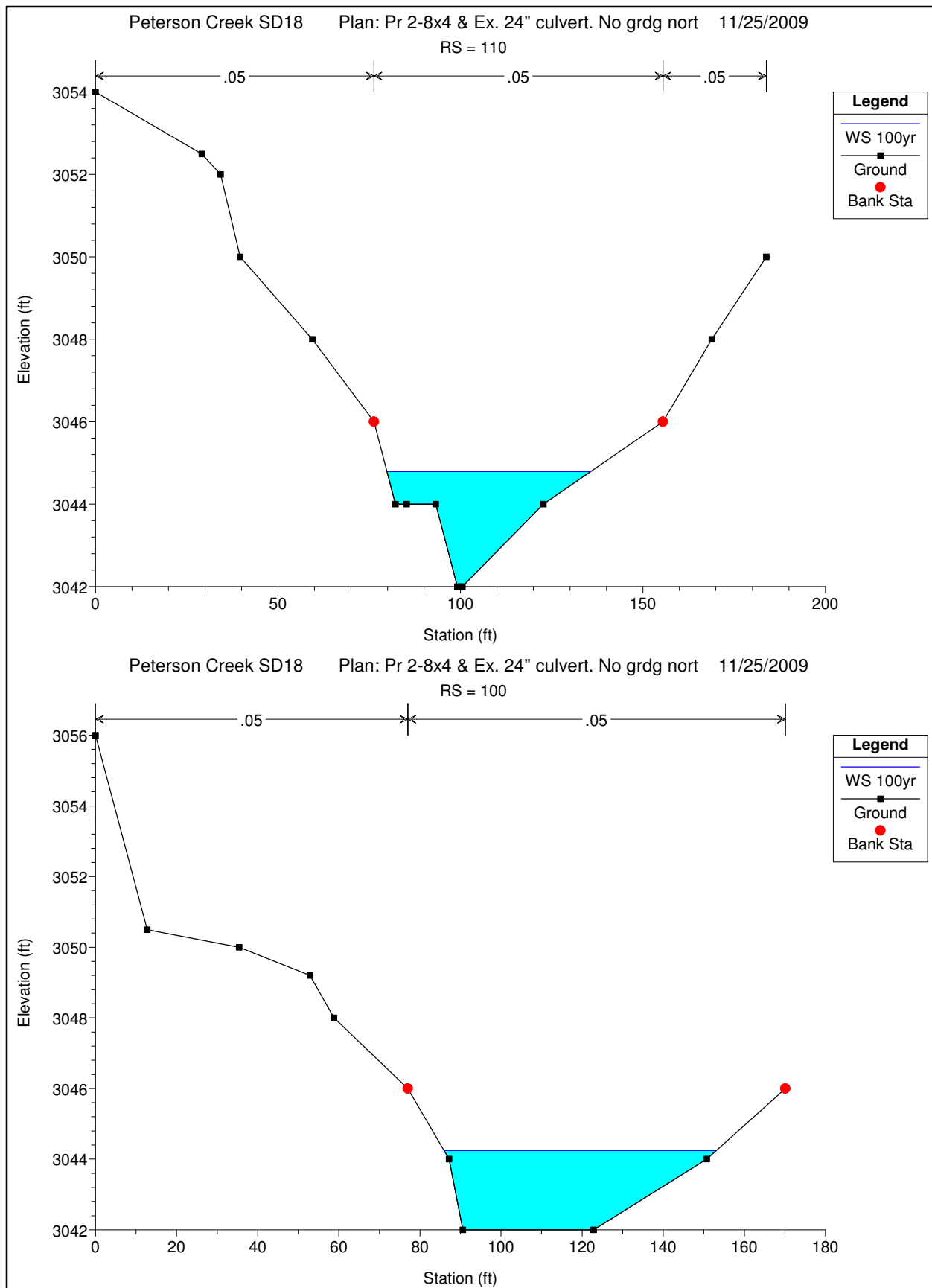
POC BASIN 300

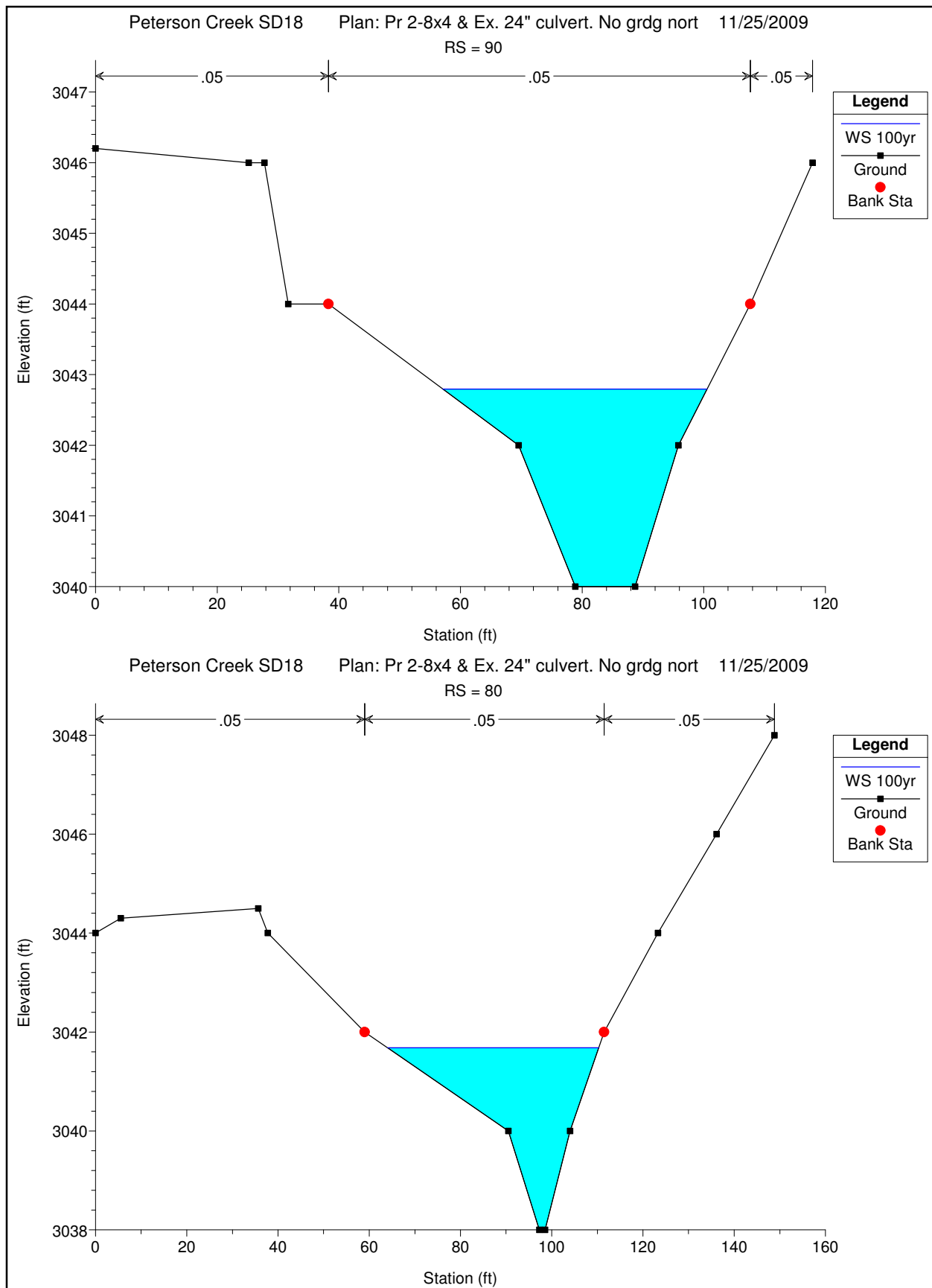


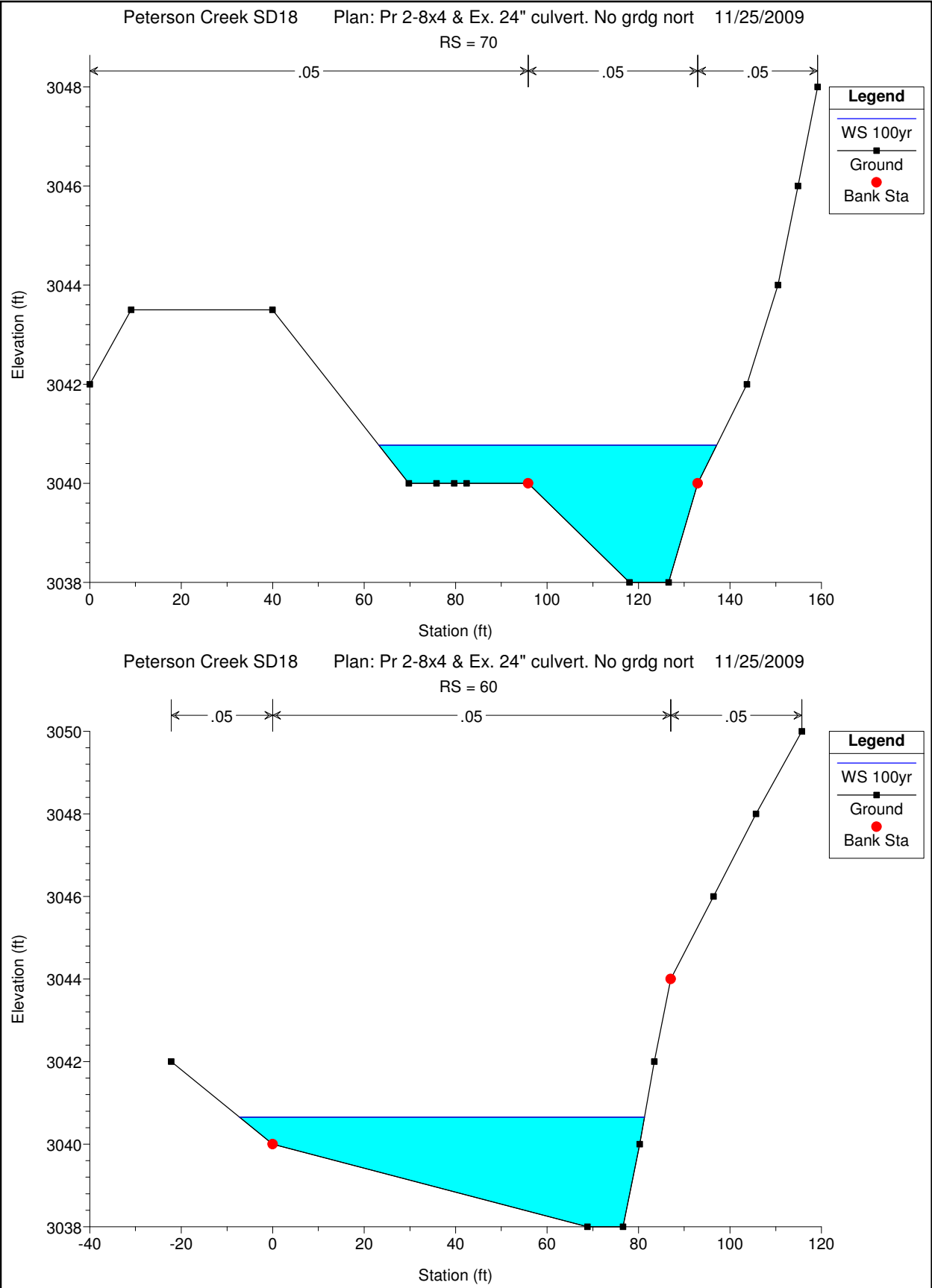
HEC-RAS CALCULATIONS

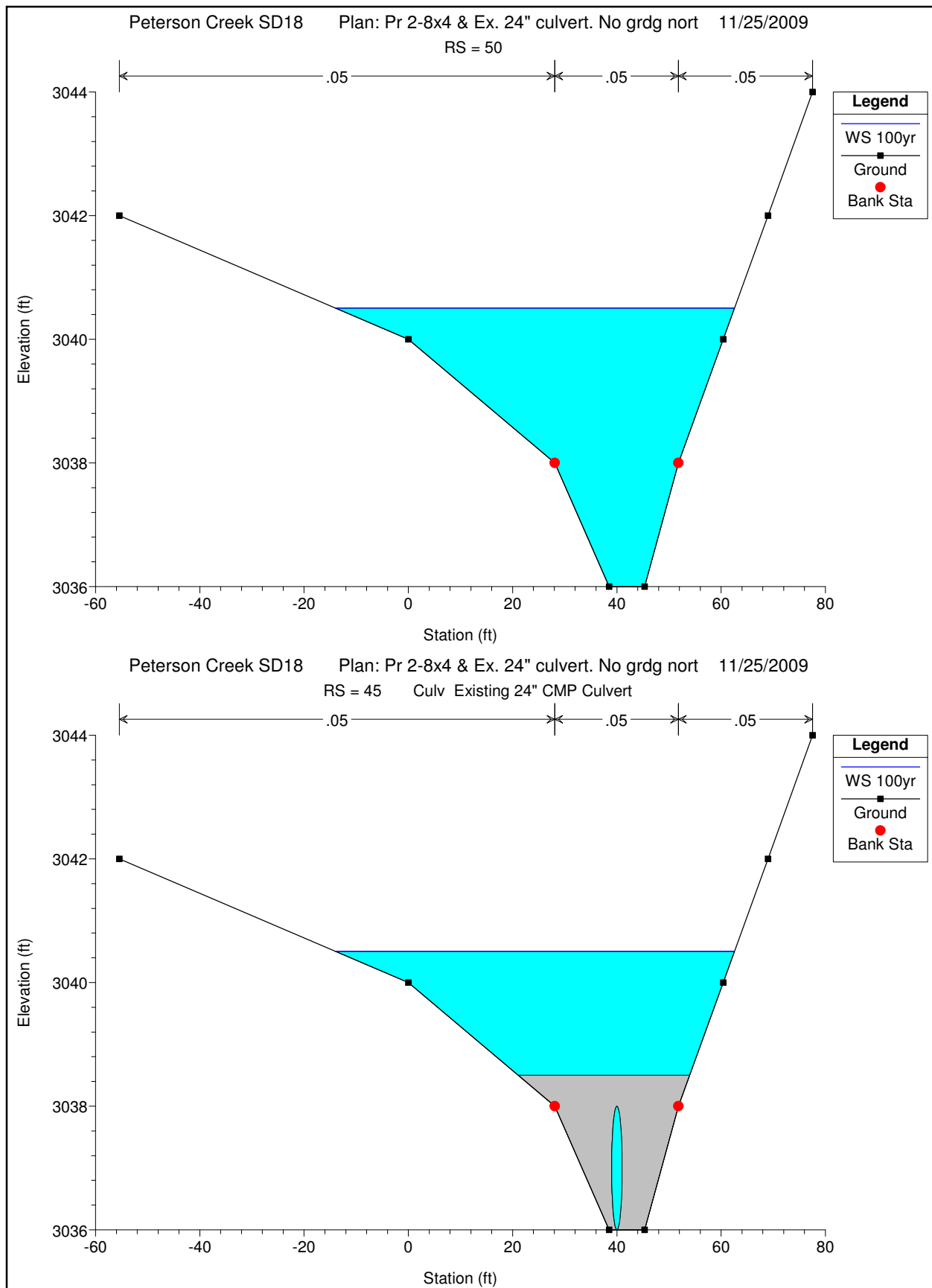


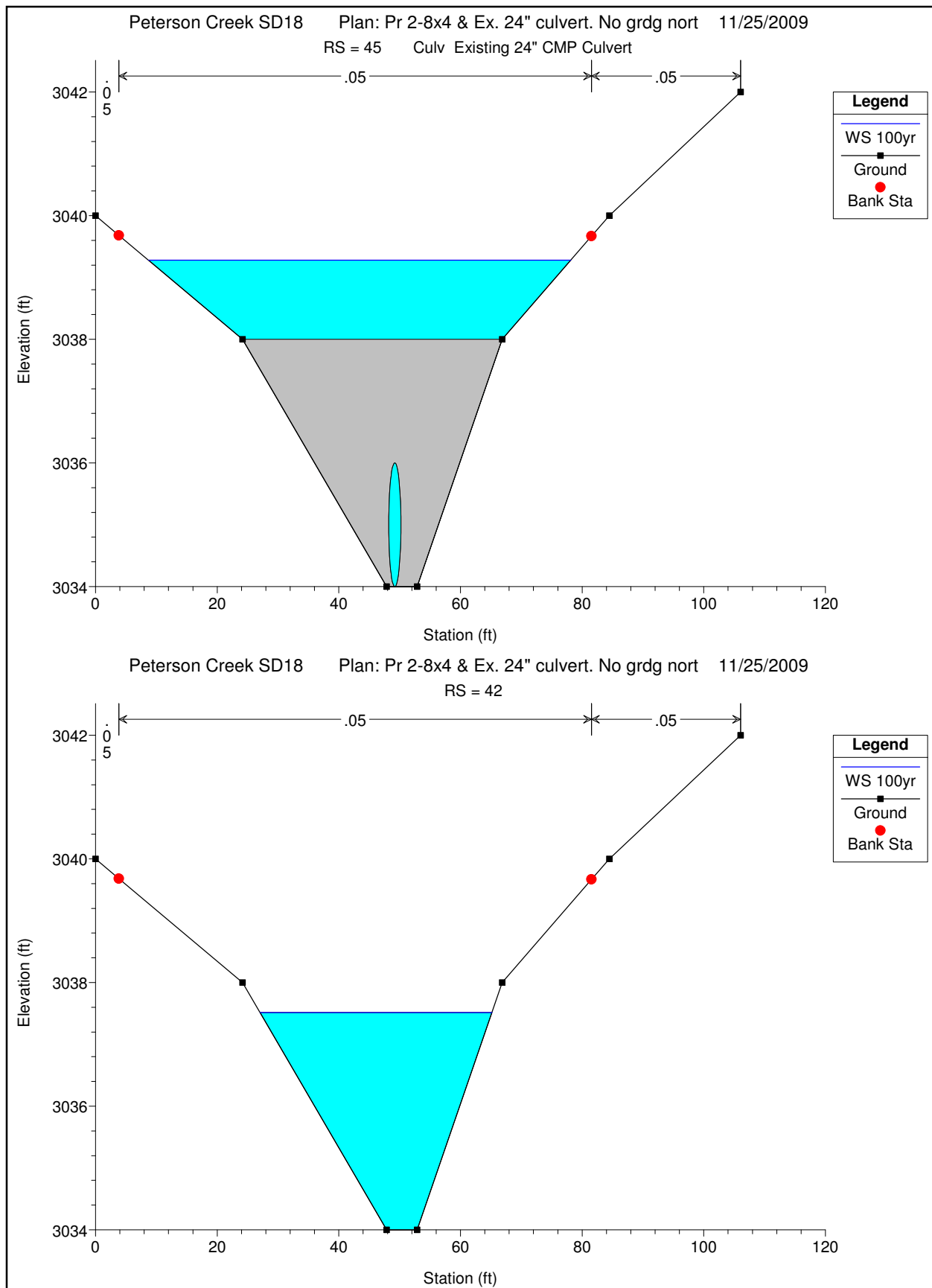


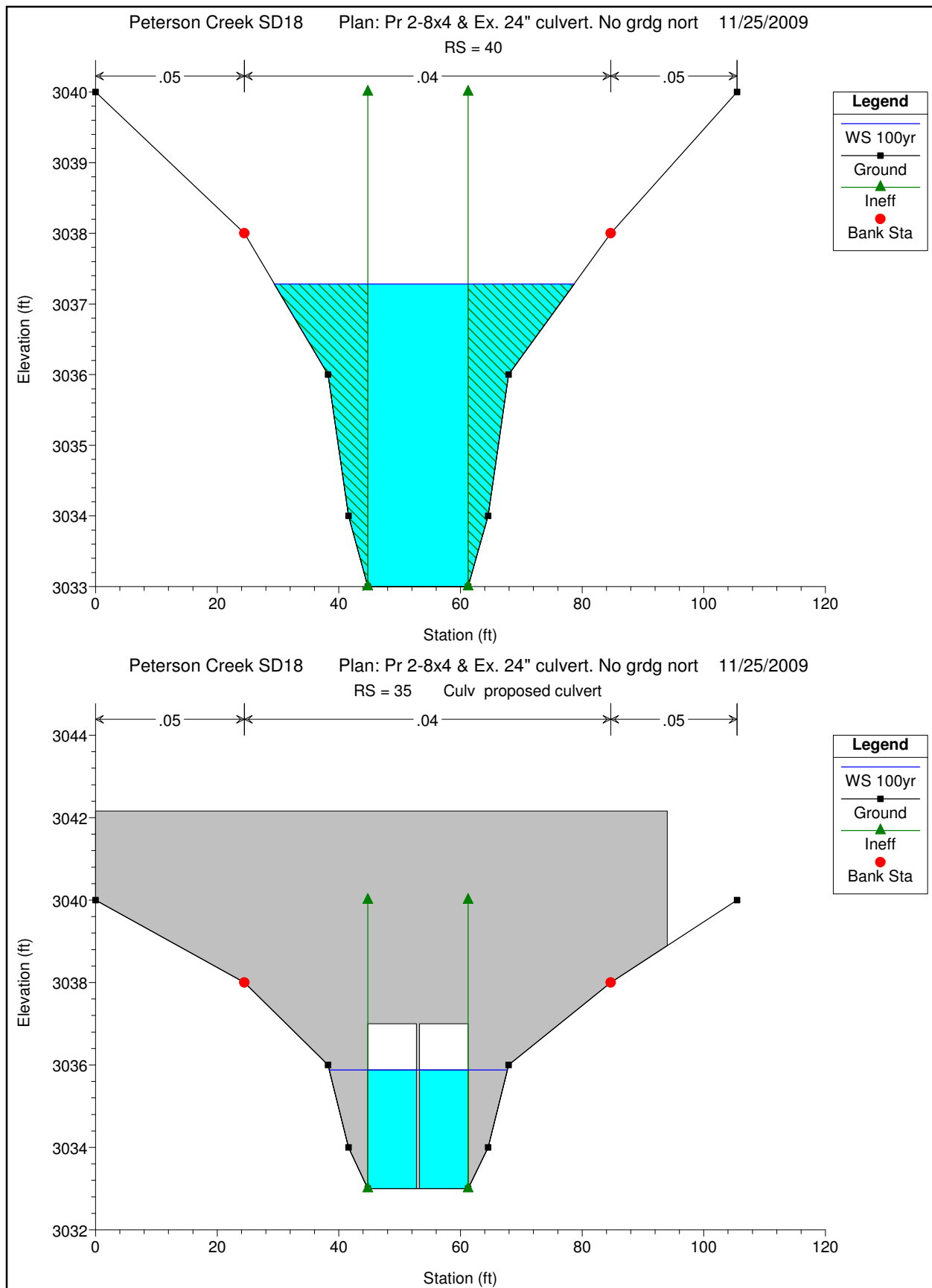


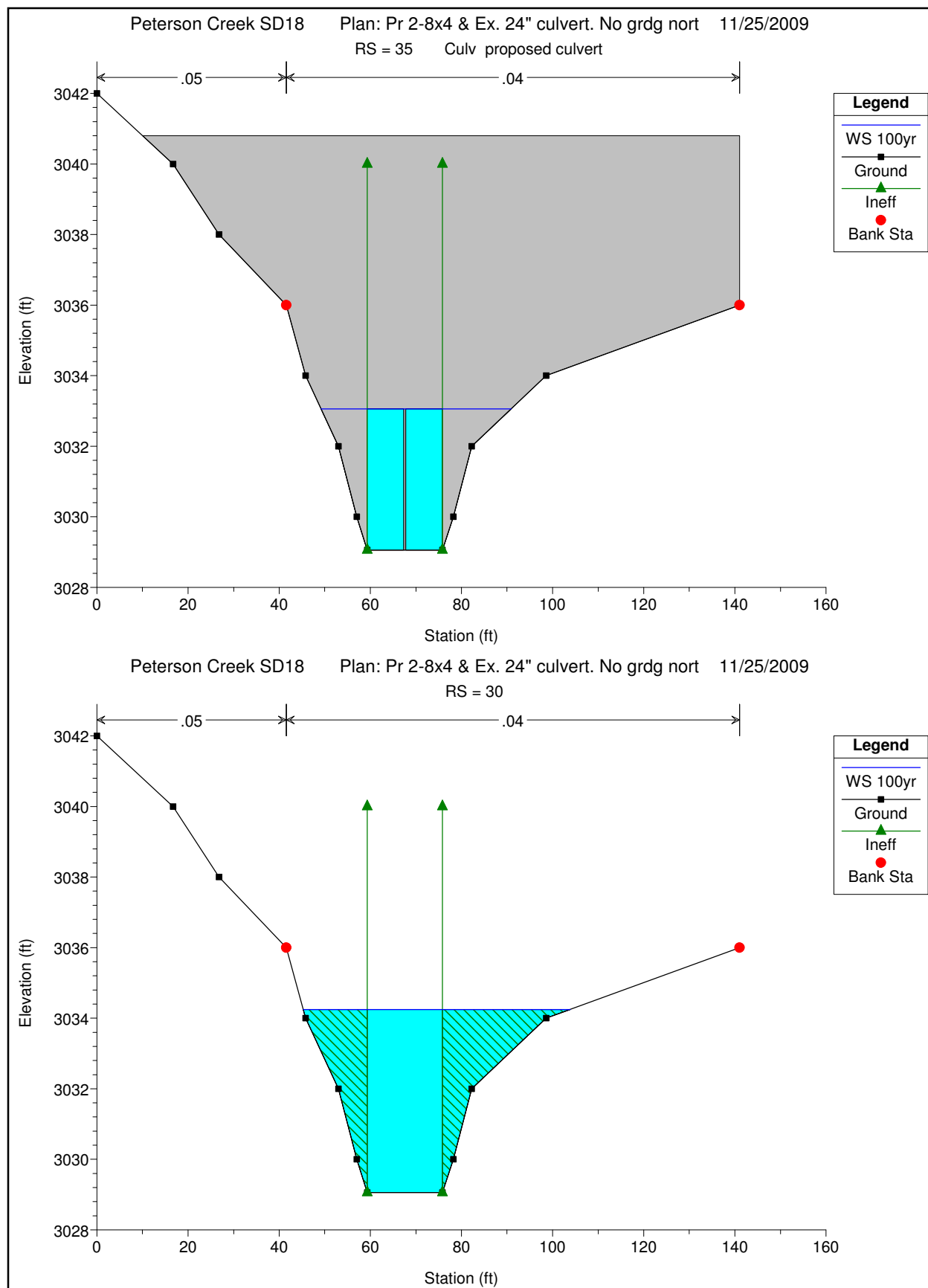


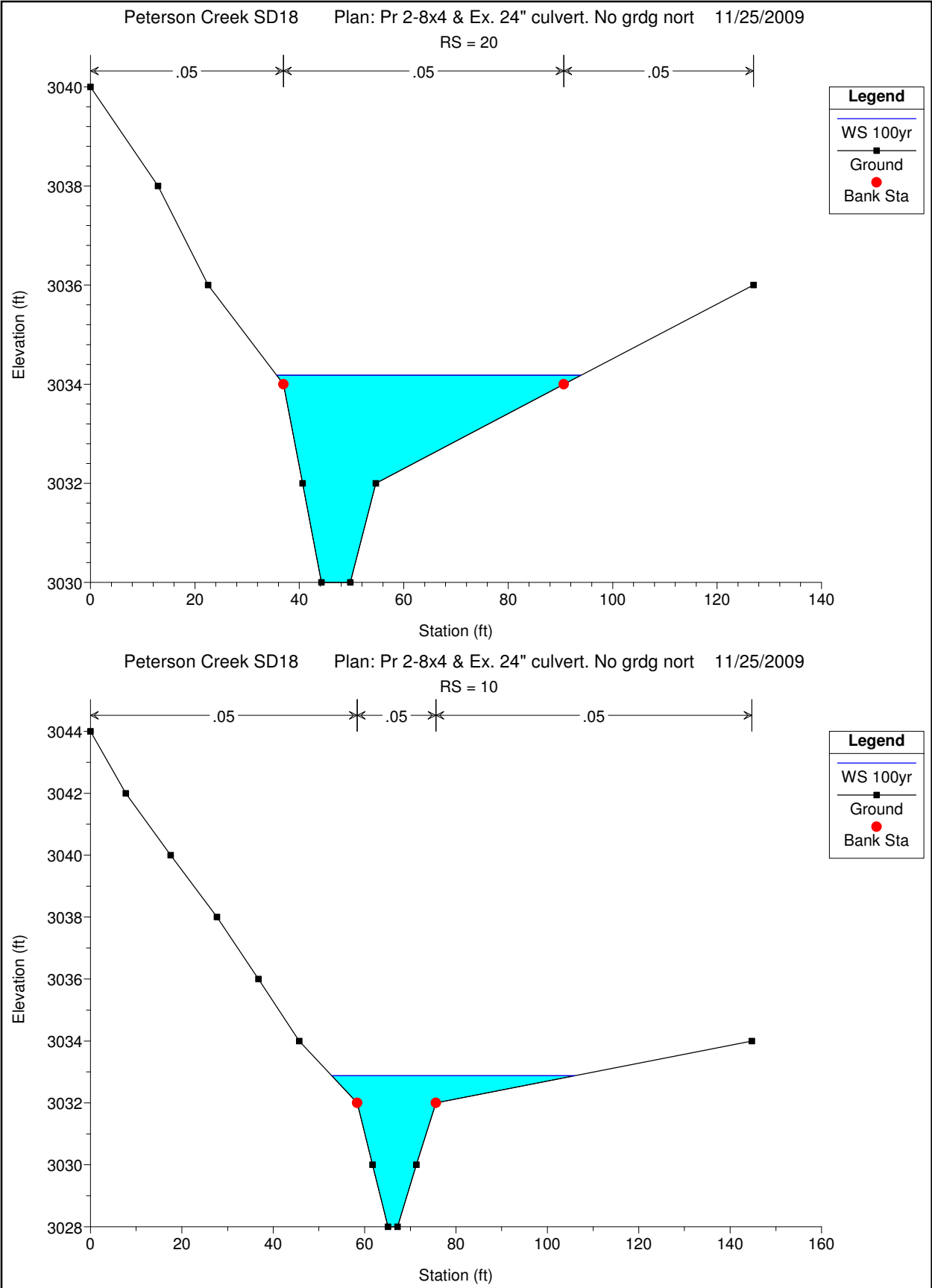






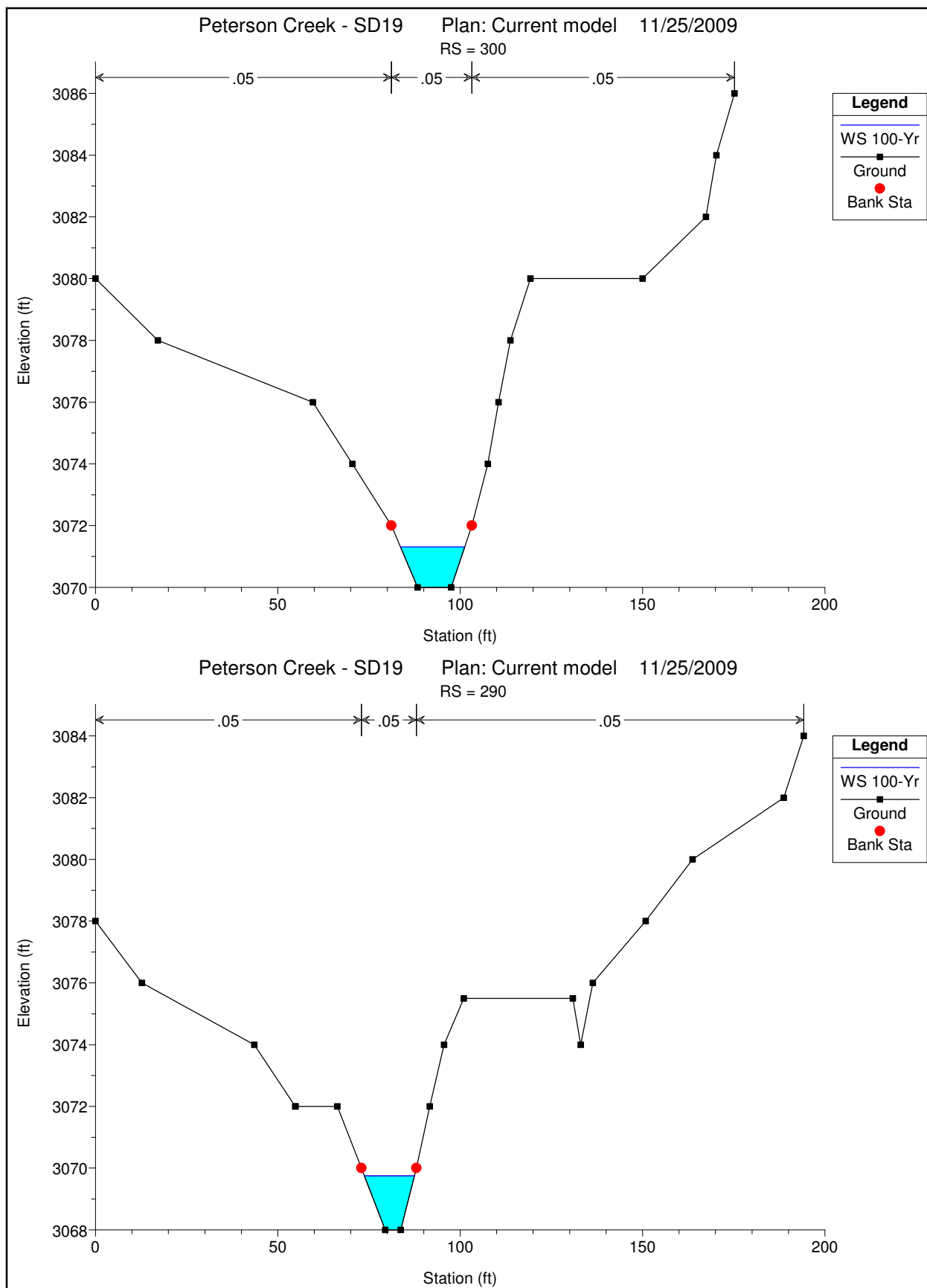


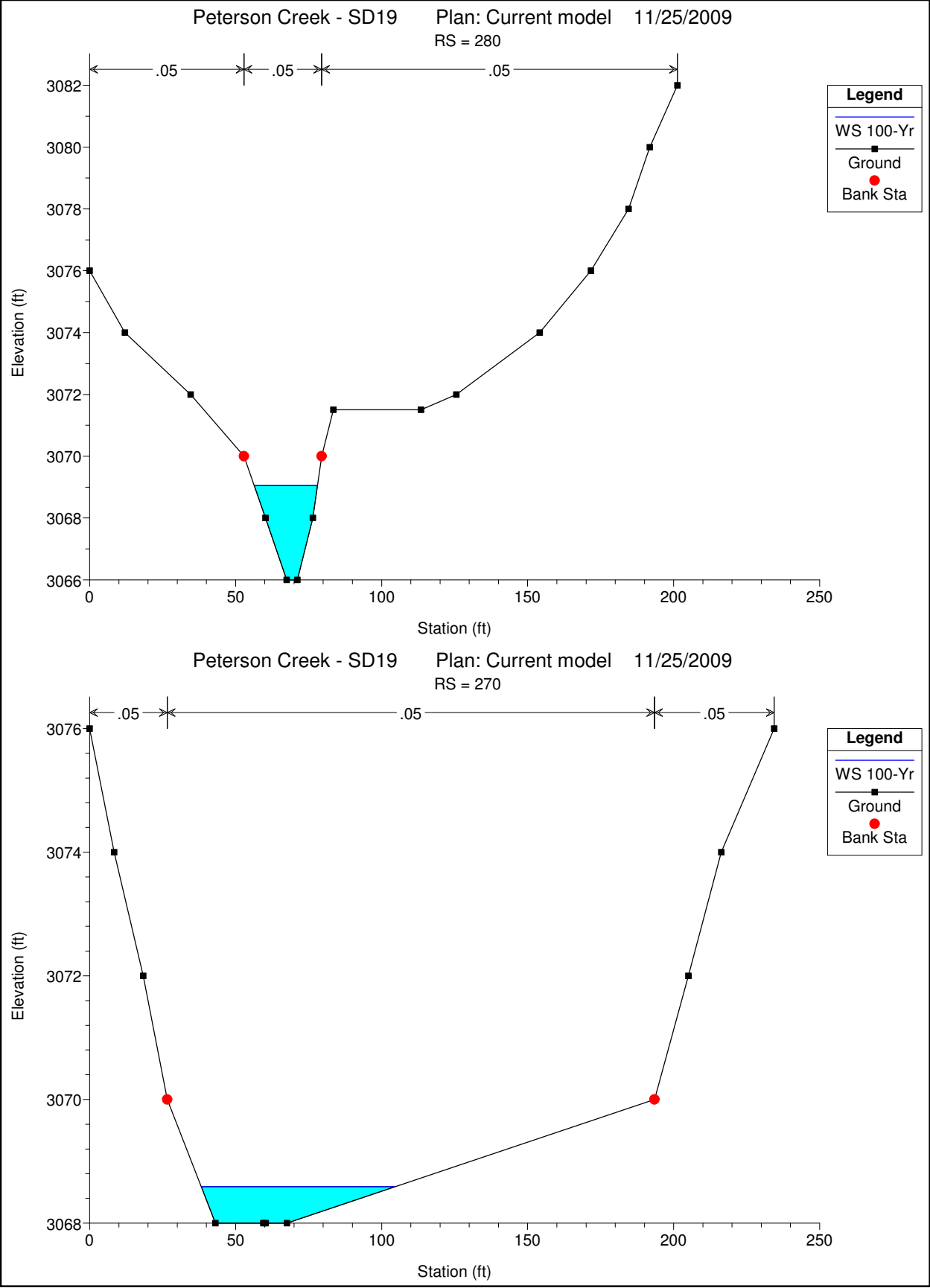


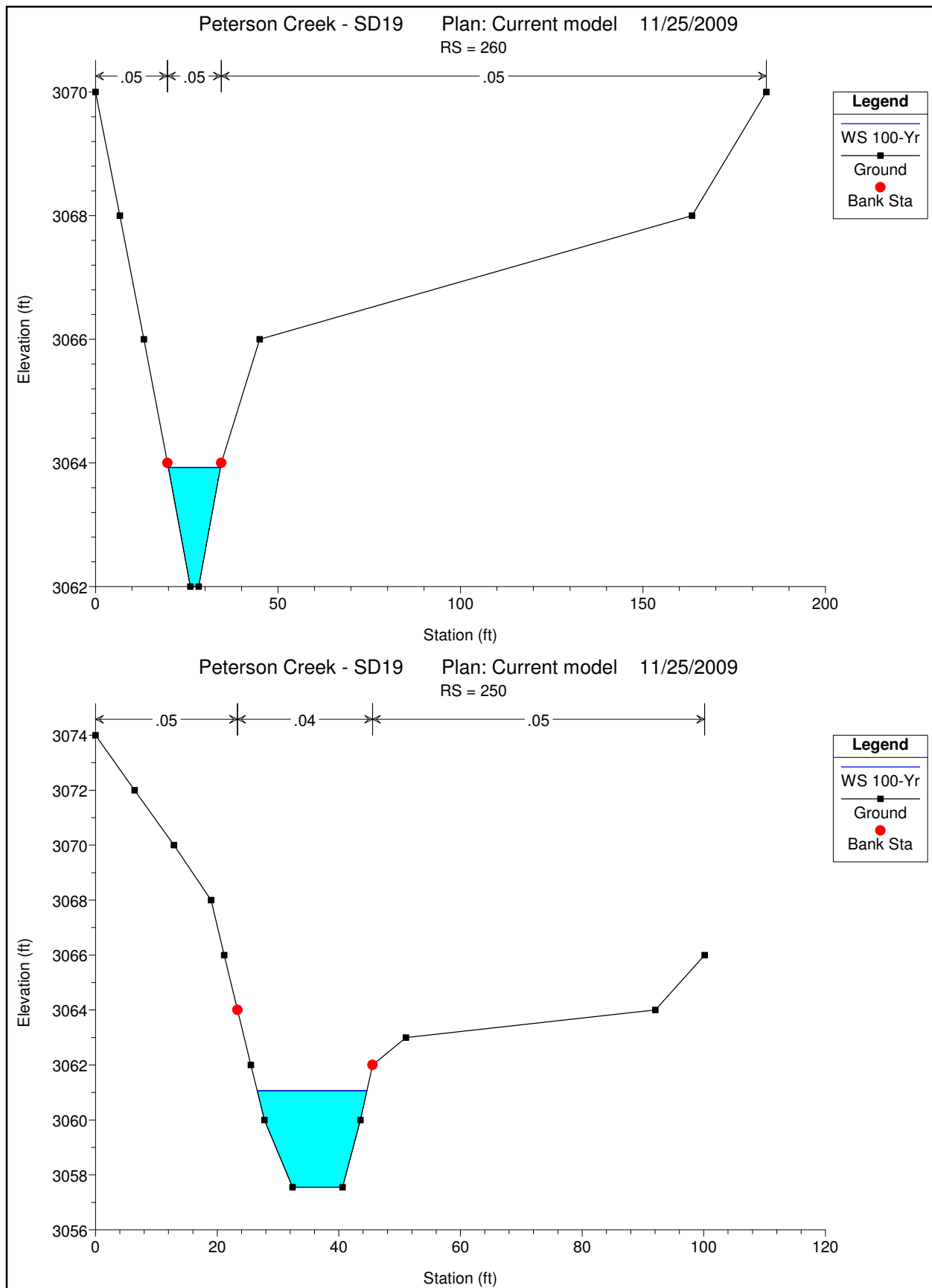


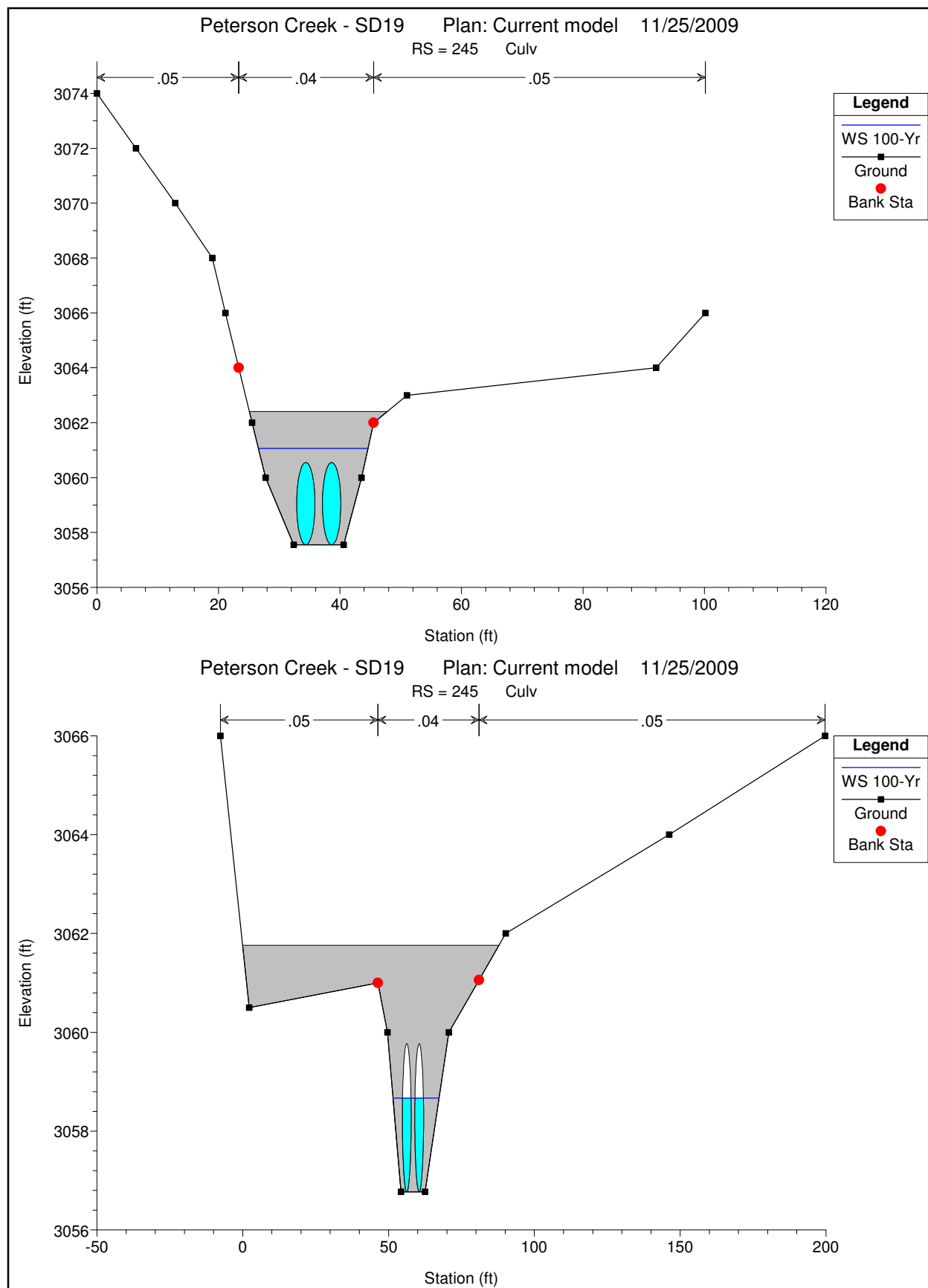
HEC-RAS Plan: 8x4 & 24" River: River #1 Reach: Reach #1 Profile: 100yr

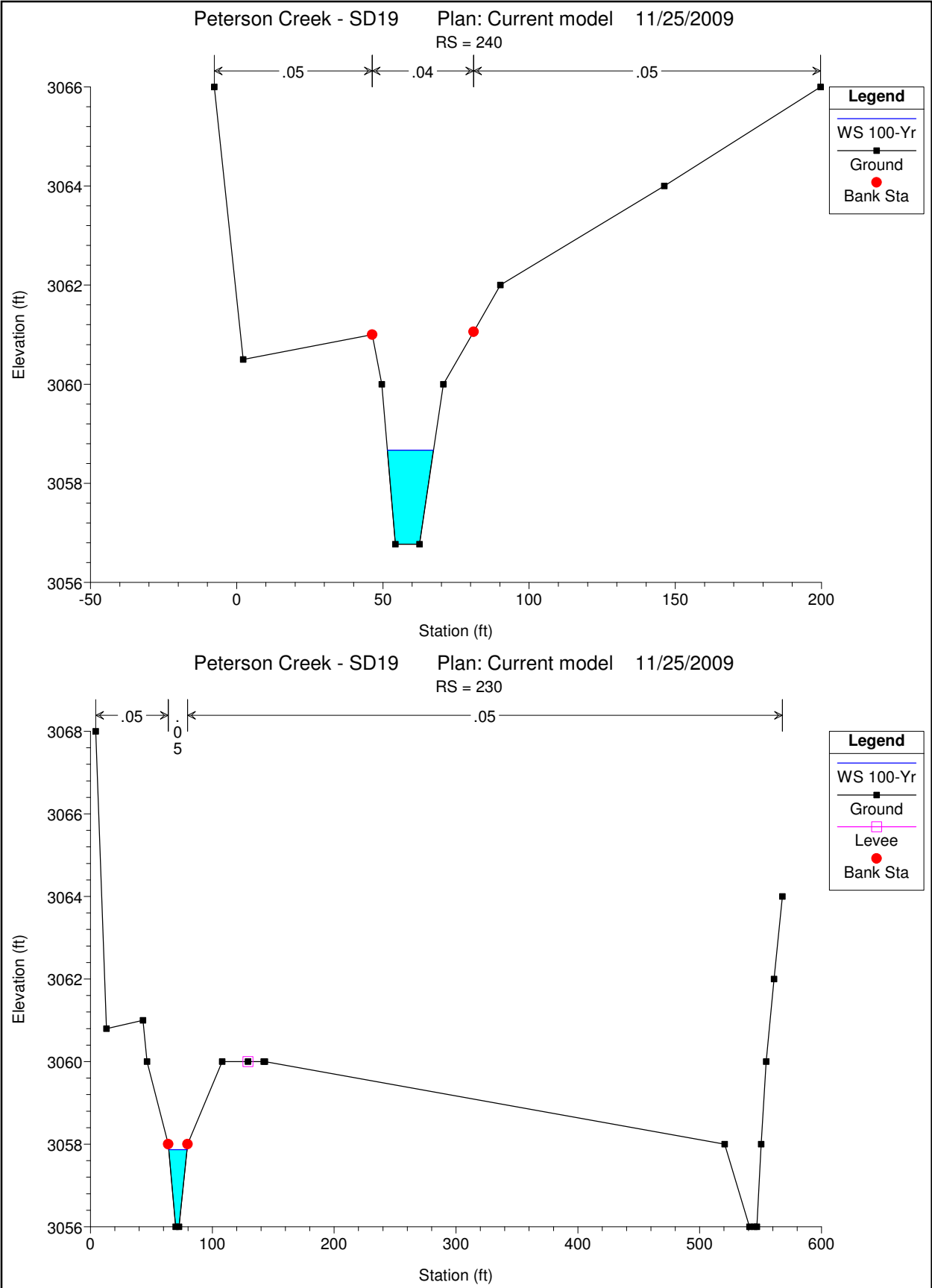
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach #1	150	100yr	443.80	3050.00	3052.05	3052.05	3052.72	0.033842	6.57	67.55	52.06	1.01
Reach #1	140	100yr	443.80	3048.00	3050.62		3050.81	0.006168	3.45	128.57	71.12	0.45
Reach #1	130	100yr	443.80	3048.00	3049.48	3049.48	3050.05	0.035909	6.07	73.10	65.16	1.01
Reach #1	120	100yr	443.80	3046.00	3047.48	3047.48	3048.07	0.035464	6.15	72.13	62.42	1.01
Reach #1	110	100yr	443.80	3042.00	3044.79	3044.79	3045.44	0.035642	6.42	69.10	55.84	1.02
Reach #1	100	100yr	443.80	3042.00	3044.25		3044.49	0.009114	3.96	112.00	67.20	0.54
Reach #1	90	100yr	443.80	3040.00	3042.80	3042.80	3043.55	0.033283	6.95	63.88	43.45	1.01
Reach #1	80	100yr	443.80	3038.00	3041.68	3041.68	3042.41	0.034643	6.84	64.84	46.20	1.02
Reach #1	70	100yr	443.80	3038.00	3040.77		3041.14	0.012163	5.17	98.36	73.89	0.64
Reach #1	60	100yr	443.80	3038.00	3040.65		3040.81	0.005422	3.14	143.09	88.53	0.42
Reach #1	50	100yr	443.80	3036.00	3040.51	3039.02	3040.69	0.002854	3.81	149.22	76.57	0.34
Reach #1	45		Culvert									
Reach #1	42	100yr	443.80	3034.00	3037.52		3038.05	0.015967	5.86	75.77	38.11	0.73
Reach #1	40	100yr	443.80	3033.00	3037.28	3035.84	3037.90	0.004108	6.28	70.67	49.31	0.53
Reach #1	35		Culvert									
Reach #1	30	100yr	443.80	3029.06	3034.25	3031.90	3034.67	0.002169	5.18	85.60	58.58	0.40
Reach #1	20	100yr	443.80	3030.00	3034.18		3034.51	0.011165	4.57	97.36	58.24	0.60
Reach #1	10	100yr	443.80	3028.00	3032.88	3032.88	3033.72	0.016704	7.62	69.48	53.19	0.76

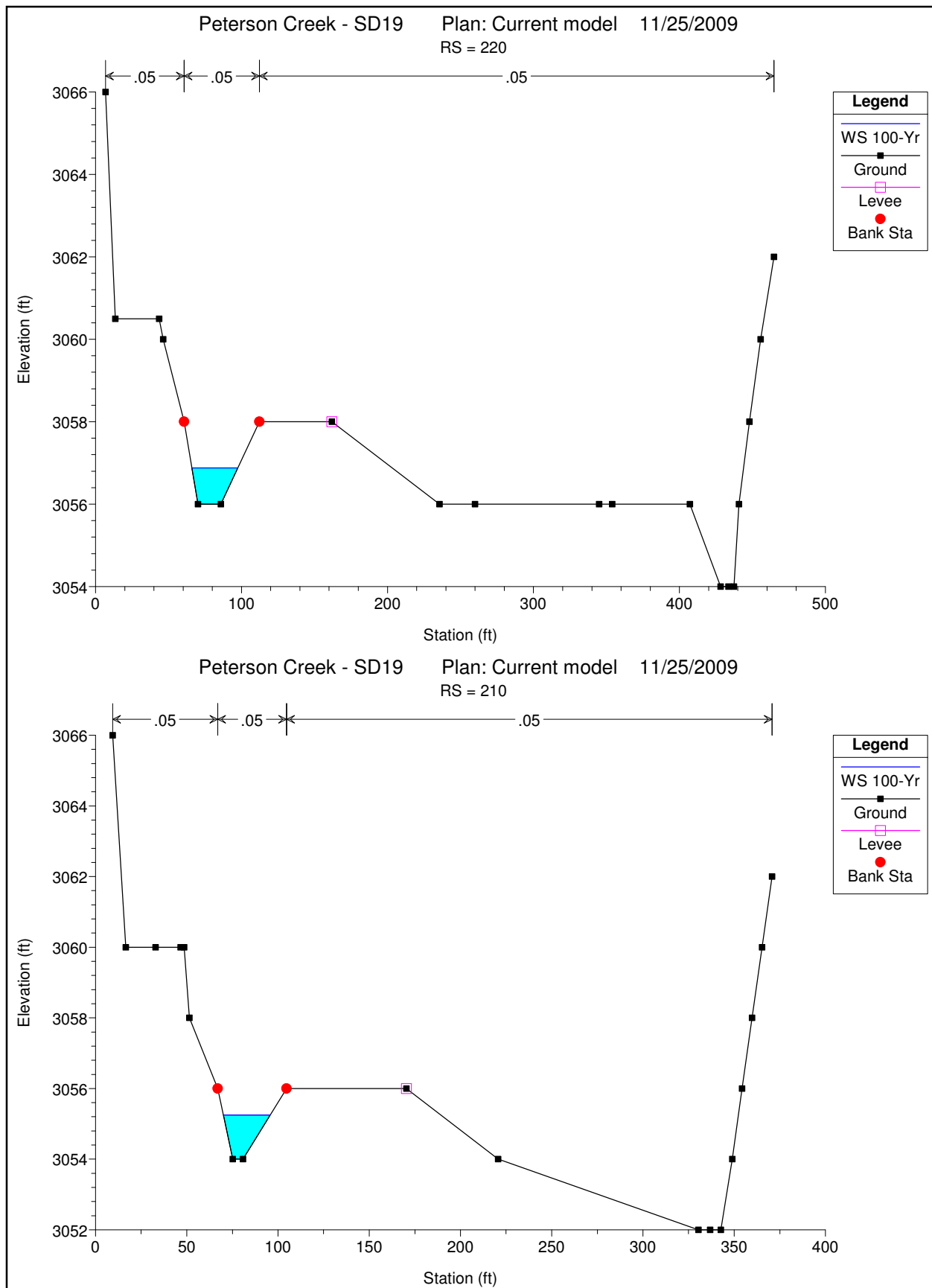


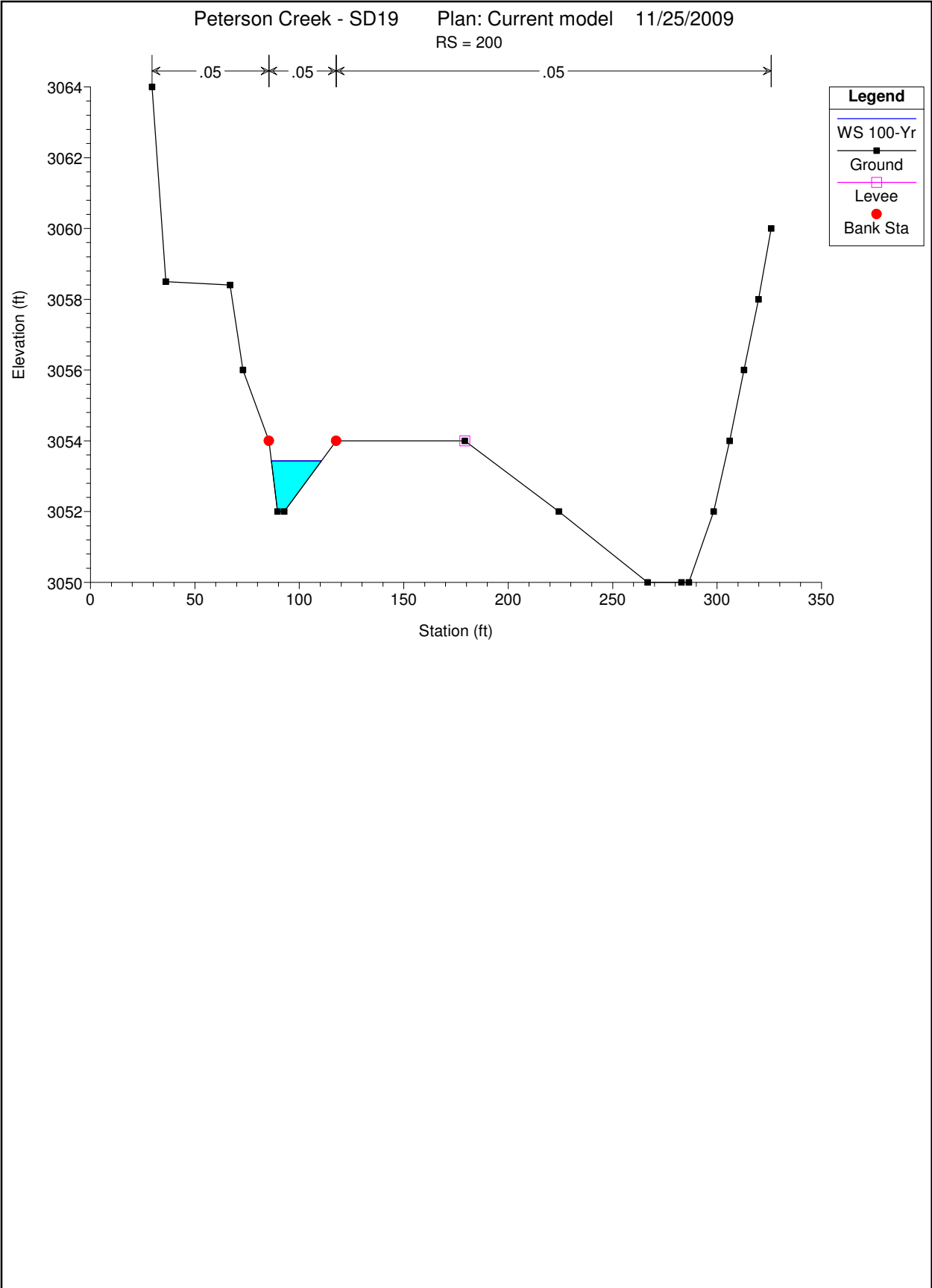








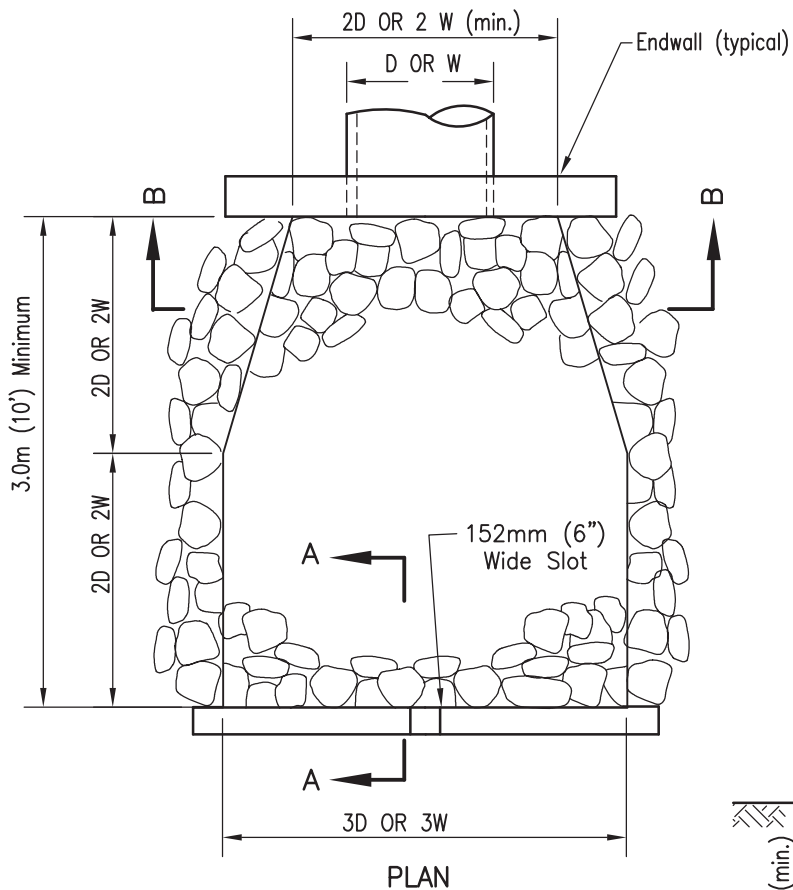




HEC-RAS Plan: SDB039600 River: River #1 Reach: Reach #1 Profile: 100-Yr

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach #1	300	100-Yr	96.21	3070.00	3071.31	3071.28	3071.78	0.035277	5.48	17.54	17.61	0.97
Reach #1	290	100-Yr	96.21	3068.00	3069.75	3069.75	3070.33	0.037476	6.11	15.76	13.77	1.01
Reach #1	280	100-Yr	96.21	3066.00	3069.05		3069.15	0.003145	2.42	39.79	21.67	0.31
Reach #1	270	100-Yr	96.21	3068.00	3068.59	3068.59	3068.79	0.049409	3.60	26.72	66.35	1.00
Reach #1	260	100-Yr	96.21	3062.00	3063.93	3063.93	3064.49	0.037402	6.03	15.97	14.26	1.00
Reach #1	250	100-Yr	96.21	3057.55	3061.07	3059.01	3061.13	0.000962	2.02	47.52	18.04	0.22
Reach #1	245		Culvert									
Reach #1	240	100-Yr	96.21	3056.77	3058.67		3058.94	0.008564	4.22	22.77	15.76	0.62
Reach #1	230	100-Yr	96.21	3056.00	3057.87	3057.84	3058.39	0.035310	5.82	16.52	14.95	0.98
Reach #1	220	100-Yr	96.21	3056.00	3056.88	3056.88	3057.21	0.042717	4.64	20.73	31.45	1.01
Reach #1	210	100-Yr	96.21	3054.00	3055.25	3055.25	3055.63	0.039532	4.91	19.60	25.69	0.99
Reach #1	200	100-Yr	96.21	3052.00	3053.44	3053.42	3053.81	0.037209	4.94	19.48	23.97	0.97

CULVERT AND RIPRAP SIZING SUMMARY TABLE

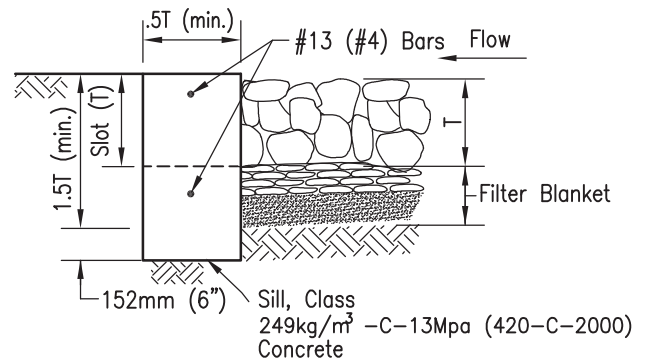


Design Velocity m/sec (ft/sec)*	Rock Classification	T (min)
1.8-3 (6-10)	No. 2 Backing	320mm (1.1ft)
3-3.7 (10-12)	220 kg (1/4 ton)	823mm (2.7ft)
3.7-4.3 (12-14)	450 kg (1/2 ton)	1.1m (3.5ft)
4.3-4.9 (14-16)	900 kg (1 ton)	1.3m (4.4ft)
4.9-5.5 (16-18)	1.8 tonne (2 ton)	1.6m (5.4ft)

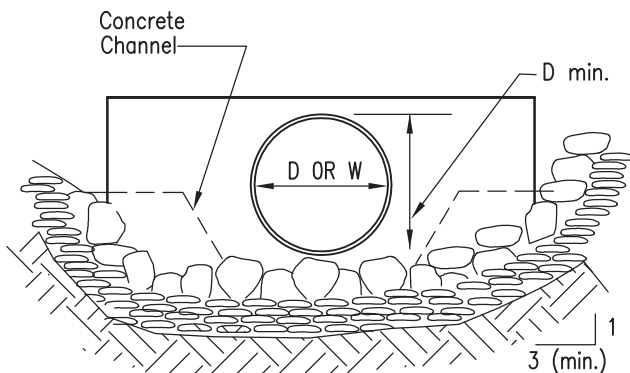
*over 5.5 mps (18 fps) requires special design

D = Pipe Diameter

W = Bottom Width of Channel



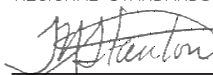
SECTION A-A



SECTION B-B

NOTES

- Plans shall specify:
 - Rock Class and thickness (T).
 - Filter material, number of layers and thickness.
- Rip rap shall be either quarry stone or broken concrete (if shown on the plans.) Cobbles are not acceptable.
- Rip rap shall be placed over filter blanket which may be either granular material or filter fabric (woven filter slit film fabric shall not be used).
- See Regional Supplement Amendments for selection of filter blanket.
- Rip rap energy dissipators shall be designated as either Type 1 or Type 2. Type 1 shall be with concrete sill; Type 2 shall be without sill.

Revision	By	Approved	Date	SAN DIEGO REGIONAL STANDARD DRAWING		RECOMMENDED BY THE SAN DIEGO REGIONAL STANDARDS COMMITTEE
ORIGINAL		Kercheval	12/75	RIP RAP ENERGY DISSIPATER		 04/27/2006 Chairperson R.C.E. 19246 Date
Add Metric		T. Stanton	03/03			
Add Rip Rap Table		S. Brady	04/06			
Edited		T. Stanton	02/09			
						DRAWING NUMBER D-40

Culvert and Rip Rap Sizing Summary Table

	SD01	SD02	SD03	SD04	SD05	SD06	SD07	SD08	SD09	SD10	SD11	SD12	SD13	SD14	SD15	SD16	SD17	SD18	SD19	SD20	SD01-P	SD02-P
Culvert Data	⁽³⁾ 31+32.45	⁽¹⁾ 33+37.23	⁽¹⁾ 38+01.54	⁽³⁾ 40+34.69	N/A	⁽³⁾ 56+21.82	⁽²⁾ 59+66.37	⁽¹⁾ 70+82.70	⁽¹⁾ 73+01.11	⁽²⁾ 79+71.02	⁽³⁾ 83+06.52	⁽¹⁾ 86+36.04	⁽²⁾ 89+63.56	⁽³⁾ 95+06.72	⁽²⁾ 101+17.00	⁽²⁾ 103+20.95	⁽⁴⁾ 107+36.02	⁽¹⁾ 115+38.09	⁽¹⁾ 120+15.30	⁽¹⁾ 123+23.61	⁽³⁾ 12+59.60	⁽³⁾ 17+60
Tributary Area (acres)	8.20	1.50	19.02	22.51	5.41	17.72	4.97	7.70	121.89	0.69	8.43	12.87	5.52	11.26	2.27	1.46	13.81	N/A	N/A	11.71	8.59	19.35
cfs/acre ratio	1.9	N/A	1.9	1.9	2.0	1.9	2.0	N/A	N/A	2.0	1.9	N/A	2.0	1.9	2.0	2.0	1.8	N/A	N/A	N/A	1.9	1.9
Discharge (cfs)	15.58	3.01	36.14	42.77	10.82	33.67	9.94	14.71	139.02	1.38	16.02	23.58	11.04	21.39	4.54	2.92	25.27	443.80	96.21	16.88	16.32	24.40
Quantity of Culverts	1	1	1	2	1	1	1	1	2	1	1	1	1	1	1	1	1	2	2	1	1	1
Diameter (in.)	24	18	30	24	18	30	18	24	42	18	24	24	18	24	18	18	30	NA	36	18	24	30
Diameter (ft.)	2	1.5	2.5	2	1.5	2.5	1.5	2	3.5	1.5	2	2	1.5	2	1.5	1.5	2.5	8x4	3.0	1.5	2	2.5
Velocity (fps)	14.06	10.65	18.30	10.39	13.16	16.93	15.65	10.74	15.63	7.52	11.66	14.71	14.85	13.79	10.13	9.41	14.13	5.18	4.22	19.90	14.95	9.02
Tailwater Depth (ft)	0.54	0.36	0.67	0.63	0.43	0.68	0.41	0.56	1.36	0.34	0.56	0.88	0.42	0.57	0.39	0.37	0.67	5.19	1.9	0.43	0.7	0.75
y _w Culvert Normal Depth (ft)	0.68	0.29	0.87	1.14	0.63	0.89	0.53	0.80	1.27	0.22	0.79	0.91	0.60	0.85	0.43	0.34	0.84	2.25	1.95	0.66	0.69	1.23
D', Adjusted Culvert Rise (ft)	1.34	0.90	1.69	1.57	1.07	1.70	1.02	1.40	2.39	0.86	1.40	1.46	1.05	1.43	0.97	0.92	1.67	#VALUE!	2.48	1.08	1.35	1.87
D ₅₀ Riprap size (ft)	0.97	0.28	1.75	2.58	1.02	1.57	1.02	0.81	3.28	0.11	0.92	0.92	1.08	1.28	0.40	0.25	1.11	#VALUE!	1.37	1.79	0.79	0.82
Edge of Road Elev. (ft)	3147.53	3149.91	3126.58	3126.10	3077.90	3056.82	3079.66	3072.37	3068.18	3095.50	3085.33	3068.78	3056.89	3074.91	3084.96	3083.06	3070.12	3042.16	3062.41	3084.14	3229.24	3190.36
Allowable Headwater Elev.	3146.53	3148.91	3125.58	3125.10	3076.90	3055.82	3078.66	3071.37	3067.18	3094.50	3084.33	3067.78	3055.89	3073.91	3083.96	3082.06	3069.12	3041.16	3061.41	3083.14	3228.24	3189.36
Computed Headwater Elev (ft)	3146.14	3146.86	3125.46	3124.19	3076.90	3055.54	3078.17	3070.13	3066.86	3091.92	3083.23	3067.78	3055.43	3073.67	3082.39	3080.22	3068.20	3037.28	3061.07	3083.13	3228.23	3189.36
Freeboard From Edge of Road (ft)	1.39	3.05	1.12	1.91	1.00	1.28	1.49	2.24	1.32	3.58	2.10	1.00	1.46	1.24	2.57	2.84	1.92	4.88	1.34	1.01	1.01	1.00
Rip Rap Per D-40				T=2.7' 1/2 Ton	N/A				T=4.4 1 Ton	T=1.1 No. 2						T=1.1 No. 2		T=1.1 No. 2	T=1.1 No. 2			T=1.1' No. 2
⁽⁵⁾ Rip Rap Per Mod. RSD D-40	T=2.5' Light L=10'	T=1.5' Light L=10'	T=3.7' 1/2 Ton L=20'		N/A	T=3.7' 1/2 Ton L=17.5'	T=2.5' Light L=10'	T=2.0' Light L=10'			T=2.5' Light L=10'	T=2.5' Light L=12'	T=2.5' Light L=10'	T=3.7' 1/2 Ton L=14'	T=1.5' Light L=10'		T=2.5' Light L=15'			T=3.7' 1/2 Ton L=12'	T=2.0' Light L=10'	

(1) Rational Method Analyses used to calculate peak discharge

(2) Peak discharge calculated using 2.0 cfs/acre ratio

(3) Peak discharge calculated using 1.9 cfs/acre ratio

(4) Peak discharge calculated using 1.8 cfs/acre ratio

(5) HEC-14 used to calculate D₅₀ for selection of rock class in Modified RSD D-40. Rock classification selection taken from CALTrans Gradation Table 5-1.

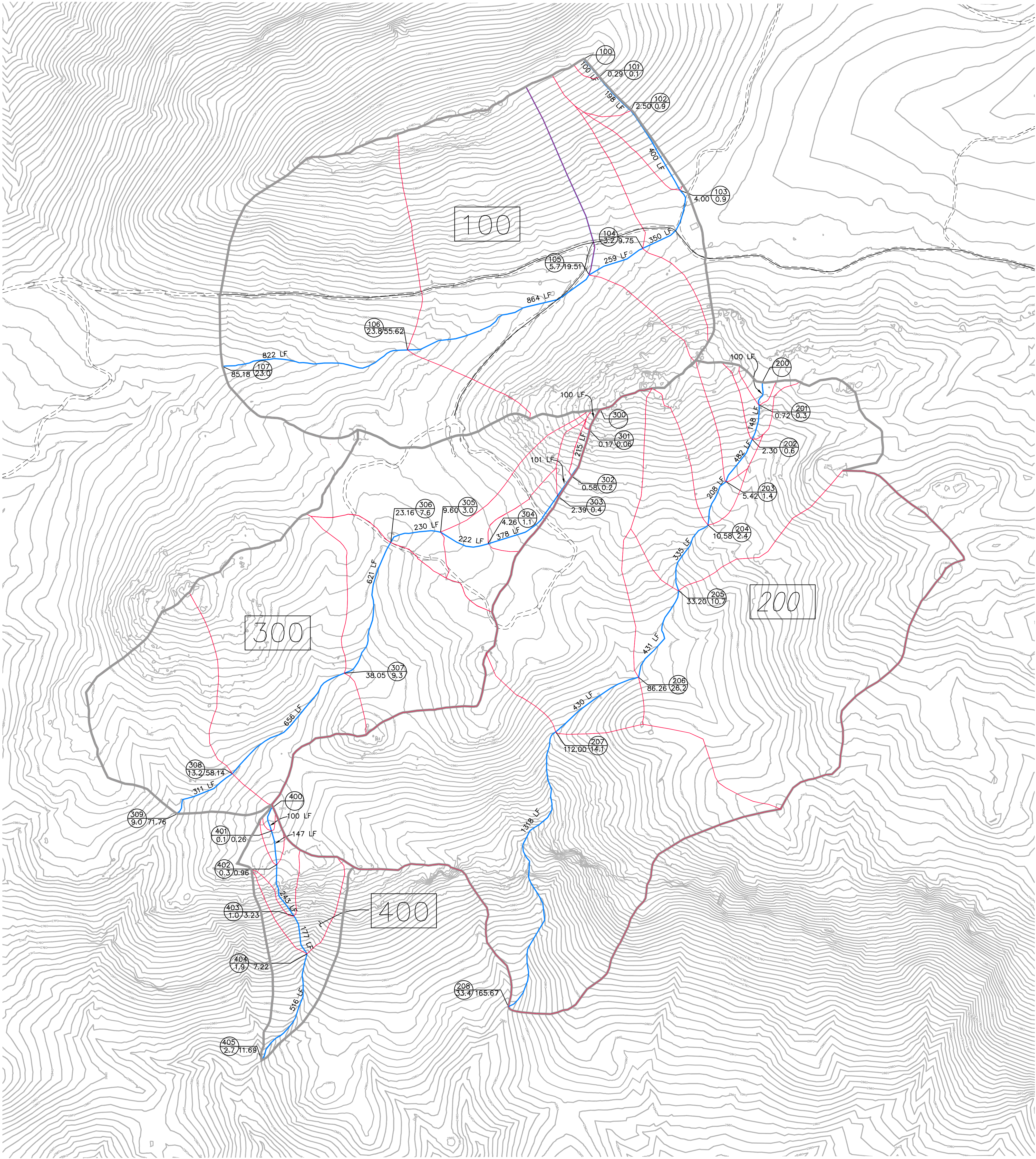
APPENDIX C: MAPS & EXHIBITS

Existing Hydrology Map

Proposed Hydrology Map

Proposed Access Road Hydrology Map

Peterson Creek HEC-RAS Exhibits



LEGEND

- 550' ——— EXISTING CONTOUR
- x577.2 ——— EXISTING SPOT ELEVATION
- MAJOR BASIN BOUNDARY
- SUB BASIN BOUNDARY
- 500 LF ——— FLOW PATH

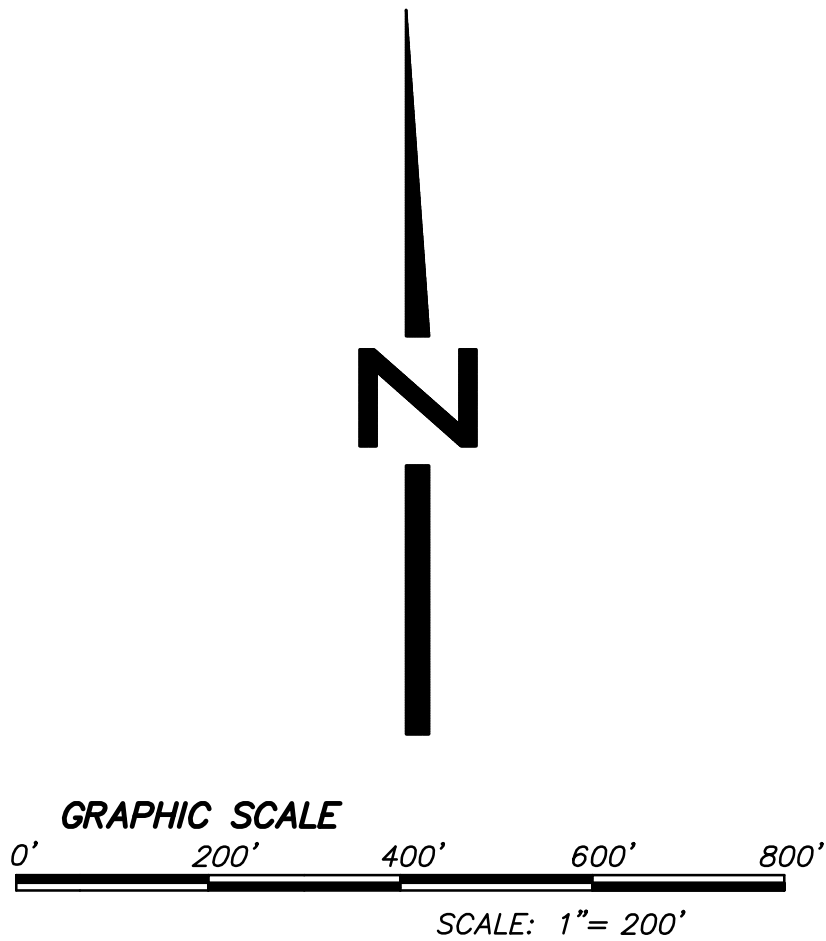
NODE

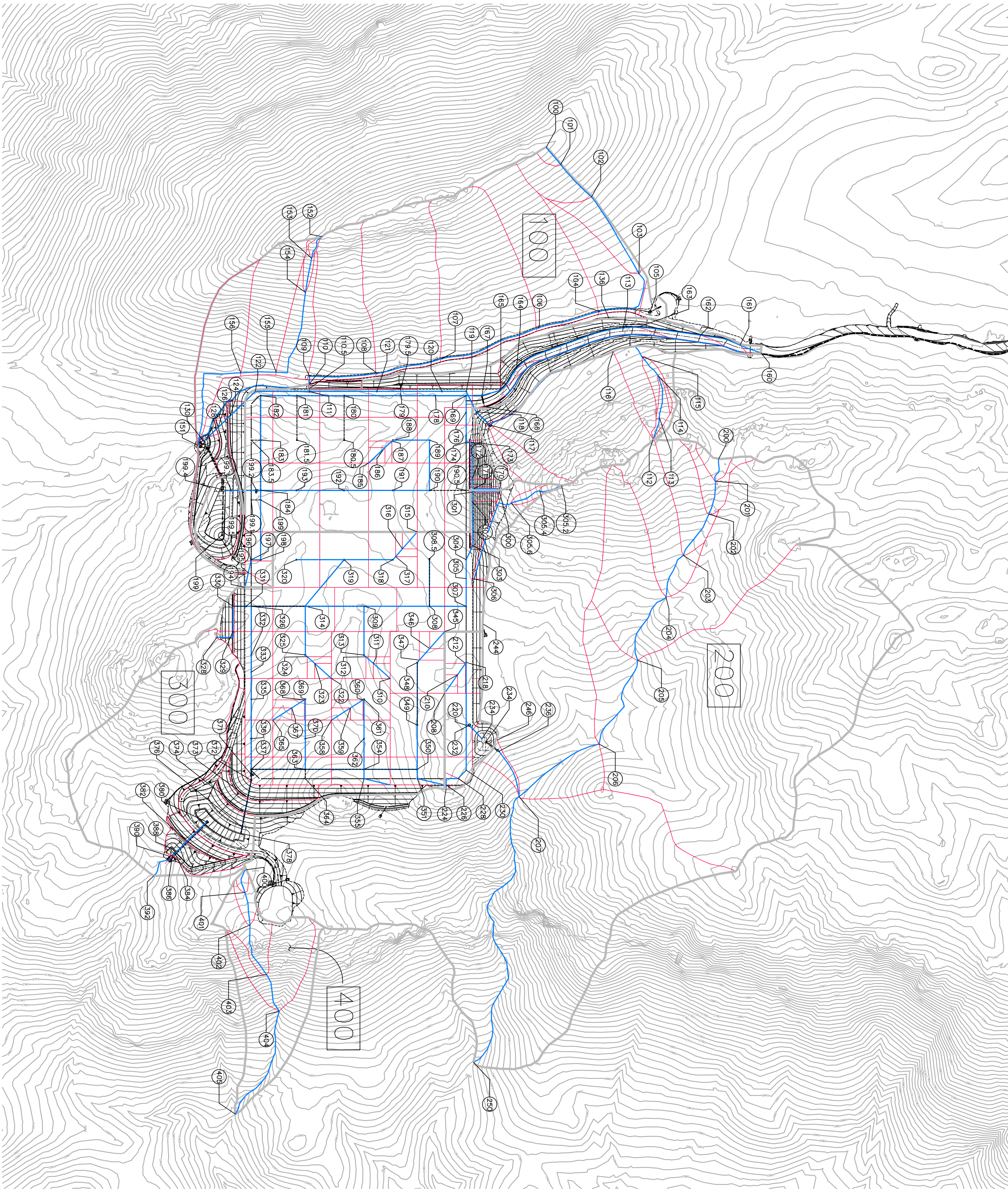
614
50.7
30.20

SUB-BASIN AREA (ACRES)

TOTAL FLOWS (CFS)

NOTE: ALL FLOWS BASED ON 100 YEAR STORM (Q_{100}).





NOTE: ALL FLOWS BASED ON 100 YEAR STORM (Q₁₀₀).

550

EXISTING CONTOUR

X=577.2

EXISTING SPOT ELEVATION

MAJOR BASIN BOUNDARY

BOUNDARY

FLOW PATH

614

NODE

0'

200'

400'

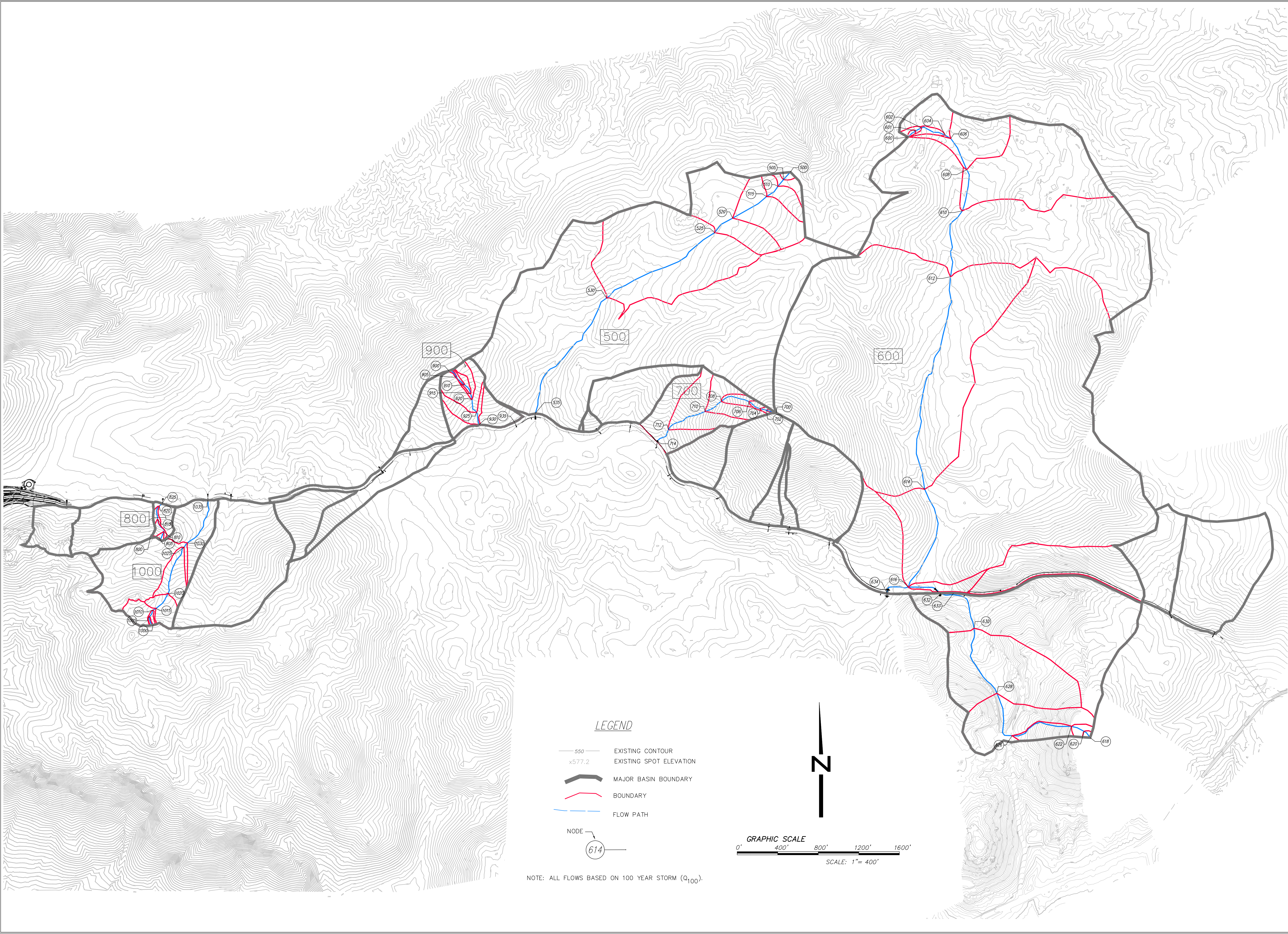
600'

800'

GRAPHIC SCALE

SCALE: 1" = 200'

N

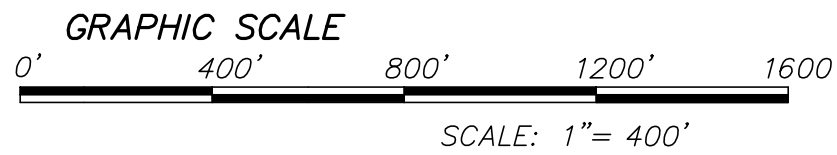


LEGEND

- 550 EXISTING CONTOUR
- x577.2 EXISTING SPOT ELEVATION
- MAJOR BASIN BOUNDARY
- BOUNDARY
- FLOW PATH
- NODE

614

NOTE: ALL FLOWS BASED ON 100 YEAR STORM (Q_{100}).



SUNCREST SUBSTATION
PROPOSED ACCESS ROAD HYDROLOGY

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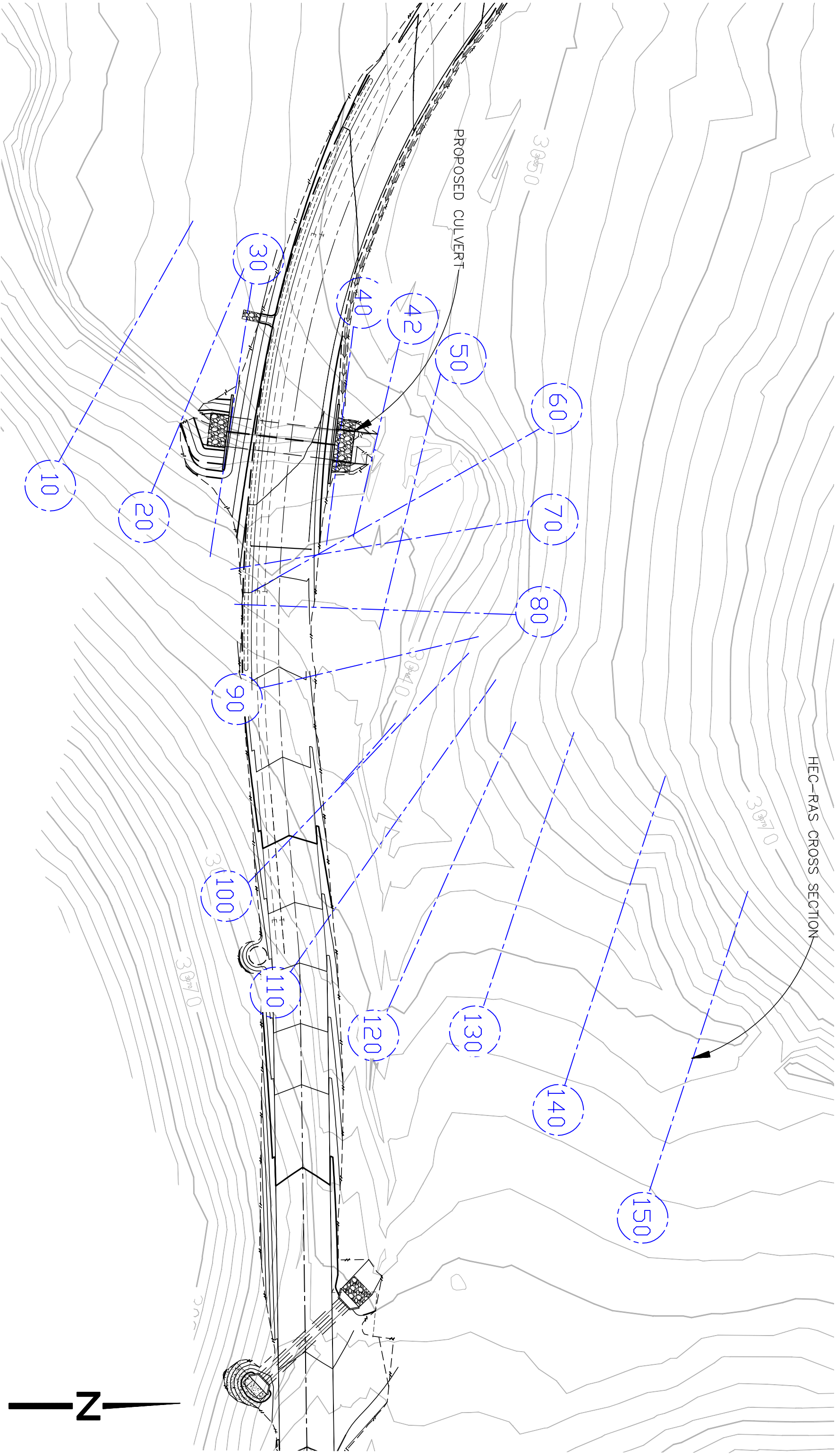
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OF **1** SHEETS
SCALE
VERTICAL: 1" = N/A
HORIZONTAL: 1" = 400'
JOB NUMBER
SDB039600

DATE SUBMITTED: 11/20/09

PREPARED FOR: BETA ENGINEERING

NO. BY DATE REVISIONS

CAUTION
The engineer preparing these plans will not be responsible for, or liable for, unauthorized changes to or use of these plans. The engineer preparing these plans must be approved by the preparer of these plans.



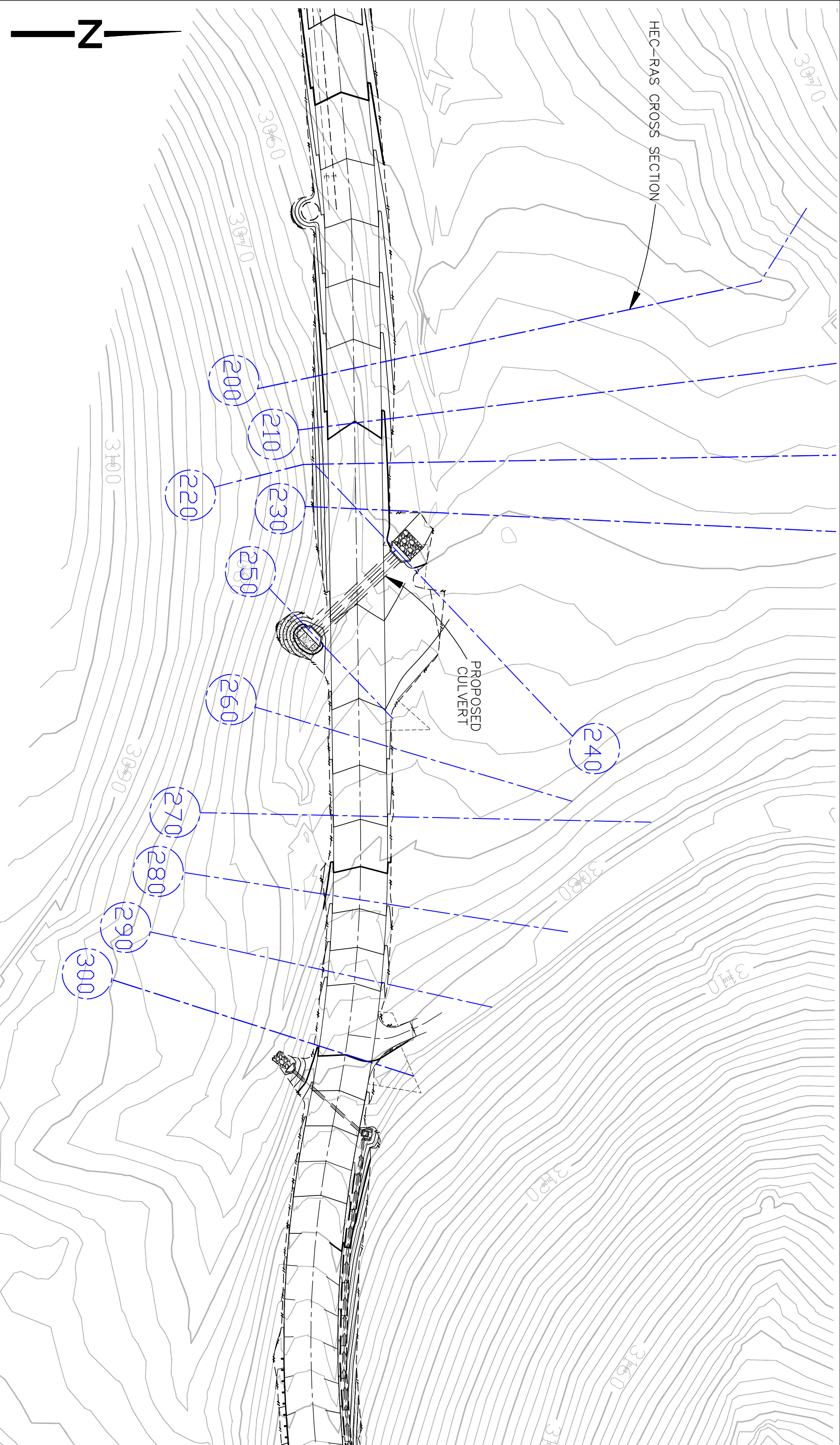
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JOB NUMBER
SDB039600



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